



PATRICK HENRY RECREATION CENTER
FEASIBILITY STUDY - APPENDIX
JUNE 5, 2015



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8.1 Existing Facility Condition Assessment

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Via US Mail and Email: (Bill Conkey <billc@sorgarchitects.com>)

9 March 2015

Bill Conkey, AIA
Sorg Architects
918 U Street NW
Washington DC 20001

Re: Patrick Henry Elementary School
Limited Building Enclosure Evaluation
WJE No. 2014.6716

Dear Mr. Conkey:

Wiss, Janney, Elstner Associates, Inc. (WJE) is pleased to provide the following report to Sorg Architects (Sorg) for the limited evaluation of the building enclosures of Patrick Henry Elementary School located in Alexandria, Virginia. This report includes our visual observations pertaining to the building enclosure, our recommendations for future repair and maintenance, and our opinions of probable life expectancy of the conditions of the building envelope observed.

DISCLAIMER

Our observations were limited to exposed and accessible exterior wall and roof elements which were visible from grade, roofs, and selected interior locations. They did not include examination of concealed wall elements. Also, our inspection was not an investigation to determine the cause or causes of problems that were discovered or may be occurring at the building. As a result, our recommendations may also include further investigation of existing conditions where appropriate.

PROJECT BACKGROUND

Patrick Henry Elementary School (PHES) was constructed in 1953 and serves students in grades Pre-Kindergarten through fifth grades in the City of Alexandria, Virginia. A total of eight modular classroom units were added around 1995. The Patrick Henry Recreation Center (PHRC) constructed in 1973 with a major addition in 1990 is attached to the western end of PHES and contains a gymnasium facility as well as offices, kitchen, and multi-purpose rooms. The original PHES is roughly H-Shaped in plan with a wing extending on the west elevation to which the PHRC is attached. The additional modular classrooms are located between the two southern wings of the H-Shaped building.

The facade of the original PHES facility is comprised of brick masonry, exterior insulation finish system (EIFS), and stucco with aluminum ribbon windows. The roof of PHES appears to be a single-ply Polyvinyl Chloride (PVC) roof membrane and several skylights are present along the low slope roof areas

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(along hallways). The PHRC is comprised of brick masonry veneer with an ethylene propylene diene monomer (EPDM) ballasted roof. The facade of the modular class rooms is comprised of open-jointed formed metal panels, and corrugated metal panels installed as an apparent rain screen system, as well as thin brick masonry veneer precast panels along the base of the walls. The roof of the modular class rooms is a thermo-set roof membrane. The library has a standing seam metal roof.

The Alexandria City Public Schools (ACPS) is in the planning stages of an expansion to the facility that currently houses PHES and the attached PHRC as well as the athletic fields. The expansion project, referred to as the Patrick Henry Capacity Project, is intended to increase classrooms and core spaces to meet the current and future demands of the City of Alexandria. The expansion project also includes an assessment of the current facilities, exterior improvements including the expansion of parking spaces, modifying existing roadways and sidewalks, and landscaping. In accordance with the ACPS, the project is broken into two major phases; the first phase is the planning and feasibility assessment and study which includes the development of multiple concepts for design of the project. The second phase will commence after the City's review and approval of the first phase and its selection of a design concept.

To assist Song in the development of the Phase I of the ACPS, Song has requested that WJE perform an evaluation of the building envelope, which includes the above grade walls and roofs, in order to identify areas of moisture and air infiltration, determine the current condition of all assemblies in order to provide an expected life expectancy, and identify any deficient conditions visible in the building envelope. It is our understanding that Song intends to use the building envelope evaluation as part of their overall evaluation of the current buildings, aiding in the development of a master plan which will compare demolition of the existing building and construction of new facilities with renovation and construction of an addition to the existing facilities.

OBSERVATIONS

On February 4, 2015; Messrs. Frederick Peters, and Joel Hackett of WJE visited the site to evaluate the building enclosure of both PHES and PHRC. Observations were made from the ground, roofs, interior rooms, and the interior common areas as noted. The purpose of our visual survey was to identify deficient conditions that we believe will warrant repair, maintenance, or other corrective action over the next 10 years. Notable conditions observed during our site visit are summarized below which are divided into separate sections for the PHES and PHRC.

Patrick Henry Elementary School

Enclosure of Original Building

- The exposed parged concrete foundation are in fair condition with isolated cracking (see Figure 1). The mortar joint between the foundation and the brick masonry is deteriorated and mortar is missing in several locations (see Figure 2).
- The below-grade waterproofing membrane is exposed in several locations on the south and east elevations and is in poor condition due to exposure and impact (see Figure 3). In two locations the membrane has debonded from the masonry wall. The below-grade waterproofing appears to have been removed during the installation of sidewalks which are poured tight to the face of the masonry without capability for expansion (see Figure 4).
- The brick masonry facade from grade to the bottom of the ribbon windows on all elevations is in generally good condition with some minor cracking through the masonry and mortar joints (see Figure 5 and 6). The brick masonry infill below the ribbon windows on the east and south elevations at locations

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of apparent abandoned vents or mechanical units is in good conditions; however, an improperly colored and possibly incorrect type of mortar was used (see Figure 7).

- The rowlock course of masonry supporting the ribbon windows is in poor condition with missing masonry units, significant spalling, cracking and mortar loss (see Figure 8 and 9). In several locations, brick masonry and mortar of the rowlock course have been replaced (see Figure 10). The mortar joint between the brick masonry wall and the rowlock course is significantly deteriorated and has vegetative growth in isolated locations (see Figure 11).
- The brick masonry piers between the doorways on the south elevation are in generally good condition with minor cracking on the top five courses (see Figure 12).
- The break metal closure installed at building expansion joints is in generally good condition; however, it is not air/water tight at horizontal seams and the perimeter sealant between the ribbon windows and/or brick masonry is in poor condition (see Figure 13).
- Corrosion is present on the exposed conduits which run along the metal fascia panels (see Figure 14). In two locations the conduit is not connected, exposing the wiring (see Figure 15).
- The plastic light protecting covers are in poor condition, are typically discolored and cracked and broken in isolated locations (see Figure 16).
- The metal panel fascia panels are in generally fair condition; however, several open vertical seams are present (see Figure 17). In isolated locations sections of the metal panel fascia have been replaced (see Figure 18). Galvanized nails secure the metal panels and are exposed and do not appear to be sealed (see Figure 19). At isolated locations the metal panel fascia is loose and dislodged and/or bowed outward (see Figure 20). The perimeter sealant and step flashing at the intersection of the metal panel fascia and adjacent facade elements is in poor condition (see Figure 21). A coating applied to metal base flashing of the fascia panels has failed on the south elevation (see Figure 22).
- The stucco between the ribbon windows and metal panel fascia panels on the south elevation is in serviceable condition with minor cracking at control joints (see Figure 23).
- The aluminum ribbon windows, including the window frames and glazing are in serviceable condition (see Figure 24). The glazing gaskets are in generally good condition with minimal shrinkage. The perimeter sealant between window frame components at operable units as well as between the window frames and adjacent facade elements is in poor condition (see Figure 25 and 26). The aluminum subsill pan flashing does not contain end dams at its termination at the structural columns which allows air/moisture to migrate behind the metal column closures (see Figure 27).
- The ribbon windows are installed between the structural steel columns which occur every nine feet. The structural columns are enclosed by a brake metal closure flashing which is intended to be sealed to the aluminum window jamb frame with two sided butyl tape. The butyl tape securing the metal column closures has failed and the closures are not well adhered (see Figure 28). Perimeter sealant is present in several locations between the ribbon window frame and the column closure which is in poor condition (see Figure 29). In several locations the column closure are dislodged exposing the steel columns and interior finishes (see Figure 30). In one location the column closure is missing (see Figure 31). The column closures are not water or air tight and allow significant moisture to migrate to the interior spaces resulting in interior damage of finishes as well as corrosion of the steel columns (see Figure 32). This condition is compounded by the lack of end dams on the subsill flashing which allows moisture to migrate around the column closure.
- In several locations including along the east elevation the fascia metal panel extends over the head track of the ribbon window preventing observation of the transitional detailing of the ribbon window (see Figure 33).

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- The metal panel canopy over the main entrance is in generally good condition. The termination of the canopy roof membrane on the brick masonry is deteriorated (see Figure 34). The sealant between the metal panels of the canopy and the adjacent brick masonry is in poor condition.
- The aluminum downspouts are in generally poor condition and in several locations are detached from the wall. Sections of the downspout are missing, particular the lower segments (see Figure 35).
- The aluminum storefront at the main entrance is in serviceable condition and the doors appear to be properly functioning (see Figure 36).
- The double and single entry doors are in serviceable condition and the doors properly function (see Figure 37). The weather stripping is in fair condition. The perimeter sealant between the metal door frame and the brick masonry is in poor condition (see Figure 38).
- Corrosion is present on the exposed areas of the steel lintels at the window openings, above louver and doorways on the north elevation at the service entrance (see Figure 39).
- The mortar around the louvers is in poor condition and is typically missing along the steel lintel (see Figure 40).
- At two locations the upper lite of glass of the ribbon window is cracked (see Figure 41).
- Aluminum punched windows are set into individual openings at isolated locations of the connecting segments of the wings connecting classroom. The aluminum window frames and glazing are in serviceable condition (see Figure 42). The perimeter sealant between the aluminum window frame and the brick masonry is in fair condition with minor adhesive failure (see Figure 43). The steel lintel above the window has been painted and has minimal corrosion (see Figure 44). Previous repairs to the mortar joints and brick masonry are present above the punched windows to address apparent cracking.
- Abandoned electrical boxes and penetrations in the brick masonry have not been appropriately patched/repared (see Figure 45).
- The EIFS over the ribbon windows on the north and east elevation is in serviceable condition with minor cracking at corners as well as minor spalls along the base above the ribbon windows (see Figure 46). The cracking at the corners is likely the result of thermal stress as no provisions for expansion are present. The metal fascia panels above the EIFS extend over the top of the EIFS and the transitional detailing cannot be observed. The reinforcement mesh is visible along the base of the EIFS and no top coat is present. The perimeter sealant between the head track of the ribbon window and the base of the EIFS is applied to the reinforcing mesh and has adhesively failed (see Figure 47). The steel column at the outside and inside corner of the buildings have been painted and the EIFS is installed tight to the column (see Figure 48). The EIFS has separated from the column and is not air/water tight (see Figure 49). The top of the column is open and does not appear to be air/water tight (see Figure 50).

Enclosure of Newer Modular Classroom Additions

- The open-jointed formed metal panel and corrugated metal panels installed on the majority of the exterior walls of the modular units is in generally good condition (see Figure 51). This system appears to be an apparent rain screen system as it is not wet glazed and the joints are open. Dry set gaskets within the metal panels system are in good condition.
- Thin brick masonry veneer which appears to be integral to precast wall segments at the base of the units as well as full height adjacent to the existing building is in generally good condition (see Figure 52).
- Pressure treated wood retaining walls are constructed around the base of the units and are filled with crushed stone to provide drainage. This system appears to be in generally good condition (see Figure 53).

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- Protection board is visible in areas with the wood retaining walls. The protection board is in generally good condition; however, in several locations it appears that the board was damaged with lawn equipment and this has exposed the waterproofing (see Figure 54). In one location, the waterproofing was damaged exposing the concrete foundation.
- The metal base flashing, copings and metal panels installed at expansion joints at the intersection of the original building are in good condition.
- The aluminum window set into individual punched window openings are in generally good condition.

Enclosure of Patrick Henry Recreation Center

- The brick masonry facade is in serviceable condition with minor cracking and spalling except at the top three courses of the parapet which is in poor condition (see Figure 55). The rowlock course of masonry along the top of the parapet wall is in poor condition and is spalled and several masonry units are missing (see Figure 56). The mortar is in fair condition, except at the top ten courses in which it is in poor condition (see Figure 57). The masonry adjacent to the northern entry doors appears to be replaced as it does not match in color or profile (see Figure 58). Cracks through the masonry are typically present at the ends of lintels for entrances, above the EIFS and at louvers (see Figure 59 and 60).
- The masonry is stained with apparent asphaltic membrane runoff along the parapet (see Figure 61).
- The concrete through-wall scuppers are in fair condition and are typically cracked with minor spalling (see Figure 62). The coating has also failed.
- In several locations, the masonry is spalled at the attachment of signs, conduits and lighting (see Figure 63).
- The conduits running to the lights are heavily corroded and have dislodged from the masonry in several areas (see Figure 64).
- The sealant installed in the masonry expansion joint is in generally good condition with minor deterioration and adhesive failure at isolated locations (see Figure 65).
- The concrete canopies over the entranceways are in poor condition and are cracked and spalled (see Figure 66 and 67). Evidence of water infiltration through the concrete canopy is present on the soffit.
- The metal emergency doors are in fair condition and are dented and damaged (see Figure 68).
- The steel lintel at the projected parapet is heavily corroded (see Figure 69).
- The downspouts are in poor condition and are corroded and missing segments, particularly at the bottom (see Figure 70). The brick masonry is in poor condition at missing sections of downspout which contributes a significant amount water to the masonry.
- The EIFS between the ribbon windows and the brick masonry on the north elevation is in serviceable condition with minor cracking and spalling along the base above the ribbon windows (see Figure 71). The perimeter sealant between the head track of the ribbon window and the base of the EIFS is in poor condition (see Figure 72). The EIFS is installed tight to the concrete columns and is sealed with a sealant joint which is in poor condition (see Figure 73).
- The aluminum ribbon windows, including the window frames and glazing are in serviceable condition. The glazing gaskets are in generally good condition with minimal shrinkage. The perimeter sealant between window frame components at operable units as well as between the window frames and adjacent facade elements is in poor condition.

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Interior

WJE discussed the performance and maintenance of the building with a representative from building facilities. The intent of our discussion was to gain an understanding of current and past issues with regard to the building envelop including air and water infiltration. Under the direction of the facility representative who provided access to the classroom and other areas, Messrs. Hackett and Peters reviewed all accessible areas which were not in use at the time of our inspection. Our inspection was performed after student dismissal; however, many after school activities were in progress which we were instructed not to interrupt.

Notable conditions in the interior include the following:

- A crack is present in the hallway leading to the library at the intersection of the newer and original building (see Figure 74).
- Several people interviewed indicated significant air infiltration into the hallways around the entrance doors. Air infiltration was detected at or around the ribbon window and entrance door assemblies in the original building which occur at three locations. At one location no seal is present between the aluminum subsill pan of the ribbon window assembly and brick masonry and daylight is visible (see Figure 75). In the other two areas, the perimeter sealant of the ribbon and entrance door assembly is in poor condition
- Several people interviewed indicated significant air infiltration along the ribbon windows at various locations throughout the original building. Several people reported that the blinds move during windy days. The interior sealant applied between the ribbon window frame and adjacent finishes is in generally poor condition (see Figure 76).
- No mechanical units or ventilation is provided in the hallways which relays on the mechanical systems in the classroom to condition the environment.
- Significant uncontrolled water leakage is present at several locations in the cafeteria, library, and throughout the recreation center (see Figure 77 through 78). In these locations the roof is low sloped. The water leakage in all three locations is typically along the perimeter of the walls, at transitions in building/roof height and around mechanical equipment. We could not gain access to the auditorium which also has a low sloped roof.
- Evidence of uncontrolled water leakage is present in 14 classrooms in the original building (see Figure 79).
- No evidence of uncontrolled water leakage is present in the newer modular classroom; however, three areas of water leakage are present in the hallways.
- Evidence of uncontrolled water leakage is present at six locations in the hallways which typically occurs at or adjacent to the skylights (see Figure 80). A newer drop ceiling is installed over top of the original ceiling tiles which makes precise identification of the leak source difficult.
- Damage to the interior finishes is typically present at the sill of the ribbon windows, particularly at or adjacent to the structural columns (see Figure 81).

Patrick Henry School Roof

The fully adhered thermo-plastic fully reinforced single-ply roof membrane assembly on the original building is in serviceable condition on the sloped section and fair condition on the low slope areas. The majority of uncontrolled water infiltration identified during our interior survey as documented above occurs on the low sloped areas of the roof or at transitions in the roof membrane. The base flashing are in generally poor condition, have been poorly installed and are not fully adhered to the parapet walls. The drains are corroded and obstructed by debris in insulated locations but appear to be functional. WJE was not able to

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walk on the sloped areas of roofs as adequate safety equipment was not immediately available and observations were made from adjacent low-sloped roof areas. WJE did not have access to the modular addition roof and all observations were made from the adjacent low roofs. The addition roof assembly appeared to be a single-ply thermo-set membrane in serviceable condition.

Notable conditions of the single-ply roof membrane include the following:

- Wrinkles are present throughout the field of the single-ply roof membrane on both the sloped and low-sloped areas; however, the membrane is largely well adhered (see Figure 82). The seams in the field of the roof are generally fully adhered and a probe could not be easily inserted (see Figure 83). Edge sealant is inconsistently applied at both vertical and horizontal seams. Where applied, the edge sealant is in generally poor condition (see Figure 84).
- Patches and previous repairs are typical on both the sloped and low-sloped roofs (see Figure 85). In several locations the patches are of different material than the single-ply roof membrane and appear to be a mixture of thermo-set and thermo-plastic products (see Figure 86). Several patches are poorly applied and are not fully adhered. Sealant is inconsistently applied to the perimeter of the patches (see Figure 87).
- In isolated locations, the single-ply roof membrane is deteriorated along the seams and the woven fabric reinforcement is exposed (see Figure 88).
- The termination of single-ply membrane with a termination bar and sealant at the intersection with the masonry parapet was poorly installed and is in generally poor condition. Several different sealants are applied between the termination bar and the masonry as well as between the bar and the roof membrane (see Figure 89).
- The single-ply roof membrane is not fully adhered on the sloped roof segment connecting the recreation center to the elementary school (see Figure 90).
- A significant bulge and adjacent depression in the roof membrane is present at the peak of the northwestern most section of sloped roofing adjacent to the recreation center low roof (see Figure 91).
- The termination sealant typically applied from the top of the mechanically fastened termination bar to the brick masonry has failed adhesively and cohesively (see Figure 92).
- Several mechanical conduits appear to have been installed following the application of the roof. The conduits are detailed with pitch pockets which are poorly applied and are in generally poor condition (see Figure 93 and 94). The patch membrane installed around the conduit penetration are typically are not the same as the single-ply roof membrane and are in poor condition.
- In mechanical vent stacks which penetrate the roof are heavy corroded. The single-ply membrane is booted around the vents and terminated with a band clamp and sealant in accordance with NRCA guidelines; however, the previous roofing membrane was not removed from the vents prior to the installation of the current roof membrane (see Figure 95). This installation does not appear to be water tight.
- The mechanical ventilation shafts are corroded. The single-ply roof membrane extends up the built-up curb supporting the unit and is terminated with a mechanically fastened termination bar and sealant. The termination sealant is poorly applied and in poor condition and as a result the base of the shaft is not fully sealed to the roof membrane (see Figure 96). Pin holes were observed at isolated locations in the target patches at the corner of the curb (see Figure 97).
- The metal C channel supports which support conduits are attached through the single-ply roof membrane on the interior face of the parapet with bolts which are inconsistently sealed (see Figure 98).

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- The base flashings are in generally poor condition and are typically not fully adhered to the parapet walls (see Figure 99). The vertical seams in the base flashing are inconsistently sealed with termination sealant. The vertical seams were roughly cut and do not appear to have a consistent overlap (see Figure 100). In several locations on the low-sloped roof areas, in locations in which the base flashing extends the full height of the parapet wall and is installed to either multiple parapet wall profiles or irregular profiles it is typically un-adhered (see Figure 101).
- The coated metal coping to which the single-ply roof membrane is terminated on the sloped roof areas is dislodged and does not appear to be water tight in several locations (see Figure 102). The application of sealant at the seams in the metal coping is inconsistent through the roof.
- The termination sealant along the termination of the single-ply roof membrane to the coated metal coping on the sloped roof areas is inconsistently applied, in poor condition and is missing in isolated locations (see Figure 103).
- Skylights manufactured by Wasco are present in the low-sloped roof areas along the hallways to provide natural light. The skylights are installed on a built-up curb on which the single-ply roofing membrane is turned up and terminated on top of the curb. The skylight metal frame wraps over the top of the curb and is typically sealed to the single-ply roofing. The sealant between the skylight glass and the metal flashing as well as between the metal flashing and the roofing is in poor condition (see Figure 104). Several of the skylights are badly scratched and discolored (see Figure 105). The outer plastic cover was broken in one location (see Figure 106). In several locations the single-ply roofing does not extend to the top of the curb and is not engaged by the termination bar (see Figure 107). At one location the termination bar for the roof membrane is mounted to the metal counter-flashing of the skylight and is not in contact with the single-ply roofing (see Figure 108).
- The exposed brick masonry on the interior of the parapet walls is in generally fair condition with cracking typical at or adjacent to the attachment of structural elements supporting mechanical equipment (see Figure 109 and 110). The mortar is also in fair condition and has bond line failure and minor mortar loss in isolated locations.
- Standing water is present on the low sloped areas of the roof, primarily above the hallways, around mechanical equipment and along the exterior perimeter of the low sloped areas (see Figure 111).
- Debris is present in the low sloped areas, primarily in the recessed areas above the hallways (see Figure 112).
- The roof access hatch is in poor condition and the weather seals have failed (see Figure 113). The access hatch is not fully sealed to the single-ply roofing.
- Aluminum gutters are present along the interior parapet at several sections of sloped roofing. The gutters do not have downspouts and drain through the holes in the gutters intended for the downspouts. In one location, an older aluminum gutter was not removed and the new gutter installed over top of the older gutter (see Figure 114).
- In one location a conduit penetrates the single-ply roofing which is not detailed to the roof membrane or installed into a pitch pocket (see Figure 115). It does not appear that this installation is water tight.
- The EIFS installed on the interior of the parapet appeared to be in good condition with minor cracking and spalling (see Figure 116).
- The single-ply thermo-set membrane roof on the classroom additions appeared to be in serviceable condition. Pounding water is present along the east and west edges of the roof (see Figure 117).
- The standing seam panels installed on the steep-sloped sections of the library roof is in good condition.

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- At one location a section of metal panel fascia is dislodged from the building, exposing the wood blocking (see Figure 118).
- The masonry chimneys are in fair conditions with minor cracking and mortar loss (see Figure 119). The concrete chimney cap is in poor condition and several spalled sections are on the roof membrane below (see Figure 120).

Patrick Henry Recreation Center Roof

The roof of the PHRC is an ethylene propylene diene monomer (EPDM) single-ply roof membrane covered with ballast. The ballast is intended to provide the weight required to prevent window uplift as the membrane is not adhered. The EPDM roof membrane is in poor condition and is nearing the end of its useful service life. WJE could not access the high roof over the gymnasium as the access ladder was not safe to use.

Notable conditions of the EPDM roof membrane include the following:

- The EPDM membrane is deteriorated, particularly at the exposed areas at curbs, transitions, separating curbs and at the base flashing (see Figure 121).
- The EPDM roof overlaps the single-ply thermo-plastic roof membrane on the connecting building segment and is poorly sealed. Several layers of sealant are present which appear to be attempts to address water infiltration (see Figure 122).
- The painted steel coping along the lower low-slope roof section is heavy corroded and the coating has failed (see Figure 123). The sealant at the seams in the coping has also failed.
- The termination sealant at the patches and seams in the EPDM membrane is in generally poor condition and additional sealant has been applied in several locations (see Figure 124).
- The base flashings are secured to the brick masonry with a mechanically fastened termination bar and sealant which is in poor condition (see Figure 125). The base flashing is not fully adhered in several locations.
- EPDM membrane is installed into a gutter which runs the length of the low roof. The membrane is not fully adhered and several voids and fish-mouths are present (see Figure 126).
- The brick masonry on the interior of the parapet is in poor condition. The mortar joints are deteriorated and open joints are present (see Figure 127). In several locations bricks are displaced and/or missing from the rowlock course of the masonry at the top of the parapet wall.
- The single-ply thermo-plastic roof membrane on the low roof over the multi-purpose room attached to the recreation center is in poor condition and is nearing the end of its service life (see Figure 128).

RECOMMENDATIONS

Based on our visual observations and our experience with similar projects, we recommend the following actions be taken to correct the conditions observed at the PHES and PHRC. Refer to Table 1 for the prioritization of these repairs and anticipated life expectancy.

Patrick Henry Elementary School

Enclosure of Original Building

- Perform repair to all cracked and spalled concrete parging. Repoint mortar joint at top of the foundation to brick masonry.

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- Repair damaged areas of below grade waterproofing to reestablish continuous waterproofing. Install new protection board over exposed below-grade waterproofing. Perform investigation of condition of below-grade waterproofing in location in which it appears to have been removed or modified to ensure that continuous waterproofing is maintained.
- Perform minor brick repair including rout-and-pointing and crack repair in accordance with BIA requirements. Cracks in stable masonry units should be routed and pointed with a mortar matching the existing. Remove and replace displaced and/or unstable units in kind. Repoint areas of deteriorated mortar. Remove existing mortar to a depth of 3/4 inch and then install a new compatible mortar.
- Remove and replace all damaged, spalled, loose or cracked brick at the rowlock course of masonry supporting the ribbon windows in kind. Repoint all head and bead joints for the rowlock course of masonry. Repoint areas of deteriorated mortar. Remove existing mortar to a depth of 3/4 inch and then install a new compatible mortar. Perform in conjunction with sealant replacement at the perimeter of the ribbon windows.
- Perform investigation to determine if the brake metal closure installed at building expansion joints is air/water tight at seams and transitions. Replace all sealant at seams and the perimeter sealant between the ribbon windows and/or brick masonry. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion.
- Repair broken and/or unconnected sections and replace heavily corroded sections of the conduit. Remove and replace any cracked or damaged light covers.
- Install new sealant at all vertical and horizontal seams, and at the intersection with adjacent facade elements in the metal panel fascia panels. Seal all fastener penetrations in the metal fascia panels. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion. Reattach any displaced or missing section of metal fascia panels.
- Perform minor repairs to stucco to address cracking and spalling. Prepare crack and install reinforcing mesh and apply based coat followed by new top coat to match existing.
- Remove and install new perimeter sealant at window frame-to-frame joints as well as frame-to-masonry around the perimeter of the ribbon windows. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion. Perform an investigation to determine the required remediation to install end dams at the termination of the aluminum subsill pan flashing. Perform in conjunction with remediation to column closure flashings.
- Perform destructive openings to determine the current detailing of the column closure flashing to the ribbon window as well as brick masonry. Remove all column closure flashing. Install new end dams for the aluminum subsill pan flashing of the adjacent ribbon windows. Apply corrosion inhibiting coating to the steel column and replace any deteriorated blocking along the structural steel column. Install water tight closure flashing along the jambs of the ribbon windows and at the masonry cavity to provide continuous waterproofing around the structural column prior to the reinstallation of new brake metal column enclosure flashing.
- Remove and install new perimeter sealant at the intersection of the canopy over the main entrance and the brick masonry. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion.
- Remove all damaged or nonfunctional sections of aluminum downspouts and replace with new aluminum downspouts to match original size, profile and color. Extend all downspouts to grade and were possible connect with below-grade drainage pipes to control runoff.

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- Remove and install new perimeter sealant between the metal door frames and the brick masonry. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion.
- Clean and paint all corroded exposed areas of the steel lintels at the window openings, louvers and doorways. Apply corrosion inhibiting coating once substrate is prepared.
- Repoint areas of deteriorated mortar at the perimeter of the louvers. Remove existing mortar to a depth of 3/4 inch and then install a new compatible mortar.
- Replace any cracked or fogged glazing lites in kind.
- Remove and install new perimeter sealant between the aluminum punched windows and the brick masonry. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion.
- Remove all abandon and non-functioning electrical boxes which penetrate the brick masonry. Replace damaged, cracking or displace brick masonry in kind.
- Perform minor repairs to EIFS on the exterior walls. Rout-and-seal cracks at control joints and corners in accordance with manufacturer's repair recommendations. Remove sealant between EIFS and head track of ribbon window and repair base of EIFS to provide base flashing. Remove sections of EIFS around exposed steel columns and install closure flashing to terminate EIFS. Install metal enclosure over columns which is integrated with EIFS closure flashing to provide air/water tight assembly. Install new sealant between the EIFS and brick masonry, ribbon windows and metal closure flashing at columns. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion.

Enclosure of Newer Modular Classroom Additions

- Repair damaged or missing areas of protection board which extend above grade. Inspect exposed waterproofing to ensure it is not deteriorated and is continuous. Repair waterproofing in accordance with manufacturer's requirements to provide continuous waterproofing.

Enclosure of Patrick Henry Recreation Center

- Perform minor brick repair including rout-and-pointing and crack repair in accordance with BIA requirements. Cracks in stable masonry units should be routed and pointed with a mortar matching the existing. Remove and replace displaced and/or unstable units in kind. Repoint areas of deteriorated mortar. Remove existing mortar to a depth of 3/4 inch and then install a new compatible mortar.
- Remove and replace all damaged, spalled, loose or cracked brick at the rowlock course of masonry at the top of the parapet wall. Repoint all head and bead joints for the rowlock course of masonry as well as masonry on the parapet walls. Repoint areas of deteriorated mortar. Remove existing mortar to a depth of 3/4 inch and then install a new compatible mortar. Perform in conjunction with sealant replacement at the perimeter of the ribbon windows.
- Repair broken and/or unconnected sections and replace heavily corroded sections of the conduit. Remove and replace any cracked or damaged light covers.
- Remove and install new sealant masonry expansion joints. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion.
- Perform repairs to concrete canopies. Remove spalled and deteriorated concrete and replace with new concrete to match the existing concrete. Rout-and-seal all cracks less than 1/8 inch wide. Apply elastomeric coating to all sides of the canopy once repairs are completed.

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- Clean and paint all corroded exposed areas of the steel lintels at the window openings, louvers and doorways. Apply corrosion inhibiting coating once substrate is prepared.
- Remove all damaged or nonfunctional sections of aluminum downspouts and replace with new aluminum downspouts to match original size, profile and color. Extend all downspouts to grade and were possible connect with below-grade drainage pipes to control runoff.
- Perform minor repairs to EIFS on the exterior walls. Rout-and-seal cracks at control joints and corners in accordance with manufacturer's repair recommendations. Remove sealant between EIFS and head track of ribbon window and repair base of EIFS to provide base flashing. Install new sealant between the EIFS and brick masonry, ribbon windows and concrete columns. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion.
- Remove and install new perimeter sealant at window frame-to-frame joints as well as frame-to-masonry around the perimeter of the ribbon windows. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion.

Interior

- Perform an investigation to determine the source of cracking in the hallway leading to the library. Ensure that the soil is not subsiding and is stable.
- Remove and install new interior perimeter sealant at window frame-to-frame joints as well as frame-to-interior finish around the perimeter of the ribbon windows. Prepare substrates and use backing materials and joint profiles to optimize service life and sealant adhesion.
- Engaged a mechanical engineer to review the current HVAC system and determine if the system is properly balanced, sufficient ventilation is provided and the interior environment is maintained as intended.
- Address water infiltration through the roof assembly as discussed below.

Patrick Henry School Roof

The majority of the reported and observed water infiltration is occurring in the low-sloped areas of the main roof including over hallways, the cafeteria, boiler room, and auditorium. Evidence of water leakage in the classrooms is typically adjacent to vertical transitions, separation curbs, penetrations in the sloped section of roof and along the perimeter of the sloped roof areas in which the slope is minimal and pounding water is present. As the single-ply thermo-plastic reinforced roof membrane is nearing the end of its service life and as a result of extensive water infiltration reported and observed, consider removing and replacing the single-ply roof membrane in the low-slope areas, all base flashing and at vertical transitions and curbing. Due to the complexity of the roof layout a built-up multi-ply roof assembly should be considered. Additionally, perform investigation at all areas with evidence of, or reported water infiltration in the sloped roof areas and repair in accordance with manufacturer's recommendations. Consider complete roof replacement if the investigation reveals wide spread deficiencies within the sloped roof areas or if the water infiltration and intended roof performance cannot be achieved.

If complete roof replacement is not performed or will be performed at a future date the following is recommended:

- Inspect all previous patches and remove all poorly adhered patches as well as patches made of a different material than the single-ply thermo-plastic membrane. Install new edge sealant compatible with the single-ply roof membrane. Prepare substrate and prime as required to achieve optimal adhesion.

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- Install target patches over all deteriorated areas in which the reinforcement mesh is exposed in the single-ply roof membrane.
- Remove all sealant along the termination of the base flashing along the termination bar and install new edge sealant between the termination bar and facade elements. Remove and reinstall the termination bar as required to achieve full compression on the single-ply roof membrane.
- Reestablish full adhesion of the single-ply roof membrane at all bulged, loose and adhered areas suspected of water infiltration.
- Inspect all mechanical conduits which penetrate the single-ply roof membrane to ensure that they are detailed in a water tight manner. Remove any poorly detailed roof membrane, pitch pockets and failed sealed. Install new single-ply roof membrane detailed in accordance with NRCA recommendations. Install new curbing as required to provide NRCA detailing at penetrations. Ensure that all conduits are of proper type and spacing and replace as required to achieve water tight penetration and comply with NRCA requirements. Remove and install new sealant around all repaired conduits.
- Remove all roof membrane on the mechanical vent stacks. Remove all corrosion and prepare the vent stacks to allow for installation of new single-ply roof membrane detailed in accordance with NRCA recommendations. Install new single-ply membrane boot around cleaned vent stacks and terminate with new band clamp and sealant.
- Inspect the single-ply membrane roofing, termination bar and sealant at the base of the mechanical ventilation shafts. Remove and reinstall the single-ply roofing membrane in location in which it does not extend to the top of the curb. Ensure that the termination bar provides compression on the single-ply membrane and remove and reinstall as required. Remove the edge sealant on the termination bar and install new compatible edge sealant. Ensure that the target patches at the base of the mechanical curb are fully adhered and sealed. Remove all poorly adhered patches as well as patches made of a different material than the single-ply thermo-plastic membrane. Install new edge sealant compatible with the single-ply roof membrane. Prepare substrate and prime as required to achieve optimal adhesion.
- Remove all unsealed fasteners for the metal C channels that support conduits, back-seal the hole in the single-ply membrane and reinstall fastener. Remove all C channels which are not in use and patch all holes in the single-ply membrane in accordance with the manufacture's requirements.
- Replace all vertical base flashing which are deteriorated, poorly adhered or damaged. Install new single-ply membrane fully adhered to prepared substrate in accordance with the manufacture's requirements. Ensure required overlap between the new and existing membrane is achieved. Install edge sealant along the vertical seams. Install termination bar along the top termination of the base flashing. In locations were on the low-sloped roof areas, the base flashing extends the full height of the parapet wall and is installed to either multiple parapet wall profiles or irregular profiles install additional termination bar to provide compression and secure the membrane in locations in which the membrane changes profile from vertical to horizontal.
- Inspect all seams in the coated metal coping and reattach any displaced sections of coping. Ensure that the coping is secured to the parapet. Remove all existing sealant at seams and transitions and install new sealant. Prepare substrate and prime as required to achieve optimal adhesion
- Remove the skylight in their entirety. Remove existing single-ply membrane which is poorly applied, does not extend the full height of the curb or it deteriorated. Install new single-ply membrane which extends onto the top of the curb. Install new termination bar at the top of the curb to secure the membrane. Reinstall the skylight over the remediated single-ply roofing and bed seal the metal frame to the single- ply membrane on top of the curb. Install new sealant joint between skylight frame and

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single-ply membrane at the termination bar to provide water tight installation. Remove and replace all sealant within the skylight assembly. Replace broken or scratched outer plastic covers.

- Address all areas of standing water which last longer than 48 hours following a rain event. Install additional slope in insulation or membrane as required to prevent ponding.
- Remove all debris from the low-sloped areas of the roof and at all drains.
- Repair the gaskets in the roof access hatch. Replace the handle for the access hatch.
- Remove double gutter assemblies and provide single gutter with downspouts that extend to the drains.
- Perform visual inspection of the single-ply thermo-set membrane roof on the classroom additions. Address all areas of standing water which last longer than 48 hours following a rain event. Install additional slope in insulation or membrane as required to prevent ponding.
- Perform visual inspection of the standing seam panels installed on the steep-sloped sections of the library to determine the source of the water infiltration occurring. Repair standing seam roof as required to provide water tight assembly as intended.
- Perform minor brick repair including rout-and-pointing and crack repair in accordance with BIA requirements to the chimneys. Cracks in stable masonry units should be routed and pointed with a mortar matching the existing. Remove and replace displaced and/or unstable units in kind. Repoint areas of deteriorated mortar. Remove existing mortar to a depth of 3/4 inch and then install a new compatible mortar. Replace the concrete chimney cap with new concrete cap to match existing profile.

Patrick Henry Recreation Center Roof

The ethylene propylene diene monomer (EPDM) single-ply roof membrane covered with ballast roof on the PHRC is nearing the end of its service life. The visible areas of the membrane are deteriorated and in poor condition, the field of the membrane cannot be observed without removal of the ballast. Due to the age of the membrane and number of reported and observed water leaks the roof assembly should be replaced with a new built-up roof assembly or fully adhered single-ply assembly. Un-adhered EPDM membrane roofs are difficult to determine the source of water infiltration as once water penetrates the membrane it is free to migrate laterally below. The expense of performing water penetration testing and removal of ballast makes repairs impractical. The adjacent single-ply low sloped roof over the multi-purpose room should also be replaced in kind as it is nearing the end of its service life.

As part of the roof replacement the following notable conditions should be addressed:

- The heavily corroded painted steel coping along the lower low-slope roof section should be removed and replaced with a stainless steel or aluminum coping.
- The gutter which runs the length of the low roof and collects all water from the higher roofs should be abandoned and other means of drain for each roof be provided. Either scuppers which extend through the parapet wall or internal drain lines should be provided.
- Perform brick repair including rout-and-pointing and crack repair in accordance with BIA requirements. Cracks in stable masonry units should be routed and pointed with a mortar matching the existing. Remove and replace displaced and/or unstable units in kind. Repoint areas of deteriorated mortar. Remove existing mortar to a depth of 3/4 inch and then install a new compatible mortar. Remove and replace all damaged, spalled, loose or cracked brick at the rowlock course of masonry at the top of the parapet wall.

OPINION OF ANTICIPATED LIFE

Table 1 contains our opinions of the anticipated life expectancy of major facade elements as well as assigning a priority for the recommendations outlined above. The table also categorizes each recommendation as follows:

- *Life Safety:* Conditions that require immediate attention to limit risk to building occupants or pedestrians.
- *Repair and Maintenance:* Conditions that require repair or maintenance to limit further deterioration or deterioration of other systems that would result from deferring maintenance.
- *Optional Improvement:* Conditions that are recommended to improve occupancy comfort or increase service life, or reduce future maintenance.

The table assigns an urgency rating to each repair required for the building envelop component identified as well as the corresponding anticipated life expectancy of that component provided that the repairs are performed in the timeframe provided.

If you have any other questions, or if we can be of any further assistance, please do not hesitate to contact us. Thank you for the opportunity to work with you on this challenging project.

Very truly yours,

WISS, JANNEY, ELSTNER ASSOCIATES, INC



Frederick Peters, P.E.
Senior Associate

Attachments
Appendix A – Figures
Appendix B – Table of Probable Cost

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Patrick Henry

Alexandria Public Schools
4643 Taney Ave, Alexandria, Virginia
Structural Condition Assessment

Prepared For:
Sorg Architects

918 U Street NW
Washington, DC 20001

June 5, 2015

Prepared By:
Keast & Hood
K&H Job No. 145212

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1.0 Executive Summary

In an effort to facilitate the planning and feasibility for the expansion and/or renovation of Patrick Henry School Elementary School, Keast & Hood (K&H) performed a structural assessment and condition assessment survey.

A site visit was conducted of Patrick Henry Elementary School by K&H engineers Matthew Daw and Laura Burke Tuesday, 9 December 2015.

The recommendations and comments herein are based on K&H's visual observations during site assessment and the professional judgment and experience of K&H engineers. The review was focused upon overall structural condition of the building as obtained through visual observation. The following documents Keast & Hood's site investigation and lists recommendations to address noted areas of concern.

An additional section has been added to address the following three (3) potential Master Plan concepts:

1. Concept 1: New Building Option 1
2. Concept 2: New Building Option 2
3. Concept 3: Renovation and Addition

2.0 General Overview and Purpose of Assessment

In order to proceed with the planning and feasibility for the expansion and/or renovation of Patrick Henry Elementary School a structural analysis was requested. The elementary school is located at 701 South Highland Street in Alexandria, Virginia.

In an effort to facilitate the development of the Master Plan Concepts Keast & Hood (K&H) was retained by Sorg Architects to perform the following tasks:

- Structural condition assessment survey related to the existing structure's capacity to accommodate renovation and expansion.
- Provide outline of geotechnical requirements to assist geotechnical project engineer in developing recommendations for the project, and
- Establish structural design criteria and identify structural requirements to further develop the three (3) master plan concepts.

3.0 Existing Documentation

K&H reviewed the following documents and reports in part or in their entirety:

1. Original base building drawings by Joseph H. Saunders, AIA and associated design team dated 01 November 1952.
2. Addition to Patrick Henry School Construction drawings by Joseph H. Saunders and associated design team dated 25 February 1955, and
3. ACPS Modular Classroom Addition Construction Drawings by Maginniss+Del Ninno Architects and associated design team dated 04 March 2011.
4. Patrick Henry Recreation Center Feasibility Study by The Arcadis/Lukmire Partnership dated October 2008.

Please reference Section 5.0 for commentary on existing structure.

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4.0 Description of Assessment Methods

Matthew J. Daw (Principal) and Laura Burke (Structural Designer) from Keast & Hood visited Patrick Henry Elementary School to conduct a site visit and structural condition assessment of the existing building condition and its surrounding area on Tuesday, 9 December 2015.

K&H Engineers performed a self-guided interior and exterior assessment of all buildings encompassed by Patrick Henry Elementary School.

When visible, the building's structural system was observed for signs of distress, impaired structural integrity, and other non-structural related concerns. Where the building's structural system was covered by finishes, K&H examined finishes for evidence of distress.

Exploratory probe work (removal of finishes at select locations) was not conducted at this time.

Photographs were taken with a digital camera to record existing conditions and areas of concern; binoculars were utilized to obtain a closer perspective, where needed; and descriptive information was recorded in field notes for the buildings encompassed by Patrick Henry Elementary School. No materials were removed for testing.

A description of noted areas of concern with photographs and recommendations is presented in Section 6.0 of this report.

5.0 General Description and Discussion of Building Structure

Originally established in 1925 to replace Alexandria’s first public school, Patrick Henry has seen 2 major renovations since the current building’s original conception in 1952.

Today, Patrick Henry Elementary School is currently home to over 20 different nationalities.

Figure 1 illustrates the individual buildings that encompass Patrick Henry Elementary School.

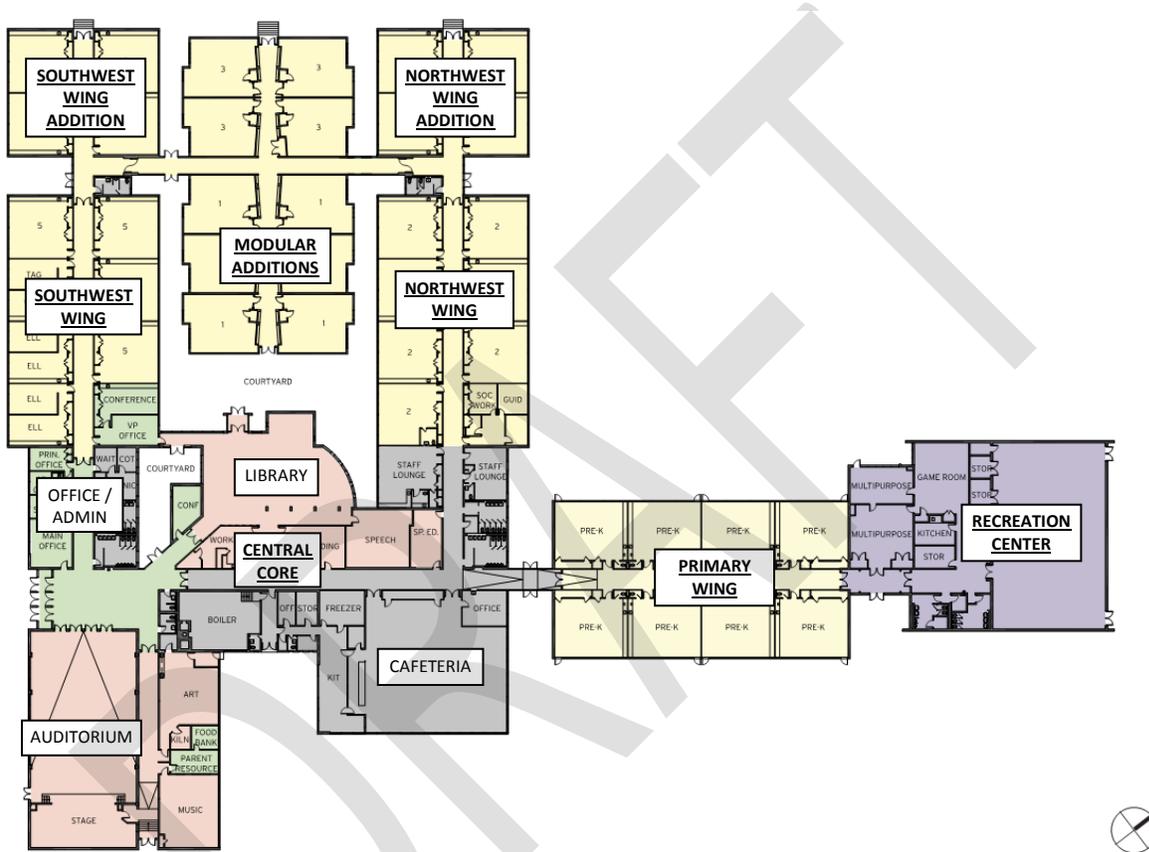


Figure 1: Patrick Henry existing building layout (Sorg Architects).

Building Construction Properties by Phase

Original 1952 Building

The following properties were obtained from item 1 noted in Section 3.0. The properties apply to the Primary Wing, Central Wing, Northwest Wing and Southwest Wing.

- Concrete Compressive Strength
 - 2500 psi for reinforced structure
 - 2000 psi for slabs on grade, wall footings, and column footings
- Live Load
 - Roof = 35 psf
 - Classrooms = 70 psf
 - Stage = 100 psf
- Soil Bearing Pressure
 - 6000 psf

1955 Building Northwest and Southwest Wing Additions

The following properties were obtained from item 2 noted in Section 3.0.

- Concrete Compressive Strength
 - 2500 psi for reinforced structure
 - 2000 psi for slabs on grade, wall footings, and column footings
- Live Load
 - Roof = 35 psf
 - Classrooms = 70 psf
 - Stage = 100 psf
- Soil Bearing Pressure
 - 6000 psf

2011 Modular Additions

The following properties were obtained from item 3 noted in Section 3.0.

- Concrete Compressive Strength
 - 3500 psi for slabs on grade and slabs on metal deck
 - 3000 psi for footings, piers, and walls.
- Loading
 - Snow Ground Load (pg) = 25 psf
 - Lateral Load - Wind
 - Wind Speed = 90 mph
 - Wind Importance Factor = 1.15

- Wind Exposure Category = B
- Lateral Load – Seismic
 - Seismic Importance Factor = 1.25
 - Short Period Spectral Acceleration = 0.16
 - (1) Second Period Spectral Acceleration = 0.053
 - Seismic Use Group = III
 - Seismic Design Category = B
 - Site Classification = D
 - Response Modification Factor = 1.5
 - Deflection Amplification Factor = 1.25
 - Seismic Base Shear = 20 kips
- Live Load
 - Slab on Grade = 100 psf
 - Classrooms = 40 psf
 - Corridor = 80 psf
 - Roof = 30 psf
- Soil Bearing Pressure
 - 2000 psf for footings on undisturbed soil or controlled structural fill

Recreational Center

Original base building drawings were not available for review.

Primary Wing

The existing Primary Wing is a single story building with structural framing consisting of two rows of sloped steel joists spanning approximately 27'-6" between steel wide flange beams. The wide flange beams are supported on exterior steel columns at the perimeter of the building and interior steel columns at the corridor. The corridor roof is framed with wide flange beams and angles spanning approximately 15'-0". Exterior steel columns are supported on a continuous wall footing around the building perimeter. Interior steel columns are supported on individual spread footings.

CMU infill walls are present to laterally brace the gravity loaded brick masonry exterior façade.

Central Core

The central core area encompasses the auditorium, cafeteria, and main corridor. The auditorium has a clear floor to ceiling height of approximately 18 feet. The existing structural framing is similar in concept to the Primary Wing with sloped steel joists spanning between steel wide flange beams. The wide flange beams are supported on exterior columns and interior columns.

A single exposed column is provided in the center of cafeteria to provide structural support for the roof members. The concept helps to reduce the structural depth necessary to accommodate the 64'-9" x 64'-9" open cafeteria space.

Steel joists support the auditorium roof load and span between exterior masonry bearing walls and wide flange steel beams. The wide flange steel beams span 50'-0" across the width of the auditorium in order to facilitate the open auditorium space below. The wide flange beams are spaced approximately 10'-10 3/8" on center and are supported by steel columns. The auditorium perimeter walls are concrete masonry block units (CMU) supporting brick masonry façade.

Additional support of the select steel roof framing is provided by masonry bearing walls, both interior and exterior supported on continuous wall footing.

CMU infill walls are present to laterally brace the gravity loaded brick masonry exterior façade.

Exterior steel columns are supported on a continuous wall footing around the building perimeter. Interior steel columns are supported on individual spread footings. The auditorium steel columns are supported on individual spread footings. A continuous wall footing is located around the auditorium perimeter between each column footing.

Northwest & South West Wings

The existing Northwest and Southwest Wing buildings are framed similar to the Primary Wing with two rows of sloped steel joists spanning approximately 28'-4" between steel wide flange beams. The wide flange beams are supported on exterior steel columns at the perimeter of the building and interior steel columns at the corridor. The corridor roof is framed with wide flange beams and steel angles spanning approximately 12'-4". Exterior steel columns are supported on a continuous wall footing around the building perimeter. Interior steel columns are supported on individual spread footings.

CMU infill walls are present to laterally brace the gravity loaded brick masonry exterior façade.

The intermediate wing between the classrooms and the central core consists of steel joists spanning 19'-2" between exterior masonry bearing walls and interior masonry corridor bearing walls. The corridor roof is framed with steel joists spanning approximately 9'-0". All CMU bearing walls are supported on continuous wall footings.

Northwest & South West Wing Additions

The existing Northwest and South West Additions are framed similar to the Northwest and South West Wing buildings with two rows of sloped steel joists spanning approximately 27'-6" between steel wide flange beams. The wide flange beams are supported on exterior steel columns at the perimeter of the building and interior steel columns at the corridor. The corridor roof is framed with wide flange beams and steel angles spanning approximately 13'-0". Exterior steel columns are supported on a continuous wall footing around the building perimeter. Interior steel columns are supported on individual spread footings.

CMU infill walls are present to laterally brace the gravity loaded brick masonry exterior façade.

2011 Modular Addition

The modular addition consists of 1-story prefabricated system. The foundations were designed as a continuous wall footing around the building perimeter with intermediate spread footings supporting concrete piers. Similarly the interior of the building foundation plan has regularly spaced piers supported on individual spread footings.

Perimeter concrete piers appear to have been designed to support columns from the modular framing prefabricated system. Concrete piers in line with the modular framing corridor appear to be designed to receive interior columns supporting roof loads.

Existing Recreational Facility

From review of existing documentation K&H understands the existing structure to be steel roof joists and infill structural steel beams supported on CMU bearing walls. Steel columns may be encased within the masonry bearing walls to provide additional support. Wall footings are designed for an assumed 2000 psf soil bearing pressure.

From review of the October 2008 Patrick Henry Recreation Center Feasibility Study by the Arcadis/Lukmire Partnership team, the recreational facility has undergone at least one renovation and addition senses its construction in 1973. Original base building and subsequent renovation drawings for the Recreational Facility were not available for K&H's review.

6.0 Discussion of Observations and Recommendations for Repairs

The following highlights areas of notable concern observed during Keast & Hood's structural investigation and condition assessment:

- Section 6.1 – Exterior Observations
- Section 6.2 – Interior Distress

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6.1 – Exterior Observations

6.1.1 – Structural Steel



Figure 2: Deterioration of MEP roof top steel.



Figure 3: Deterioration of exposed structural steel col.



Figure 4: Step Cracking in mortar joint below exposed column (see Figure 5).

Observations:

Deterioration of the MEP rooftop steel over the Northwest and Southwest Wing additions is pictured in Figure 2.

Figure 3 depicts deterioration of an exposed structural steel column (typical 3 locations observed). The exposure may have or may lead to moisture accumulation at the base of the column causing further deterioration.

Step cracking observed in Figure 4 may indicate moisture accumulation at base of exposed steel column, corrosion and additional deterioration of structural support.

Recommendations:

All steel members displaying signs of rust and distress shall be wire brushed clean to bare metal. Steel shall be re-primed and coated with an appropriate exterior grade rust-inhibitive coating. Supplemental structural work to either reinforce or replace the deteriorated framing may be required. If cleaning reveals significant steel delamination.

Where step cracking is visible a probe is recommended to investigate the integrity of the structural back-up system. Following review of the structural backup system, repair of the cracks to restore the integrity of the building envelop is recommended. Repair should consist of securing the brick to the backup structure using supplemental masonry ties, installing horizontal reinforcement across the cracks and grout injection (where necessary). Broken bricks should be replaced



Figure 5: Exposed structural steel column @ Cafeteria.

with bricks to match the existing condition. A mortar analysis is recommended to determine the appropriate color and strength of repair mortar.

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6.1.2 – Moisture Issues at Concrete Canopies at Gym Exits



Figure 6: Efflorescence staining at Gym Canopy



Figure 7: South Gym Canopy Condition

Observations:

Efflorescence staining and deterioration of the existing concrete canopies were apparent at the north elevation exists of Recreational Building.

Moisture staining is also visible at the interface of the existing brick façade and exterior concrete canopies.

Recommendations:

K&H recommends the evaluation of existing reinforcing for signs of excessive rusting. Existing canopies be sounded to detect potential areas of delamination. Partial or full depth repairs utilizing an appropriate concrete patching mortar containing a rust inhibitor are recommended at areas exhibiting deteriorated concrete. Alternatively the existing canopy structures may be removed and replaced if necessary with an alternative modern canopy system.

Furthermore, the use of gutters or other runoff collection systems may be employed to reduce the amount of staining at the interface of the existing brick façade and the exterior canopy structures.

6.1.3 – Site Grading and Drainage Issues



Figure 8: Negative Drainage & Exposed Foundation Wall.



Figure 9: Pooling of drainage at modular foundation walls (clogged)



Figure 10: Lack of connection to foundation drainage system.

Observations:

Negative drainage slope was observed around the entire building perimeter thereby allowing runoff to be diverted towards building exterior walls.

Exposed concrete foundations walls were observed at numerous locations around the building perimeter (Figure 8).

Figure 9 illustrates pooling water from a downspout directly against the foundation walls of the modular addition. This condition was noted around the perimeter of the modular addition.

Figure 10, 11, 12 and 13 illustrate inadequate downspout termination above grade (no connection to the foundation drain system) thereby allowing runoff to accumulate and sit at building exterior walls. Significant moisture staining and biological growth is evident indicating consistent moisture in Figure 10.

Figures 14 and 15 picture moisture staining of the exterior brick façade. The staining is due in part to inadequate or missing gutter runoff control systems above. In some instances the roof overhangs are insufficient to cover the projection of the brick sill courses below windows. Furthermore no flashing is apparent at skyward facing mortar joints.

Recommendations:

K&H recommends the site be regraded to raise the elevation above existing foundation walls and provide positive drainage, away from the building



Figure 11: Inadequate attachment to foundation drain.



Figure 12: Lack of attachment to foundation drain system.

perimeter.

In addition K&H recommends that the working capacity of the existing foundation drain system be evaluated. All downspouts shall be connected to the foundation drain or oriented such that runoff is directed away from the building perimeter.

Installation of flashing to protect all skyward facing mortar joints is recommended where joints are exposed runoff at window sills and similar locations.



Figure 13: Lack of attachment to foundation drain system.



Figure 14: Moisture staining of brick.



Figure 15: Moisture staining of brick

6.1.4 – Deteriorated Concrete Retaining Wall



Figure 16: Deteriorated Concrete Retaining Wall.

Observations:

Concrete deterioration was apparent at the concrete retaining wall pictured in Figure 16. Efflorescence staining is apparent along with significant out of plane movement.

In addition the anchorage of the blue railing post appears to be insufficient.

Recommendations:

K&H recommends removal of the existing retaining structure and replacement with structure sized to appropriately resist soil and applicable surcharge loading.

6.1.5 – Inadequate Flashing Detail



Figure 17: Deteriorated Flashing at Bldg Return.

Observations:

An inadequate flashing detail was observed at the northwest corner of the cafeteria exterior wall.

Recommendations:

K&H recommends the detail be revised and repaired.

6.1.6 – Open Mortar Joints



Figure 18: Cracking at lintel steel lintel bearing.



Figure 19: Open mortar joints.

6.1.7 – Perimeter Sealant Issues



Figure 20: Open joints at windows.

Observations:

Settlement cracking (Figure 18) was observed at the bearing of the brick relief angle over the auditorium windows. If a joint in the masonry is not provided at this location, the brick will naturally settle and mortar joints may crack and open due to the high load concentration at the lintel bearing.

Figure 19 depicts open mortar joints beneath the masonry sill course and window above. The condition is common around the building perimeter.

Recommendations:

K&H recommends repair of all exterior cracks and open mortar joints to restore the integrity of the building envelope. Crack repair should consist of full-depth repointing with an appropriate repair mortar or grout injection, based on observed extent and depth of cracking.

Observations:

Open sealant joints at the exterior windows were observed at numerous locations around the building perimeter.

Recommendations:

K&H recommends all open joints be repaired in order to reestablish the building envelope.

6.1.8 – Unventilated Crawl Spaces



Figure 21: Unventilated crawl space.

Observations:

The crawl spaces for the Northwest and Southwest Wing Additions were observed to be unventilated.

Unventilated crawl space traps moisture and may lead to deterioration of the structural elements.

Recommendations:

K&H recommends the installation of vented louvers to provide a means of passive ventilation within existing crawl spaces.

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6.2 – Interior Distress

6.2.1 – Moisture Distress



Figure 22: Plaster cracking at window.

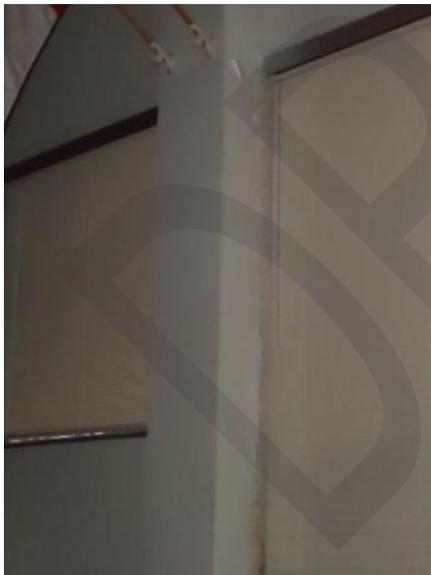


Figure 23: Signs of moisture infiltration at Auditorium Column.

Observations:

Existing moisture infiltration issues were noted throughout the building interior survey most primarily located around building windows.

Recommendations:

K&H recommends all brick masonry joints be repointed as noted in Item 6.1.6 – Open Mortar Joints in addition to the repair of all open joints around windows as noted in Item 6.1.7 – Perimeter Sealant Issues.

Repair of the cosmetic elements including peeling paint may be conducted at the discretion of the owner.

7.0 Master Plan

As part of the Master Plan concept, K&H has teamed with Sorg Architects to present the pro's and cons of three potential concepts in addition to design of a recreational building:

1. Concept 1: New Building Option 1
2. Concept 2: New Building Option 2
3. Concept 3: Renovation & Addition
4. New Recreational Building (part of Concepts 1, 2, and 3).

The following briefly discusses the structural implications for each of the above options in addition to the new recreational building.

Referenced Building Codes and Prescribed Load Criteria

- 2004 Commonwealth of Virginia Construction and Professional Service Manual (CPSM)
- 2012 Virginia Uniform State Wide Building Code (VUSBC)
- 2012 Virginia Construction Code (VCC)
- 2012 International Building Code (IBC)
- ASCE7-10, Minimum Design Loads for Buildings and Other Structures
- ACI 318-11, Building Code Requirements for Structural Steel
- AISC 360-10, Specification for Structural steel Buildings, American Institute of Steel Construction
- ACI 530-11, Building Code Requirements and Specification for Masonry Structures

The following values are specified by the applicable codes and standards or are higher values selected for use on this project:

- Structural Live Loads: The following preliminary values are minimum requirements specified by the applicable codes and standards or are higher values selected for use on this project (psf = pounds per square foot). Design of all floor areas for a minimum 100 psf live load may be considered for maximum future flexibility.

Occupancy or Use

Uniform Live Load

Classrooms	40 psf (+15 psf partition)
Labs	60 psf (+15 psf partition)
Offices	50 psf (+15 psf partition)
Libraries (stack rooms)	150 psf
Libraries (reading rooms)	60 psf
Mechanical Space (see note)	150 psf estimated (at ground and roof)
Storage	125 psf

Lobbies	100 psf
Corridors on first floor	100 psf
Corridors on upper floors	80 psf (or same as occupancy served)
Stairs	100 psf
Roof (snow)	30 psf minimum + snow drift

Note: All loading conditions due to mechanical equipment will be confirmed with mechanical engineer during the course of design coordination.

- Floor live load deflection shall be limited to 1/360 of span length.
- Roof deflection for unoccupied space shall be limited to 1/240 of the span length.
- Green roof deflection for unoccupied space shall be limited to 1/360 of the span length.
- Spandrel beam deflection shall be limited to 1/600 of the span length or 0.3 inches where masonry cladding is supported.
- Deflection of CMU or metal stud backing shall be limited to 1/720 of the vertical span length (or 1/200 * veneer thickness) where appropriate).
- Lateral building displacement due to wind loads shall be limited to h/400.
- Lateral building displacement due to seismic loads shall be limited to requirements as set per ASCE 7, depending on the selected lateral system(s).
- Floor vibrations due to walking or rhythmic excitation will be evaluated for the proposed program requirements in accordance with the provisions in AISC's Design Guide 11, Floor Vibrations Due to Human Activity.
- Wind Design Criteria:
 - Exposure C
 - Occupancy Category = III
 - Wind Importance Factor (I) = 1.00
 - Basic Wind Speed (V) = 120 mph
- Seismic Design Criteria:
 - Site Classification D (per geotechnical report)
 - Seismic Use Group III
 - Seismic Importance Factor (I) = 1.25
 - Short Period Spectral Response Acceleration Value (S_s) = 0.15g
 - 1-Second Period Spectral Response Acceleration Value (S_1) = 0.06g
 - Response Modification Factor (R) = 3.5 (Ordinary Steel Moment Frames) or
 - (R) = 3.25 (Ordinary Steel Concentrically Braced Frames)

Concept 1: New Building Option 1

Foundations:

- Per the geotechnical report foundations shall be shallow isolated column spread footings and continuous wall footings with an allowable 4,000 psf allowable bearing capacity. The foundations shall be set a minimum of 2.5 feet below top of existing grade for frost protection.
- The ground floor slab for new additions will likely be a 5+ inch slab on grade (SOG).

Superstructure:

- The primary structural system will consist of structural steel columns supported on shallow spread footing foundations. Steel wide-flange girders will span between steel columns to create a grid of structural bays. Where possible the structural framing will follow classroom and corridor building layout in order to locate columns within interior building walls.
- The elevated second and third floor structures will consist of a 2 ½" normal-weight concrete slab over 2" 20GA metal deck (assumed total depth of 4") spanning between open-web steel joists spaced at an approximate 5'-0" on center and spanning the length of each classroom across each classroom. The vibration performance of the above noted open-web steel joists is a concern and will be evaluated further. Welded wire mesh will be used to reinforce the concrete slab with supplemental rebar reinforcing.
- The depth of the elevated floor structures may change based on floor occupancy, vibration and acoustical requirements.
- For a building of construction type IIA, a one-hour fire rating will be required for all structural superstructure framing, likely with sprayed-on fire-proofing. The slab on metal deck was selected to allow for a 1-hour fire rating without requiring sprayed-on fire-proofing, per UL-D916.
- The roof structure consist of a 2+ normal-weight concrete slab over 1-1/2" 20GA metal deck (assumed total depth of 3½") spanning between 20+ open-web steel joists spaced at 5' on center and spanning from exterior to the corridor. The above noted design will increase to support an intensive green roof with 18" soil depth (if desired). Additional structural steel wide-flange beams will be required at the roof to support mechanical units and other architectural elements such as solar chimneys and skylights.
- In order to accommodate the open geometry and proposed window framing between structural steel framing, the lateral system may consist of structural steel Braced Frames or Moment Frames.

1. Braced Frames:

- Cons:
 - Diagonal braces between columns may visually extend through open glass extents at exterior and interior walls.
- Pros:
 - Reduced size of structural steel framing (weight)
 - Connections are less expensive and labor intensive as compared to moment frame connections.

2. Moment Frames:

- Cons:
 - Column and beam framing sizes are significantly heavier per linear foot than required for braced frames. Often increase required depth of ceiling structure.
 - Connections are more expensive and labor intensive.
- Pros:
 - Allows for large extents of visually uninterrupted glass extents at exterior and interior walls.

Moment and braced frames will be located at select classroom dividing walls in both the transverse and longitudinal directions and oriented to allow doors for classroom access.

Site Exterior Slab/Stair/Ramp:

Non-Structural Elements:

- Exterior walls are expected to consist of cold-formed steel (metal Stud) framing with large extents of glass.
- Solar Chimneys
- New mechanical units are expected to be located?
- Intensive vs Extensive Green Roof
- Solar Screens
 - Thermal Isolation breaks shall be provided by utilizing bolted connections and a product similar to Fabreka's Thermal Insulation Material.
- Brick Cladding

Concept 2: New Building Option 2

The structural system will be similar as noted for Option 1.

The primary difference between Option 1 and Option 2 new construction will be location and configuration of the structural columns and beams. Where possible the structural framing will follow classroom and corridor building layout in order to locate columns within interior building walls.

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Concept 3: Renovation & Addition

Foundations:

- Per the geotechnical report foundations shall be shallow isolated column spread footings and continuous wall footings with an allowable 4,000 psf allowable bearing capacity. The foundations shall be set a minimum of 2.5 feet below top of existing grade for frost protection.
- Per further structural analysis and design, where the existing building is to remain 1-story in height, existing foundations may be structurally sufficient to support proposed renovations. However, new shallow foundations will be required at the current modular classrooms and the southwest wing floor level additions.
- New shallow foundations will be required at the modular classroom infill and the southwest wing floor level additions
- The ground floor slab for new additions will likely be a 5+ inch slab on grade (SOG).

Superstructure:

- Where existing building structures are to remain, new framing may be required. New framing may include but is not limited to the following locations:
 1. Penetrations through or elimination of masonry bearing walls,
 2. Roof penetrations including skylight and MEP, and
 3. Elimination of existing structural steel framing such as columns in order to create larger clear spans.
- The primary structural system for the new southwest wing renovation and modular classroom infill will consist of structural steel columns supported on shallow spread footing foundations. Steel wide-flange girders will span between steel columns to create a grid of structural bays.
- The elevated second and third floor structures will consist of a 2 ½" normal-weight concrete slab over 2" 20GA metal deck (assumed total depth of 4") spanning between open-web steel joists spaced at an approximate 5'-0" on center and spanning the length of each classroom across each classroom. The vibration performance of the above noted open-web steel joists is a concern and will be evaluated further. Welded wire mesh will be used to reinforce the concrete slab with supplemental rebar reinforcing.
- The depth of the elevated floor structures may change based on floor occupancy, vibration and acoustical requirements.

- For a building of construction type IIA, a one-hour fire rating will be required for all structural superstructure framing, likely with sprayed-on fire-proofing. The slab on metal deck was selected to allow for a 1-hour fire rating without requiring sprayed-on fire-proofing, per UL-D916.
- The roof structure consist of a 2+” normal-weight concrete slab over 1-1/2” 20GA metal deck (assumed total depth of 3½+”) spanning between 20+” open-web steel joists spaced at 5’ on center and spanning from exterior to the corridor. The above noted design will increase to support an intensive green roof with 18” soil depth (if desired). Additional structural steel wide-flange beams will be required at the roof to support mechanical units and other architectural elements such as solar chimneys and skylights.
- In order to accommodate the open geometry and proposed window framing between structural steel framing, the lateral system may consist of structural steel Braced Frames or Moment Frames.
 1. Braced Frames:
 - Cons:
 - Diagonal braces between columns may visually extend through open glass extents at exterior and interior walls.
 - Pros:
 - Reduced size of structural steel framing (weight)
 - Connections are less expensive and labor intensive as compared to moment frame connections.
 2. Moment Frames:
 - Cons:
 - Column and beam framing sizes are significantly heavier per linear foot than required for braced frames. Often increase required depth of ceiling structure.
 - Connections are more expensive and labor intensive.
 - Pros:
 - Allows for large extents of visually uninterrupted glass extents at exterior and interior walls.

Moment and braced frames will be located at select classroom dividing walls in both the transverse and longitudinal directions and oriented to allow doors for classroom access.

Site Exterior Slab/Stair/Ramp:

Non-Structural Elements:

- Exterior walls are expected to consist of cold-formed steel (metal Stud) framing with large extents of glass.
- Solar Chimneys
- New mechanical units are expected to be located?
- Intensive vs Extensive Green Roof
- Solar Screens
 - Thermal Isolation breaks shall be provided by utilizing bolted connections and a product similar to Fabreeca's Thermal Insulation Material.
- New steel framed open stairs are to be adjacent to the southwest wing addition.
- Brick Cladding

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New Recreational Building

Through discussions with Sorg Architects, two concepts are proposed for the new Recreational Building:

- Concept A – Freestanding, 2-part Building
 - Pre-Engineered Long Span Structure
 - House indoor soccer field with elevated running track.
 - 1-Story Facility Space
 - House fitness rooms, a multi-purpose room, offices, locker rooms, and other amenities.
 - Facility Space shall be attached to southeast corner of new educational building.
- Concept B – Freestanding, 2-part Building
 - Pre-Engineered Long Span Structure
 - House indoor soccer field with elevated running track.
 - 2-Story Facility Space
 - House a multi-purpose room, offices, restrooms, and other amenities on the First Floor.
 - House a fitness room and additional single occupant restrooms on the Second Floor.
 - Facility Space First Floor shall be attached to southeast corner of new educational building.
- Concept C – Addition to New Education Building
 - 1-Story Addition to Educational Building
 - House flex court with perimeter walking/running track, a multi-purpose room, offices, restrooms, and other amenities.
 - Addition shall be attached to southeast corner of new educational building.

Concepts A & B

- The structural engineer for the project will provide foundation design to accommodate the new pre-engineered long span structure in addition to complete design for the 2-Story Facility Space.
- Important aspects for the prefabricated long span structure include design and accommodation of the elevated running track. Calculations shall include consideration of vibrational and impact loads for the elevated track.
- The adjacent Facility Space will likely be a steel framed structure, with CMU or light gauge infill walls. The elevated Second Floor for Concept B will need to be evaluated for proposed fitness space use including vibrational aspects associated with weight lifting and cardio equipment.

Concept C

- Framing of the recreational addition to school Option 2 will continue the primary structural system of structural steel columns supported on shallow spread footing foundations. Steel wide-flange girders will span between steel columns to extend the grid of structural bays. Where possible the structural framing will follow the designated room layout and corridor building layout in order to locate columns within interior building walls.
- The roof structure will likely consist of 2+” normal-weight concrete slab over 1-1/2” 20 GA metal deck (assumed total depth of 3½”) spanning between open-web steel joists spaced at 5’-0” on center spanning from exterior perimeter walls to the corridor walls.
- The roof structure over the flex court with perimeter walking/running track will be framed with long-span steel trusses to perimeter steel columns and/or CMU bearing walls.
- In order to accommodate the open geometry and proposed window framing between structural steel framing, the lateral system may consist of structural steel Braced Frames or Moment Frames.
 1. Braced Frames:
 - Cons:
 - Diagonal braces between columns may visually extend through open glass extents at exterior and interior walls.
 - Pros:
 - Reduced size of structural steel framing (weight)
 - Connections are less expensive and labor intensive as compared to moment frame connections.
 2. Moment Frames:
 - Cons:
 - Column and beam framing sizes are significantly heavier per linear foot than required for braced frames. Often increase required depth of ceiling structure.
 - Connections are more expensive and labor intensive.
 - Pros:
 - Allows for large extents of visually uninterrupted glass extents at exterior and interior walls.

Moment and braced frames will be located at select program space dividing walls in both the transverse and longitudinal directions and oriented to allow doors for classroom access.

8.0 Conclusion

Overall, the existing building structure appears to be in good condition.

Please reference preceding sections 6.1 through 6.2 for detailed observations and recommendations.

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The recommendations and comments contained herein are based on K&H's visual observations during site assessment of the existing conditions and the professional judgment and experience of K&H engineers. This report represents the extent of Keast & Hood's review.

Please do not hesitate to contact the undersigned if we can be of continued assistance or if we may answer any questions regarding K&H's observations and recommendations.

Very Truly Yours,

KEAST & HOOD



Laura M Burke, EIT



Matthew J Daw, P.E., LEED® AP

DRAFT



christopher
consultants

Patrick Henry School and Recreation Center

Site Civil Analysis

Prepared for:
Sorg Architects
918 U Street NW
Washington, D.C. 20009

Prepared by:
Christopher Consultants
9900 Main Street #400
Fairfax, VA 22031

June 5, 2015

PATRICK HENRY SCHOOL AND RECREATION CENTER

Site Grading/Drainage Analysis

Existing Conditions

The approximate 13 acres site can be roughly characterized with grades that range from flat to moderate for the majority of the site. The grade differential for most of the site is less than 5% with a ridge line that extends from Taney Avenue through the existing school to an apex in the property's northeast corner where a tennis court sits over 15 feet higher than the rest of the property.

The majority of the site drains to a collection point at the intersection of North Latham and Taney with the remainder draining to the southeast corner near the existing school's entrance off of Taney Avenue.

Proposed Alternatives

Each alternative for the new school proposes to construct the new school while the existing school is occupied. As such, the new school will need to be graded and sited on the existing grounds in a fashion that allows the existing school to continue to function, thus dictating that existing drainage patterns will need to be mostly maintained without a drastic revision to the grades around the site. The new school and recreation building will be relatively close in finished grade elevations to maintain a walkable and accessible site to each component of the new facilities.

The new construction will need to anticipate reaching a balanced earthwork on the site with excess cuts generated from construction of the new school and its foundation being utilized to replace the void left from the demolition of the existing school and creation of the various activity fields proposed. Cut material generated from the removal of the existing tennis courts in the northwest corner would be transferred to the new parking lots located on the north side of the building and the areas to the south and west of the new school.

Care will need to be taken on the design of the loop road proposed on the property's north side. Drainage from the wooded property to the north will need to be addressed with any design. The water will need to be conveyed through the property to the existing outfall locations previously described.

To reach its apex height behind the tennis court, the new loop road will require grades approaching at least 5% as it travels along the east side of the property heading north after passing the new recreation center in all of the alternatives. It is envisioned that the road will approach grades 10 - 12 feet higher than the rest of the site as it turns the corner and heads west and downward at a more moderate slope towards North Latham and the parking facilities on the north side of the site.

The new school will sit at moderate grade above Taney Avenue and North Latham Avenue as it will be required to design each buildings at an elevation slightly higher than existing grade to accommodate the drainage patterns that approach from the north side of the site. The new school will act as a drainage divide with water from the roof equally split to the north and south. Runoff on the north side of the building will be piped around the new school to the west. Grades will need to be adjusted to allow for the additional runoff and piping materials to allow water to be collected and piped around the building. The east side of the new buildings will be graded in a manner that will be compatible with the existing parking lot located along the property border as functionality of the existing school will be required

while the new construction proceeds. As noted before the new school will rest slightly higher than Taney and North Latham Avenues on the projects south and west borders with moderate slopes to the roadway. Runoff will be captured, treated and directed to the current outfalls via a storm pipe network that will be design around the existing school.

Stormwater Management

Existing Conditions

The existing site includes an elementary school with associated parking lots, access drives, athletic fields, tennis courts, recreation center, and playgrounds. Approximately 32% of the existing land cover type is impervious. The north portion of the site is vegetated and sloped and has been left in its natural condition.

The only water quality or quantity measures currently onsite treat the modular classroom addition that was constructed in 2011. Ten flow-through planter box sand filters were connected to the addition roof drains providing water quality treatment for the new modular addition. In order to meet City requirements for control of water quantity generated by the addition, the modular project installed a 29"x45" elliptical pipe downstream of the planter boxes. This system then connects to the existing onsite stormwater system that flows to the west toward the intersection of North Latham Street and Taney Avenue. There are no known existing BMP or SWM features treating the remainder of the site.

The remainder of the site's runoff is uncontrolled and connects the existing City drainage system at two locations. The first outfall is at the southeast corner of the property near the location of the parking lot entrance at Taney Avenue. The second outfall location is to the west toward the intersection of North Latham Street and Taney Avenue.

Water Quality

On March 15, 2014, the City of Alexandria adopted a new Article XIII of the City Ordinance that instituted new and more stringent requirements for addressing and mitigating stormwater runoff and pollutants. The new regulations include the continuation of previous City requirements that require providing the Water Quality Volume Default which requires providing the volume equal to the first 0.5 inch of runoff for the total impervious area of the site. In addition to these requirements, the new regulations will enforce limits of total phosphorous loading of 0.41 lb/acre for any new development or require redevelopment projects to reduce current phosphorous loads by 10% or 20% depending on the size of the project, 10% reduction for sites under one acre and 20% reduction for sites greater than one acre.

Currently, approximately 32% of the existing site is impervious, and based on the three layout options all three options will have an overall increase in impervious area; Option 1A increases the impervious cover onsite approximately 14%; Option 2 approximately 17%; and Option 3 approximately 19%. The total phosphorous for the new additional impervious area cannot exceed 0.41 lb per acre year. For the existing impervious areas, the calculation shall be designed to reduce the total phosphorous load 20% below the predevelopment total phosphorous load. These calculations will be performed through the use of the Virginia Runoff Reduction Method and associated worksheets.

In addition to the phosphorous loading requirements, the City's requires that all the impervious area is treated directly by providing a minimum of ½ inch of water quality storage for all impervious areas. For

each of the options the storage requirements will range between 11,470 cf for Option 1 to 12,808 cf for Option 3 (Renovation).

To achieve these limits and meet the new standards, the water quality requirements will be met through the use of green roofs proposed with the new architecture, grass swales, bioretention areas, selected areas of permeable pavement including along pathways and within the parking lot, and/or other aesthetic water quality features in accordance with the City and State regulations. The selected BMP will be properly determined based on the area of drainage, and its location within the site.

Treatment of parking areas for each option proposed will be through the use of a combination of dry swales, bioretention facilities, and manufactured BMPs such as a Stormfilter. Where appropriate the new parking lots will be designed and constructed to sheet flow into grass swales and bioretention facilities including parking islands.

For the new school, recreation facility, and the surrounding pathways, water treatment will be addressed through the use of a green roofs, bioretention areas, dry swales, and selected areas of permeable pavers. All features may be integrated into the campus design as interactive children's outdoor classrooms. If funding is available, we would recommend utilizing a cistern or other rainwater harvesting structure to capture and store surface runoff generated from the impervious areas to reuse for irrigation of the school's grounds. Direct connections from the buildings roofs could be directed into the underground facility along with the water captured from the sidewalks. The cistern would also provide additional storage to meet stormwater quantity requirements for the project.

Water Quantity

The new stormwater requirements require both channel protection and flood protection. Channel protection requirements consist of ensuring that the 2-year storm outfall does not create an erosive condition. To meet flood protection requirements, the project's post-development runoff for the 10-year storm may not exceed its current 10-year storm runoff.

As discussed above, there are two primary outfalls to the school site. These outfalls are two existing storm sewer systems that drain to the west and south east corners of the property. It is anticipated that both outfall points discharge in Holmes Run located south of the school property and therefore, we do not anticipate any significant design challenges to meet the channel protection requirements. The new project, however, will need detention measures to meet the flood protection requirements set forth in Article XIII.

To provide the required detention volume, bioretention areas, dry and wet swales will be designed to provide additional volume for water quantity; however, they will not be able to accommodate all the detention required to meet the new stormwater management regulations. Underground stormwater management facilities will be needed to detain the necessary water quantity. These facilities can also be designed in coordination with a cistern to provide water quality measures and support for irrigation for the new school grounds to reduce the peak runoff to meet pre-development rates.

GENERAL NOTES

- 1. THE PROPERTY SHOWN HEREON IS IDENTIFIED ON THE CITY OF ALEXANDRIA, VIRGINIA GEOGRAPHIC INFORMATION SYSTEM AS ACCOUNT #30544000 AND IS CURRENTLY ZONED R-12
2. THE PROPERTY SHOWN HEREON IS NOW IN THE NAME OF THE CITY OF ALEXANDRIA, RECORDED AT DEED BOOK 344 AT PAGE 89 ALL AMONG THE LAND RECORDS OF THE CITY OF ALEXANDRIA, VIRGINIA.
3. DURING THE PROCESS OF OUR PHYSICAL SURVEY NO INDICATIONS OF A CEMETERY WERE FOUND. NO FURTHER INSPECTION OF THESE PROPERTIES HAVE BEEN MADE FOR POSSIBLE CEMETERIES.
4. NO TITLE REPORT FURNISHED. ALL UNDERLYING TITLE LINES, EASEMENTS, SERVITUDES AND OTHER MATTERS OF TITLE MAY NOT BE SHOWN HEREON.
5. THE BOUNDARY AND PHYSICAL IMPROVEMENTS SHOWN HEREON ARE BASED UPON A FIELD SURVEY DONE BY THIS FIRM BETWEEN THE DATES OF NOVEMBER 25, 2014 AND FEBRUARY 3, 2015.
6. A.) HORIZONTAL DATUM SHOWN HEREON IS REFERENCED TO THE VIRGINIA COORDINATE SYSTEM (VCS) 1983 - NORTH AS ESTABLISHED FROM A CURRENT GPS SURVEY.
B.) THE VERTICAL DATUM SHOWN HEREON IS REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88) AS ESTABLISHED FROM A CURRENT GPS SURVEY.
7. NO GEOTECHNICAL, SUBSURFACE, FIELD REVIEWS, RESEARCH, AGENCY OR GOVERNMENTAL RECORD REVIEWS, OR OTHER INVESTIGATIONS HAVE BEEN MADE FOR THE PURPOSE OF LOCATING, OR DETERMINING THE EXISTENCE OF WETLANDS, HAZARDOUS MATERIALS, OR OTHER ENVIRONMENTAL CONCERNS ON SITE IN THE PERFORMANCE OF CHRISTOPHER CONSULTANTS, LTD SERVICES FOR THE PROJECT AS SHOWN HEREON.
8. THE PROPOSED FUTURE WIDENING SHOWN HEREON IS BASED INFORMATION FOUND ON TWO FLATS RECORDED AT DEED BOOK 394 AT PAGE 379 AND DEED BOOK 432 AT PAGE 104.

FLOOD ZONE NOTE

THE AREA SHOWN HEREON IS LOCATED ON THE FLOOD INSURANCE RATE MAPS (FIRM), COMMUNITY PANEL NO. 515590028 E, WITH AN EFFECTIVE DATE OF JUNE 16, 2011. BY GRAPHICALLY DEPICTION ONLY, THE PROPERTY SHOWN HEREON IS SHOWN IN FLOOD ZONE "X" (OTHER AREAS), AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN. A FIELD SURVEY WAS NOT PERFORMED TO DETERMINE THE FLOOD ZONES LISTED HEREON. AN ELEVATION CERTIFICATE MAY BE NEEDED TO VERIFY THIS DETERMINATION OR APPLY FOR A VARIANCE FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY.

UTILITY MARKING NOTES:

- 1. THE LOCATION OF UTILITIES SHOWN HEREON ARE FROM OBSERVED EVIDENCE OF ABOVE GROUND APPURTENANCES AND SURFACE GROUND MARKINGS.
2. BEFORE DIGGING IN THIS AREA, CALL "MISS UTILITY" 1-800-552-7001 FOR FIELD LOCATIONS (REQUEST FOR GROUND MARKINGS) OF UNDERGROUND UTILITY LINES.
3. UTILITY PROFESSIONALS INC. MARKED FOR UNDERGROUND UTILITIES BETWEEN THE DATES OF NOVEMBER 25, 2014 AND FEBRUARY 5, 2015 AND LOCATED BY THIS FIRM BETWEEN THE DATES OF NOVEMBER 25, 2014 AND FEBRUARY 5, 2015.



STORM STRUCTURE DATA

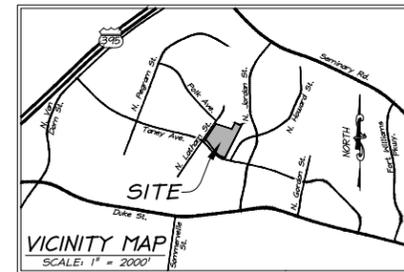
Table with 3 columns: Structure ID, Description, and Elevation/Notes. Includes entries like '28.21 LF OF 12" CMP', '56.81 LF OF 18" RCP', '45.05 LF OF 18" RCP', etc.

SANITARY STRUCTURE DATA

Table with 3 columns: Structure ID, Description, and Elevation/Notes. Includes entries like '172.57 LF OF 8" DIP', '43.65 LF OF 10" DIP', '350.51 LF OF 10" DIP', etc.

SCHEDULE OF TREES

Table with 4 columns: Tree ID, Species, Diameter, and Location. Lists various trees such as '001 Q. SP. 12" DBH', '002 Q. SP. 10" DBH', etc.



SURVEYOR'S CERTIFICATION

THIS BOUNDARY AND TOPOGRAPHIC SURVEY ON THE LANDS OF THE CITY OF ALEXANDRIA WAS COMPLETED UNDER THE DIRECT AND RESPONSIBLE CHARGE OF BRENT E. EVANS FROM AN ACTUAL GROUND SURVEY MADE UNDER MY SUPERVISION BETWEEN THE DATES OF NOVEMBER 25, 2014 AND FEBRUARY 3, 2015 AND THAT THIS PLAT MEETS MINIMUM ACCURACY STANDARDS UNLESS OTHERWISE NOTED.

GIVEN UNDER THIS 19th DAY OF FEBRUARY 3, 2015



BRENT E. EVANS, L.S. COMMONWEALTH OF VIRGINIA LICENSED LAND SURVEYOR NO. 2843

christopher consultants engineering - surveying - land planning 9417 innovation drive, manassas, virginia 20110 703-583-8887 fax 703-583-8076



BOUNDARY & TOPOGRAPHIC SURVEY ON THE LANDS OF THE CITY OF ALEXANDRIA "PATRICK HENRY ELEMENTARY SCHOOL"

Table with 3 columns: Rev#, DATE, REVISION. Includes entries for Rev# 1 on 02/09/15 with revision 'ADDED PUNCH LIST ITEMS'.

LEGEND

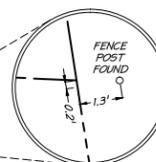
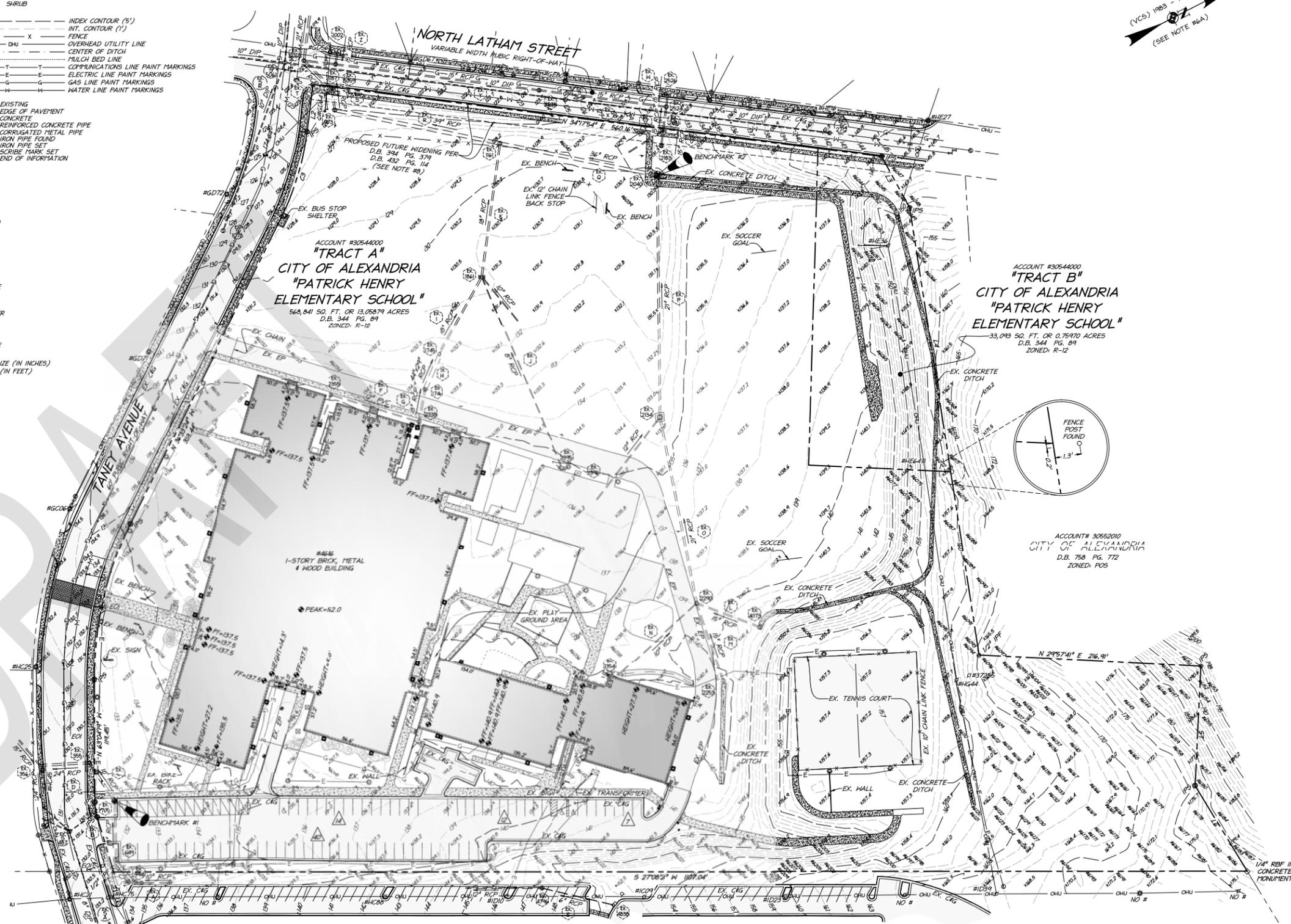
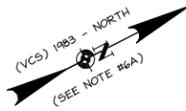
- Utilities - Storm
 - STORM MANHOLE
 - STORM CATCH BASIN
 - ROOF DRAIN
- Utilities - Sanitary
 - SANITARY MANHOLE
- Utilities - Electric
 - ELECTRIC BOX
 - ELECTRIC MANHOLE
 - LIGHT POLE
 - UTILITY METER
 - GUY WIRE
 - ELECTRIC METER
- Utilities - Water
 - WATER METER
 - WATER VALVE
 - FIRE HYDRANT
- Utilities - Gas
 - GAS VALVE
 - GAS METER
 - GAS TEST HOLE
- Utilities - Communication
 - COMMUNICATION PEDESTAL
 - COMMUNICATION VAULT
- Surfaces
 - ASPHALT AREA
 - CONCRETE AREA
 - GRAVEL AREA
- Structures
 - INTERPOLATED SPOT SHOT
 - SIGN
 - BOLLARD
 - UNKNOWN UTILITY STUB
 - FLAG POLE
 - NUMBER OF PARKING SPACES
 - SHRUB
- Linetypes
 - INDEX CONTOUR (5')
 - INT. CONTOUR (1')
 - FENCE
 - OVERHEAD UTILITY LINE
 - CENTER OF DITCH
 - MULCH BED LINE
 - COMMUNICATIONS LINE PAINT MARKINGS
 - ELECTRIC LINE PAINT MARKINGS
 - GAS LINE PAINT MARKINGS
 - WATER LINE PAINT MARKINGS
- Abbreviations
 - EX. EXISTING
 - EP. EDGE OF PAVEMENT
 - CONC. CONCRETE
 - RCP. REINFORCED CONCRETE PIPE
 - CMP. CORRUGATED METAL PIPE
 - IPF. IRON PIPE FOUND
 - IPS. IRON PIPE SET
 - SMS. SCRIBE MARK SET
 - END. END OF INFORMATION

TREE LEGEND

- DECIDUOUS TREE
- CONIFEROUS TREE
- TREE TAG NUMBER
- DT DECIDUOUS TREE
- CT CONIFEROUS TREE
- CT SZ 15/7 TRUNK SIZE (IN INCHES)
- TREE TYPE
- TREE COVER (IN FEET)

BENCHMARKS

- BENCHMARK #1 BENCHMARK #2
- STORM MANHOLE STORM MANHOLE
- N: 4,983,252.85 N: 4,984,002.45
- E: 11,878,557.94 E: 11,878,257.27
- ELEVATION=132.09 ELEVATION=130.44



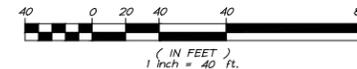
LINE TABLE

LINE	BEARING	DISTANCE
L1	N 79°55'49" W	55.42'
L2	N 55°40'19" N	46.32'
L3	N 89°03'54" E	57.35'
L4	S 114°01'29" E	440.60'
L5	S 60°02'19" E	116.62'
L6	N 85°42'41" E	97.78'

CURVE TABLE

CURVE	RADIUS	LENGTH	DELTA	TANGENT	BEARING	CHORD
C1	50.00'	14.71'	16°30'24"	7.41'	S 71°30'24" E	14.64'
C2	340.00'	157.34'	26°30'55"	80.11'	S 49°48'52" E	155.94'
C3	260.00'	86.74'	19°06'55"	43.78'	N 46°06'52" W	86.34'

GRAPHIC SCALE



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BOUNDARY & TOPOGRAPHIC SURVEY ON THE LANDS OF
THE CITY OF ALEXANDRIA
"PATRICK HENRY ELEMENTARY SCHOOL"

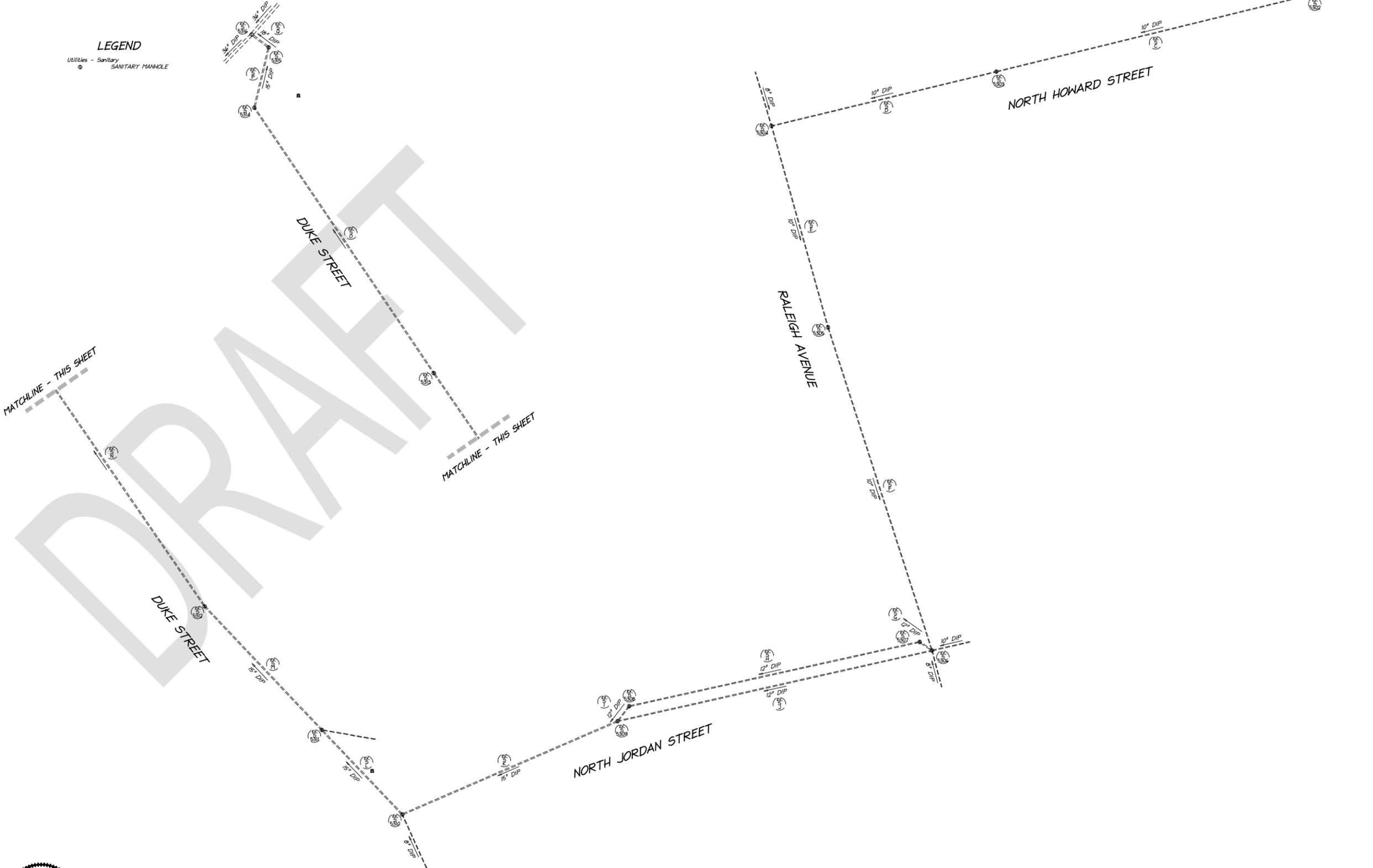
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 SCALE: 1" = 40'
 DATE: 01/16/15
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 SHEET No.

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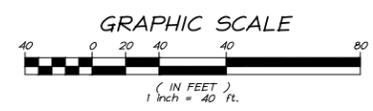
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○ SANITARY MANHOLE



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MATCHLINE - THIS SHEET

MATCHLINE - SHEET 2



PROJECT: 102676.022.00
SCALE: 1" = 40'
DATE: 01/16/15
DRAWN: WEB
CHECKED: BEE

SHEET No.
3 OF 3

BOUNDARY & TOPOGRAPHIC SURVEY ON THE LANDS OF
THE CITY OF ALEXANDRIA
"PATRICK HENRY ELEMENTARY SCHOOL"
(DEED BOOK 344 PAGE 89)
CITY OF ALEXANDRIA, VIRGINIA



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REPORT OF
SUBSURFACE EXPLORATION AND
GEOTECHNICAL ENGINEERING AND ANALYSIS
PATRICK HENRY ELEMENTARY SCHOOL
ALEXANDRIA, VIRGINIA

FOR
SORG ARCHITECTS

MARCH 26, 2015



March 26, 2015

Mr. Bill Conkey, AIA
Sorg Architects
918 U Street NW
Washington, DC 20001

ECS Project No. 01:24129

Reference: Report of Subsurface Exploration and Geotechnical Engineering Analysis,
Patrick Henry Elementary School, 4643 Taney Ave, Alexandria, VA, 22304

Dear Mr. Conkey:

As authorized by your acceptance of our Proposal No. 49337-GP, dated October 29, 2014, ECS Mid-Atlantic, LLC (ECS) has completed the subsurface exploration for the proposed addition to Patrick Henry Elementary School in Alexandria, Virginia. The enclosed report discusses the subsurface exploration procedures as well as the results of our subsurface exploration and laboratory testing programs, and presents our recommendations for the design and construction of the proposed structure. A Boring Location Diagram is included in the Appendix of this report, along with boring logs and laboratory test results

We appreciate the opportunity to be of service to you on this project. If you have any questions regarding the information and recommendations contained in the accompanying report, please do not hesitate to contact us.

Respectfully,

ECS MID-ATLANTIC, LLC

Andy Tao, E.I.T.
Staff Project Engineer

Bryan C. Layman, P.E.
Principal Engineer

James P. Racine, P.E.
Senior Project Engineer

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REPORT

PROJECT

Subsurface Exploration and
Geotechnical Engineering Analysis for
Patrick Henry Elementary School
4643 Taney Ave, Alexandria, VA, 22304

CLIENT

Sorg Architects
918 U Street NW
Washington, DC 20001

PROJECT NO.	01:24129-GP
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DATE	March 26, 2015
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PROJECT OVERVIEW

Introduction

This report presents the results of our subsurface exploration and geotechnical engineering analysis for the proposed addition to Patrick Henry Elementary School located at 4643 Taney Avenue in Alexandria, Virginia. The study was conducted in general accordance with Proposal No. 01:49337-GP, dated October 29, 2014. The site location and the approximate boring locations are shown on the Boring Location Diagram included in the Appendix of this report.

Site Description and Proposed Construction

The project site is located at 4643 Taney Avenue in Alexandria, Virginia and is bound by Taney Avenue to the south, North Latham Street to the west, wooded area to the north, and residential housing to the east. Based on our review of the provided site plan dated, March 9, 2015, it appears that the site generally slopes from the northeast towards the southwest with the existing grades ranging from EL. +140 to EL. +127 feet. The site consists of the existing Patrick Henry school building, which is on-grade with one level of above-grade space. Surrounding the school are several grassed areas and an asphalt parking lot. The existing building appears to have been constructed in several phases and additions.

We understand that the proposed project will consist of a large renovation for the Patrick Henry Elementary School, which may consist of a new building and/or renovation of the existing structures. Conceptual site plans of renovations or new building options were provided by Sorg Architects, but a final site plan was not available. The conceptual site plans include four different layouts, which can be found in the Appendix. The boring locations were selected to accommodate these layouts. For the purposes of this report, we have assumed that the new additions/renovations are on-grade and contain up to 2-levels of above-grade space. No below-grade space is anticipated.

Proposed Foundations

ECS has not been provided with any preliminary structural drawings at this time. Based on our review of the provided conceptual drawings, we have assumed that the building will be on-grade and supported by shallow spread foundations. Estimated loading has not been provided. Therefore, we have assumed that the building is relatively lightly loaded and will have maximum column loading on the order of 125 kips and wall loading on the order of 5 kips per linear foot.

If any of this information is in error, either due to our misunderstanding or due to any design changes that may occur later, ECS should be contacted so that we may review our recommendations and provide alternate or additional recommendations at that time.

Purpose and Scope of Work

The purpose of this exploration was to explore the subsurface conditions at the site and to develop engineering recommendations to guide the design and construction of the project. We accomplished these purposes by performing the following scope of services:

1. drilling borings to explore subsurface soil and groundwater conditions,
2. performing laboratory tests on selected representative soil samples from the borings to evaluate pertinent engineering properties,
3. analyzing the field and laboratory data to develop appropriate engineering recommendations, and
4. preparing this geotechnical report of our findings and recommendations.

The recommendations presented in this report are based on the results of our field subsurface exploration, laboratory testing, and review of available geological and/or geotechnical data. A total of eight borings (Borings B-1 to B-8) were performed by ECS.

The results of the completed soil borings along with a Boring Location Diagram are included in the Appendix of this report. The Boring Location Diagram was prepared based on the provided site plan, dated March 9, 2015. The borings were located in the field by representatives of ECS by pacing from existing structures. The site plan was utilized to determine the ground surface elevations noted on the attached boring logs.

Following drilling operations laboratory tests were performed on selected soil samples to identify the soils and to assist in determination of the properties of the site soils. The results of the laboratory testing are included in the Appendix of this report and are also noted on the boring logs.

EXPLORATION PROCEDURES

Subsurface Exploration Procedures

The soil borings were performed with an ATV-mounted auger drill rig, which utilized continuous flight, hollow-stem augers to advance the boreholes. In hollow-stem auger drilling operations, drilling fluid is not typically used to maintain or advance the borings. After the completion of each boring, the boreholes were backfilled with the spoils generated during drilling and the excess spoils were removed off site.

Representative soil samples were obtained by means of the split-barrel sampling procedure in accordance with ASTM Specifications D1586. In the split-barrel sampling procedure, a 2-inch O.D., split-barrel sampler is driven into the soil a distance of 18 inches by means of a 140-pound hammer falling 30 inches. The number of blows required to drive the sampler through a 12-inch interval is termed the Standard Penetration Test (SPT) "N" value and is indicated for each sample on the boring logs. This value can be used to provide a qualitative indication of the in-place relative density of cohesionless soils. In a less reliable way, it also indicates the consistency of cohesive soils. This indication is qualitative, since many factors can significantly affect the SPT value and prevent a direct correlation between drill crews, drill rigs, drilling procedures, and hammer-rod-sampler assemblies.

A field log of the soils encountered in the borings was maintained by the drill crew. After recovery, each sample was removed from the sampler and visually classified. Representative portions of each sample were then sealed and brought to our laboratory in Chantilly, Virginia for further visual examination and laboratory testing.

Laboratory Testing Program

Representative soil samples were selected and tested in our laboratory to determine pertinent engineering properties and soil classification. The laboratory testing program included visual classifications, natural moisture content tests, Atterberg Limits tests, and washed sieve analyses. All data obtained from the laboratory testing program is included on the respective boring logs and on the laboratory sheets within the Appendix of this report.

Each soil sample was visually classified on the basis of texture and plasticity in accordance with the Unified Soil Classification System (USCS). The group symbols for each soil type are indicated in parentheses following the soil descriptions on the boring logs. A brief explanation of the USCS will be included with the boring logs. The various soil types were grouped into the major zones noted on the boring logs. The stratification lines designating the interface between earth materials on the boring logs and profiles are approximate. In situ, the transitions between these strata may be gradual.

The soil samples from our exploration will be retained in our laboratory for a period of 60 days, after which they will be discarded unless other instructions are received as to their disposition.

EXPLORATION RESULTS

Regional Geology

The proposed site is located in the Coastal Plain Physiographic Province of Virginia. This Coastal Plain Province is characterized by a series of south-easterly dipping layers of relatively consolidated sandy clay deposits, with lesser amounts of gravel. These coastal Plain deposits are estimated to be approximately 250 feet thick and are underlain by the eastward continuation of the crystalline rock of the Piedmont Physiographic Province.

In general the higher elevations of the site area have few remnants of the Quaternary Age River Terrace deposits. The Quaternary Age Deposits are typically underlain, by the Potomac Group sediments of the older Cretaceous Age. The Cretaceous Age Potomac Group deposits generally consist of interbedded, layers of sand, silt, clay and gravel layers. The sand layers generally consist of fine to medium sand with variable amounts of clay and silt. In isolated areas, gravel can also be encountered.

Although not encountered during this exploration, the clay layers of the Potomac Group are commonly referred to as "marine clay" and it is generally believed that they were deposited in a deltaic environment. These very stiff to hard clays are often moderately to highly over consolidated and have a blocky structure.

Soil Conditions

The descriptions of the soil conditions encountered at the site are based on samples obtained from eight soil borings (B-1 to B-8). The borings were extended to depths on the order of 25 feet.

Stratum I – Fill Materials

Topsoil material up to 6 to 7 inches was observed in most of the borings, with the exception of B-3, which had 18 inches of topsoil material. Beneath the topsoil layer, existing fill materials were encountered to a depth of approximately 2.0 to 2.5 ± feet below the existing site grades. The fill material generally consisted of Lean CLAY (CL) with varying amounts of sand and root fragments. SPT N-Values ranged from 4 bpf to 12 bpf, which indicated soft to stiff consistencies.

Stratum II – Alluvial Soils

The soils underlying the stratum I material were observed to be consistent with the local geology. The soils encountered below the fill material generally consisted of SANDS (SP and SC) with varying amounts of clay and Lean CLAY (CL) with varying amounts of sand. The fine grained soils encountered exhibited SPT N-values ranging from 5 bpf to 30 bpf, which indicate medium stiff to very stiff consistencies. The granular materials exhibited SPT N-values ranging from 8 bpf to 28 bpf, which indicate loose to medium dense relative densities. The stratum II material was generally observed to the end of the boring depths.

Groundwater Conditions

Groundwater was not observed in any of the borings (B-1 to B-8) during drilling or before or after pulling augers. In auger drilling operations, water is not introduced into the boreholes, and the groundwater position can often be determined by observing water flowing into or out of the boreholes. Furthermore, visual observation of the soil samples retrieved during the auger drilling exploration can often be used in evaluating the groundwater conditions.

The groundwater table may undergo seasonal variations in elevation on the order of 10± feet. Generally, variations in the location of the water tables can occur at the site as a result of changes in precipitation, evaporation, surface water runoff, pumping and other factors not immediately apparent at the time of this exploration. However, perched water tables are also common at the interface of fill and natural soils.

DRAFT

ANALYSIS AND RECOMMENDATIONS

Based on the subsurface conditions encountered in the borings and on our experience in the project area, it appears the site is suited for the proposed structure from a geotechnical perspective. The conclusions and recommendations presented in this report should be incorporated in the design and construction of the project to minimize possible soil and/or foundation related problems.

The following sections present more detailed recommendations with regard to the support of the proposed structure. These include recommendations with regard to foundations, earthwork, and subgrade preparation. Discussion of the factors affecting the foundation for the proposed construction, as well as additional recommendations regarding design and construction at the project site are included below. We recommend that ECS review the final design and specifications to check that the earthwork and foundation recommendations presented in this report have been properly interpreted and implemented in the design and specifications.

Shallow Foundations – Proposed

For foundations bearing on natural soils approximately 2.5 feet below existing grades, we recommend an allowable bearing capacity of 4,000 psf. Suitable natural materials adequate to support the 4,000 psf bearing capacity can be found on the boring logs as those with a minimum Standard Penetration Test (SPT) N-value of 10 bpf or denser and classified Clayey SAND (SC), Poorly-Graded SAND (SP), and Lean CLAY (CL), each with varying amounts of sand, gravel, and clay. A minimum embedment depth of 2.5 feet is required (measured from the finished floor elevation to the bottom of footing elevation). We anticipate in some areas that the existing grades may be raised. For new, shallow footings bearing on approved, suitable, and properly compacted fill material, an allowable bearing pressure of 4 ksf may also be used.

We emphasize the need for verifying the suitability of footing subgrades during construction. The bearing pressure should be checked in the field by the geotechnical engineer of record.

General Shallow Foundation Recommendations

We emphasize the need for verifying the suitability of footing subgrades during construction. The bearing pressure should be checked in the field by the geotechnical engineer of record. Footings should be excavated, tested, and poured the same day. In the event the footing cannot be poured the same day, we recommend that the bearing surface be covered with a 3 to 4 inch lean concrete mud mat.

Settlement of a structure is a function of the bearing pressure and column loads. If our recommendations for shallow foundations are strictly followed, we expect the maximum total settlement of the footings to be less than one inch. Differential settlement between adjacent columns in the same structure is expected to be half this value. These settlement values are based on our analysis and engineering experience of the subsurface conditions and the anticipated structural loading, and are to guide the structural engineer with their design.

Recommendations included in this report apply only to development of the site at the above referenced bearing elevations. Should foundation bearing levels differ significantly from the anticipated elevations, ECS should be retained to modify the provided recommendations. Additionally, if loading conditions should change significantly, the recommendations in this report will not be suitable for support of the proposed development. In these cases, ECS should be provided the changes for our review.

Floor Slab Design

Based on our analysis of the on-site surficial soils, floor slabs on-grade are feasible for the proposed development. We recommend that unsuitable materials be removed from these subgrade areas once they are exposed. The floor slab area should be proofrolled with a loaded tandem axle dump truck with a weight not less than 10 tons and observed by an experienced soil technician during the time of construction in order to aid in locating all such unsuitable materials which should be removed.

Where new fill material is required to reach the design floor slab subgrade elevation, it is recommended that an approved inorganic material, with LL less than 40 and PI less than 20 and free of debris be used. This material should be placed in lifts not exceeding 8 inches in loose thickness, moisture conditioned to within ± 2 percentage points of the optimum moisture content and compacted to a minimum of 95% of the maximum density obtained in accordance with ASTM D698, Standard Proctor.

We recommend that the floor slab be isolated from the foundation footings so that differential settlement of the structure will not induce shear stresses in the floor slab. Also, in order to reduce the crack width of any shrinkage cracks that may develop near the surface of the slab, we recommend mesh reinforcement be used. The mesh should be in the top half of the slab to be effective.

We also recommend the building slabs on grade be underlain by a minimum of 6 inches of granular material having a maximum aggregate size of 1.5 inches and no more than 2% passing the #200 sieve. This granular layer will facilitate the fine grading of the subgrade and help prevent the rise of water through the floor slab. Prior to placing the granular material, the floor subgrade soil should be properly compacted, proofrolled, and free of standing water, mud, and frozen soil. Before the placement of concrete, a vapor barrier may be placed on top of the granular material to provide additional moisture protection. However, special attention should be given to the surface curing of the slab in order to reduce uneven drying of the slab and associated cracking.

Underslab Subdrainage

As no below-grade space is planned, we recommend that an exterior, perimeter foundation drain be installed. The drain should be a minimum 4-inch slotted PVC pipe encapsulated (all around) in 6 inches of clean gravel wrapped in filter fabric. The drain may rest on the exterior footings and should daylight to a suitable outlet.

Site Retaining Walls

We do not anticipate any site retaining walls at this time. If these walls are required, ECS should be provided with the wall details and locations we can provide specific wall recommendations.

Seismic Design Considerations

The International Building Code (IBC) 2012 requires site classification for seismic design based on the upper 100 feet of a soil profile. Where site specific data are not available to a depth of 100 feet, appropriate soil properties are permitted to be estimated by the registered design professional preparing the soils report based on known geologic conditions.

Utilizing the data obtained from the on-site boring exploration and our previous experience at neighboring sites, a mean SPT “N”-value of less than 50 blows per foot (bpf) is anticipated within 100 feet of the ground surface. Three methods are utilized in classifying sites, namely the shear wave velocity (v_s) method; the unconfined compressive strength (s_u) method; and the Standard Penetration Resistance (N-value) method. The latter method (N-Value method) was used in classifying this site.

SITE CLASS	SOIL PROFILE NAME	AVERAGE PROPERTIES IN TOP 100 ft, AS PER SECTION 1613.5.2
		Standard Penetration Resistance, N-bar
A	Hard Rock	Not Applicable
B	Rock	Not Applicable
C	Very Dense Soil and Soft Rock	N-bar > 50
D	Stiff Soil Profile	15 ≤ N-bar ≤ 50
E	Soft Soil Profile	N-bar < 15
E	–	Any profile with more than 10 feet of soil having the following characteristics: 1. Plasticity Index, PI > 10 2. Moisture content, w ≥ 40% 3. Undrained shear strength, Su-bar < 500 psf
F	–	Any profile containing soils having one or more of the following characteristics: 1. Soils vulnerable to potential failure or collapse under seismic loading such as liquefiable soils, quick and highly sensitive clays, collapsible weakly cemented soils. 2. Peats and/or highly organic clays (H > 10 ft or peat and/or highly organic clay where H = thickness of soil) 3. Very high plasticity clays (H > 25 ft with plasticity index PI > 75) 4. Very thick soft/medium stiff clays (H > 120 ft)

Based on our interpretation of the IBC 2012 Building Code and Table 1613.5.2, the project is defined as “Site Class D” for seismic design considerations. The Site Class definition should not be confused with the Seismic Design Category designation, which the Structural Engineer typically assesses. If a higher site classification is beneficial to the project, ECS would be pleased to discuss additional testing capabilities in this regard.

PROJECT CONSTRUCTION

Subgrade Preparation and Earthwork Operations

The subgrade preparation should consist of removing any deleterious, soft, or unsuitable material from the proposed building areas as required in slab, footing, and wall areas. After excavating to the desired grade, and prior to fill placement (if required), the initial exposed subgrade for the foundation should be observed by the Geotechnical Engineer of Record or his authorized representative.

The preparation of fill subgrades should be observed on a full-time basis. These observations should also be performed by an experienced geotechnical engineer, or their representative, to document that all unsuitable materials have been removed, and that the subgrade is suitable for support of the proposed construction and/or fills.

After examining the exposed soils, loose and yielding areas can be identified by proofrolling, probing, or testing. In the event that any loose natural soils are encountered during the operations, the subgrade should be either densified in-place, if deemed appropriate in the field by the geotechnical engineer, or undercut to firm ground and replaced with approved controlled fill compacted to the criteria given in the section below entitled Fill Placement. We recommend that an authorized representative of the Geotechnical Engineer of Record be present on-site working with the contractor to document the necessary depths of undercut.

If any problems are encountered during the earthwork operations, or if site conditions deviate from those encountered during our subsurface exploration, the Geotechnical Engineer should be notified immediately.

Fill Placement

All fills should consist of an approved material, free of organic matter and debris, cobbles greater than 4-inches and have a Liquid Limit and Plasticity Index less than 40 and 20, respectively. Unacceptable fill materials include topsoil and organic materials (OH, OL), and high plasticity silts and clays (CH, MH). Under no circumstances should high plasticity soils be used as fill material in proposed structural areas or close to site slopes.

The on-site materials classifying as (SC), (SP), and (CL) appear to be suitable for reuse as fill as detailed herein; however they will likely require moisture content adjustments. The planning of earthwork operations should recognize and account for these efforts and increased costs.

Fill materials should be placed in lifts not exceeding 8-inches in loose thickness and moisture conditioned to within ± 2 percentage points of the optimum moisture content. Soil bridging lifts should not be used, since excessive settlement of overlying structures will likely occur. Controlled fill soils should be compacted to a minimum of 95% of the maximum dry density obtained in accordance with ASTM D698, Standard Proctor. However, the upper one foot of soil supporting pavements, slabs, sidewalks, or gutters should be compacted to a minimum of 98% of the maximum dry density obtained in accordance with ASTM D698, Standard Proctor.

All fill operations should be observed on a full-time basis by a qualified soil technician to determine that the specified compaction requirements are being met. A minimum of one compaction test per 2,500 square foot area should be tested in each lift placed. The elevation and location of the tests should be clearly identified at the time of fill placement.

Compaction equipment suitable to the soil type used as fill should be used to compact the fill material. Theoretically, any equipment type can be used as long as the required density is achieved. Ideally, a steel drum roller would be most efficient for compacting and sealing the surface soils. All areas receiving fill should be graded to facilitate positive drainage from building pad and pavement areas of any free water associated with precipitation and surface runoff.

It should be noted that prior to the commencement of fill operations and/or utilization of any off-site borrow materials, the Geotechnical Engineer of Record should be provided with representative samples to determine the material's suitability for use in a controlled compacted fill and to develop moisture-density relationships. In order to expedite the earthwork operations, if off-site borrow materials are required, it is recommended they be comprised of a select granular material which will provide suitable support and be easily compacted and well drained.

The on-site materials may be reused, as appropriate, provided that they do not contain organic or foreign debris, are not high plasticity, are not environmentally impacted, and conform to the criteria outlined above. The suitability of any materials for use as engineered fill should be further evaluated at the time of construction.

Fill materials should not be placed on frozen soils or frost-heaved soils and/or soils which have been recently subjected to precipitation. All frozen soils should be removed prior to continuation of fill operations. Borrow fill materials, if required, should not contain frozen materials at the time of placement. All frost-heaved soils should be removed prior to placement of controlled, compacted fill, granular subbase materials, foundation or slab concrete, and asphalt pavement materials.

Construction Dewatering

Although significant excavations are not anticipated for this project, a system of trenching and sumping should be expected during foundation work, particularly during the rainy season. In addition, positive drainage should be utilized by the contractor in order to prevent rain water from running into and ponding in the site slab or footing areas. If proper runoff control is not in place, undercuts and construction delays should be expected. The French Drain Detail found in the appendix should be utilized when trenching and sumping is needed.

Closing

We recommend that if there are any changes to the project characteristics as outlined in this report, ECS is retained to review the plans and determine if modifications to the recommendations or if additional geotechnical recommendations are necessary for the proposed development. Once development details are finalized, this information should be provided to ECS in order to review our recommendations and finalize this report.

The foundation installation for the project site will be primary considerations during development and construction. We recommend that the Geotechnical Engineer of Record be retained to observe the foundation bearing surfaces and to verify the proposed design bearing pressures. All earthwork and structural renovations should be performed under the supervision of the Geotechnical Engineer of Record/ approved testing agency, or his authorized representative for compliance with the project contract drawings. ECS would be pleased to provide these services.

DRAFT

APPENDIX

Unified Soil Classification System

Reference Notes for Boring Logs

Borings Logs B-1 to B-8

Laboratory Testing Summary

Atterberg Limits Test Summary

Grains Size Test Summary

French Drain Installation Procedure

Boring Location Diagram

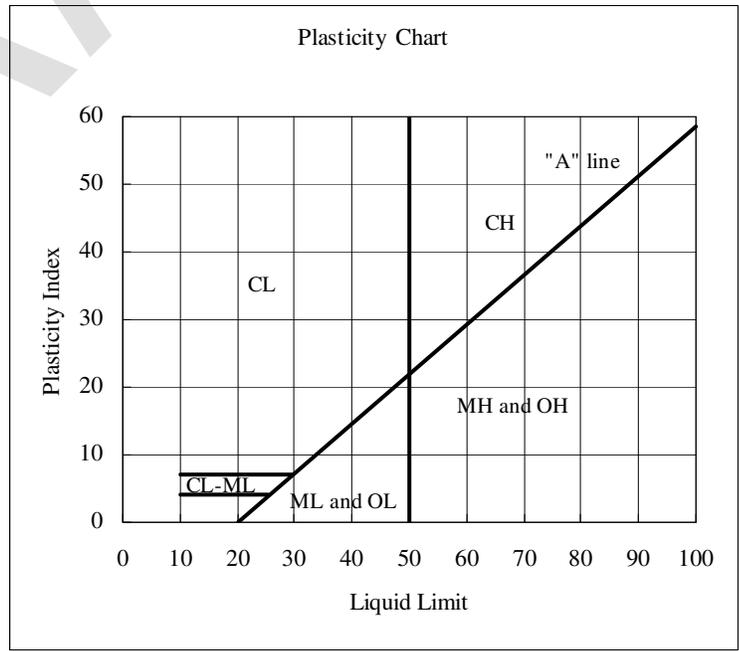
Cross Section A - A'

UNIFIED SOIL CLASSIFICATION SYSTEM (ASTM D 2487)

Major Divisions		Group Symbols	Typical Names	Laboratory Classification Criteria		
Coarse-grained soils (More than half of material is larger than No. 200 Sieve size)	Gravels (More than half of coarse fraction is larger than No. 4 sieve size)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	$C_u = D_{60}/D_{10}$ greater than 4 $C_c = (D_{30})^2/(D_{10} \times D_{60})$ between 1 and 3		
			GP			Poorly graded gravels, gravel-sand mixtures, little or no fines
		GM ^a	d	Silty gravels, gravel-sand mixtures	Atterberg limits below "A" line or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are borderline cases requiring use of dual symbols
			u			
	GC	Clayey gravels, gravel-sand-clay mixtures		Atterberg limits below "A" line or P.I. less than 7		
		Sands (More than half of coarse fraction is smaller than No. 4 sieve size)	SW	Well-graded sands, gravelly sands, little or no fines	$C_u = D_{60}/D_{10}$ greater than 6 $C_c = (D_{30})^2/(D_{10} \times D_{60})$ between 1 and 3	
	SP			Poorly graded sands, gravelly sands, little or no fines		
	SM ^a		d	Silty sands, sand-silt mixtures	Atterberg limits above "A" line or P.I. less than 4	Limits plotting in CL-ML zone with P.I. between 4 and 7 are borderline cases requiring use of dual symbols
		u				
	SC	Clayey sands, sand-clay mixtures		Atterberg limits above "A" line with P.I. greater than 7		

Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows:
 Less than 5 percent GW, GP, SW, SP
 More than 12 percent GM, GC, SM, SC
 Borderline cases requiring dual symbols ^b

Fine-grained soils (More than half material is smaller than No. 200 Sieve)	Silt and clays (Liquid limit less than 50)	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
		OL	Organic silts and organic silty clays of low plasticity
	Silt and clays (Liquid limit greater than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
		CH	Inorganic clays of high plasticity, fat clays
		OH	Organic clays of medium to high plasticity, organic silts
	Highly Organic soils	Pt	Peat and other highly organic soils



^a Division of GM and SM groups into subdivisions of d and u are for roads and airfields only. Subdivision is based on Atterberg limits; suffix d used when L.L. is 28 or less and the P.I. is 6 or less; the suffix u used when L.L. is greater than 28.

^b Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC, well-graded gravel-sand mixture with clay binder. (From Table 2.16 - Winterkorn and Fang, 1975)

REFERENCE NOTES FOR BORING LOGS

I. Drilling Sampling Symbols

SS	Split Spoon Sampler	ST	Shelby Tube Sampler
RC	Rock Core, NX, BX, AX	PM	Pressuremeter
DC	Dutch Cone Penetrometer	RD	Rock Bit Drilling
BS	Bulk Sample of Cuttings	PA	Power Auger (no sample)
HSA	Hollow Stem Auger	WS	Wash sample
REC	Rock Sample Recovery %	RQD	Rock Quality Designation %

II. Correlation of Penetration Resistances to Soil Properties

Standard Penetration (blows/ft) refers to the blows per foot of a 140 lb. hammer falling 30 inches on a 2-inch OD split-spoon sampler, as specified in ASTM D 1586. The blow count is commonly referred to as the N-value.

A. Non-Cohesive Soils (Silt, Sand, Gravel and Combinations)

<i>Density</i>		<i>Relative Properties</i>	
Under 4 blows/ft	Very Loose	Adjective Form	12% to 49%
5 to 10 blows/ft	Loose	With	5% to 12%
11 to 30 blows/ft	Medium Dense		
31 to 50 blows/ft	Dense		
Over 51 blows/ft	Very Dense		

<i>Particle Size Identification</i>		
Boulders		8 inches or larger
Cobbles		3 to 8 inches
Gravel	Coarse	1 to 3 inches
	Medium	½ to 1 inch
	Fine	¼ to ½ inch
Sand	Coarse	2.00 mm to ¼ inch (dia. of lead pencil)
	Medium	0.42 to 2.00 mm (dia. of broom straw)
	Fine	0.074 to 0.42 mm (dia. of human hair)
Silt and Clay		0.0 to 0.074 mm (particles cannot be seen)

B. Cohesive Soils (Clay, Silt, and Combinations)

<i>Blows/ft</i>	<i>Consistency</i>	<i>Unconfined Comp. Strength Q_p (tsf)</i>	<i>Degree of Plasticity</i>	<i>Plasticity Index</i>
Under 2	Very Soft	Under 0.25	None to slight	0 – 4
3 to 4	Soft	0.25-0.49	Slight	5 – 7
5 to 8	Medium Stiff	0.50-0.99	Medium	8 – 22
9 to 15	Stiff	1.00-1.99	High to Very High	Over 22
16 to 30	Very Stiff	2.00-3.00		
31 to 50	Hard	4.00–8.00		
Over 51	Very Hard	Over 8.00		

III. Water Level Measurement Symbols

WL	Water Level	BCR	Before Casing Removal	DCI	Dry Cave-In
WS	While Sampling	ACR	After Casing Removal	WCI	Wet Cave-In
WD	While Drilling	▽	Est. Groundwater Level	▽	Est. Seasonal High GWT

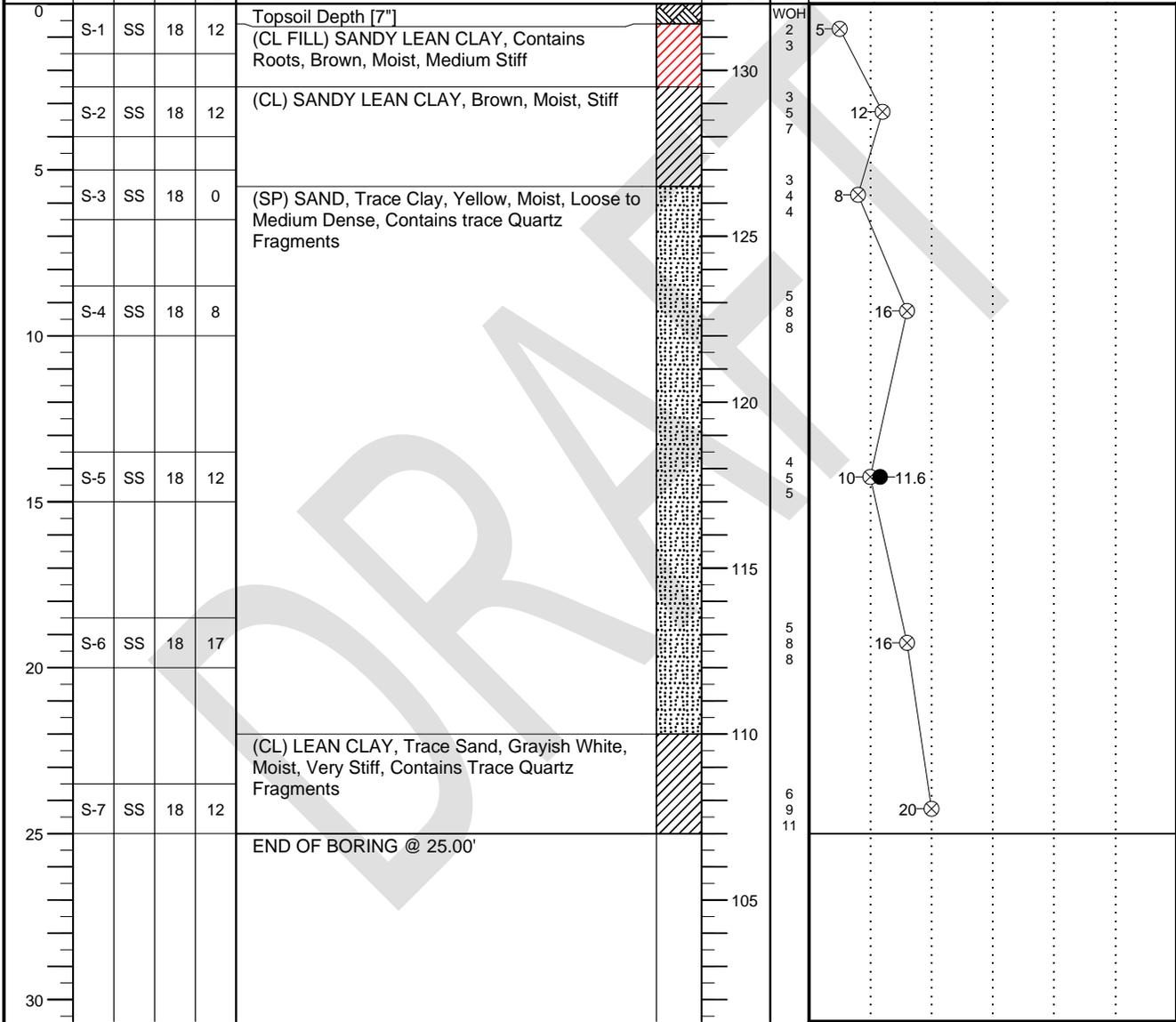
The water levels are those levels actually measured in the borehole at the times indicated by the symbol. The measurements are relatively reliable when augering, without adding fluids, in a granular soil. In clay and plastic silts, the accurate determination of water levels may require several days for the water level to stabilize. In such cases, additional methods of measurement are generally applied.

CLIENT Sorg & Associates	JOB # 24129	BORING # B-1	SHEET 1 OF 1	
PROJECT NAME Patrick Henry ES - Expansion/Renovation	ARCHITECT-ENGINEER			

SITE LOCATION
4643 Taney Avenue, Alexandria, City of Alexandria

NORTHING	EASTING	STATION
----------	---------	---------

DEPTH (FT)	SAMPLE NO.	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	ENGLISH UNITS	WATER LEVELS ELEVATION (FT)	BLOWS/6"	ROCK QUALITY DESIGNATION & RECOVERY		
									RQD% - - -	REC% - - -	
					BOTTOM OF CASING	LOSS OF CIRCULATION			PLASTIC LIMIT%	WATER CONTENT%	LIQUID LIMIT%
					SURFACE ELEVATION				X	●	△
									⊗ STANDARD PENETRATION BLOWS/FT		



THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL.

WL	WS	WD	BORING STARTED	03/09/15	
WL(BCR)	WL(ACR)		BORING COMPLETED	03/09/15	CAVE IN DEPTH @ 18.00'
WL			RIG 750 ATV	FOREMAN DAVID MCLEAN	DRILLING METHOD 2.25 HSA

CLIENT Sorg & Associates	JOB # 24129	BORING # B-2	SHEET 1 OF 1	
PROJECT NAME Patrick Henry ES - Expansion/Renovation	ARCHITECT-ENGINEER			

SITE LOCATION
4643 Taney Avenue, Alexandria, City of Alexandria

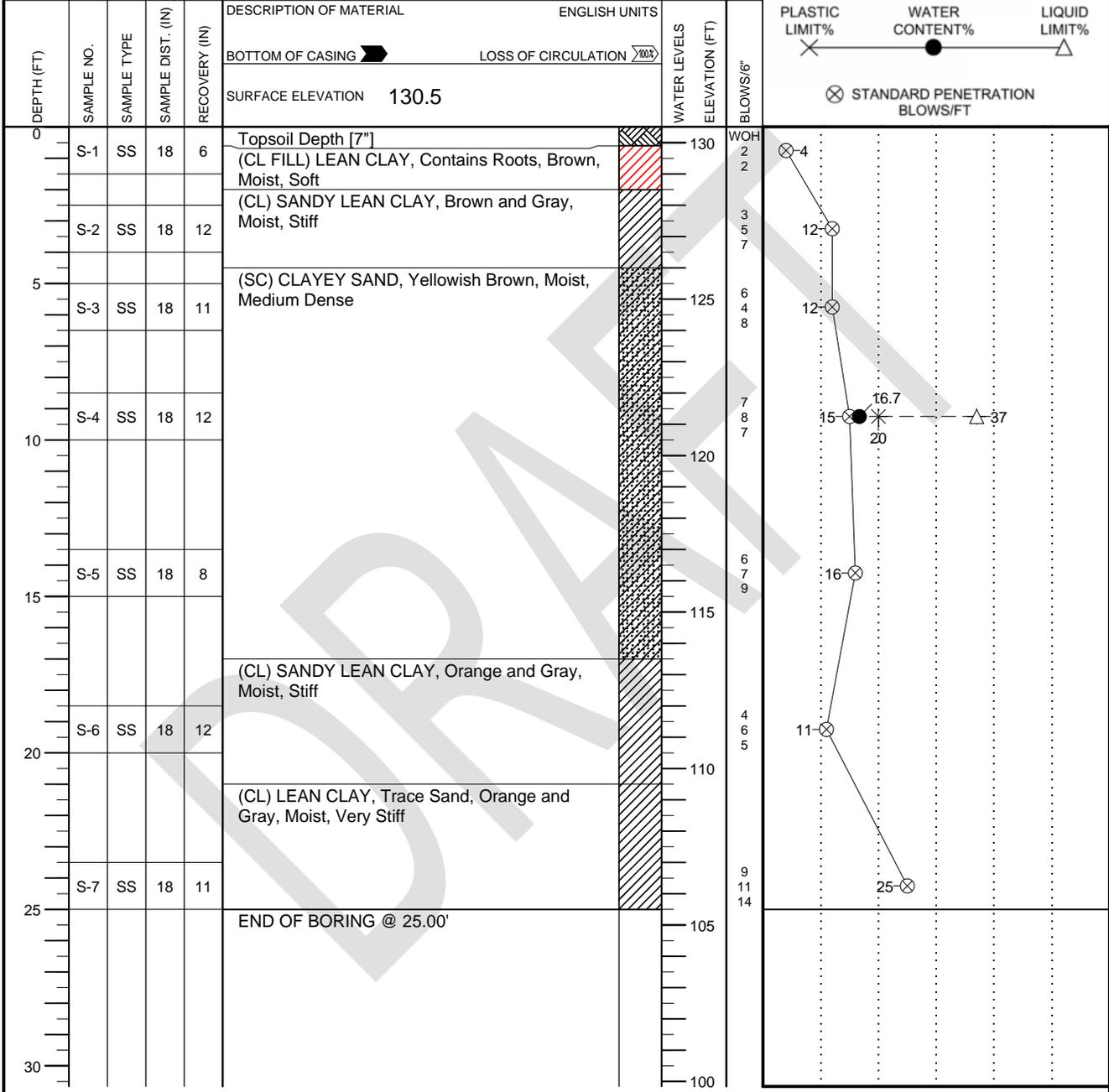
NORTHING	EASTING	STATION
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○ CALIBRATED PENETROMETER TONS/FT²

ROCK QUALITY DESIGNATION & RECOVERY
RQD% - - - REC% - - -

PLASTIC LIMIT% WATER CONTENT% LIQUID LIMIT%

⊗ STANDARD PENETRATION BLOWS/FT



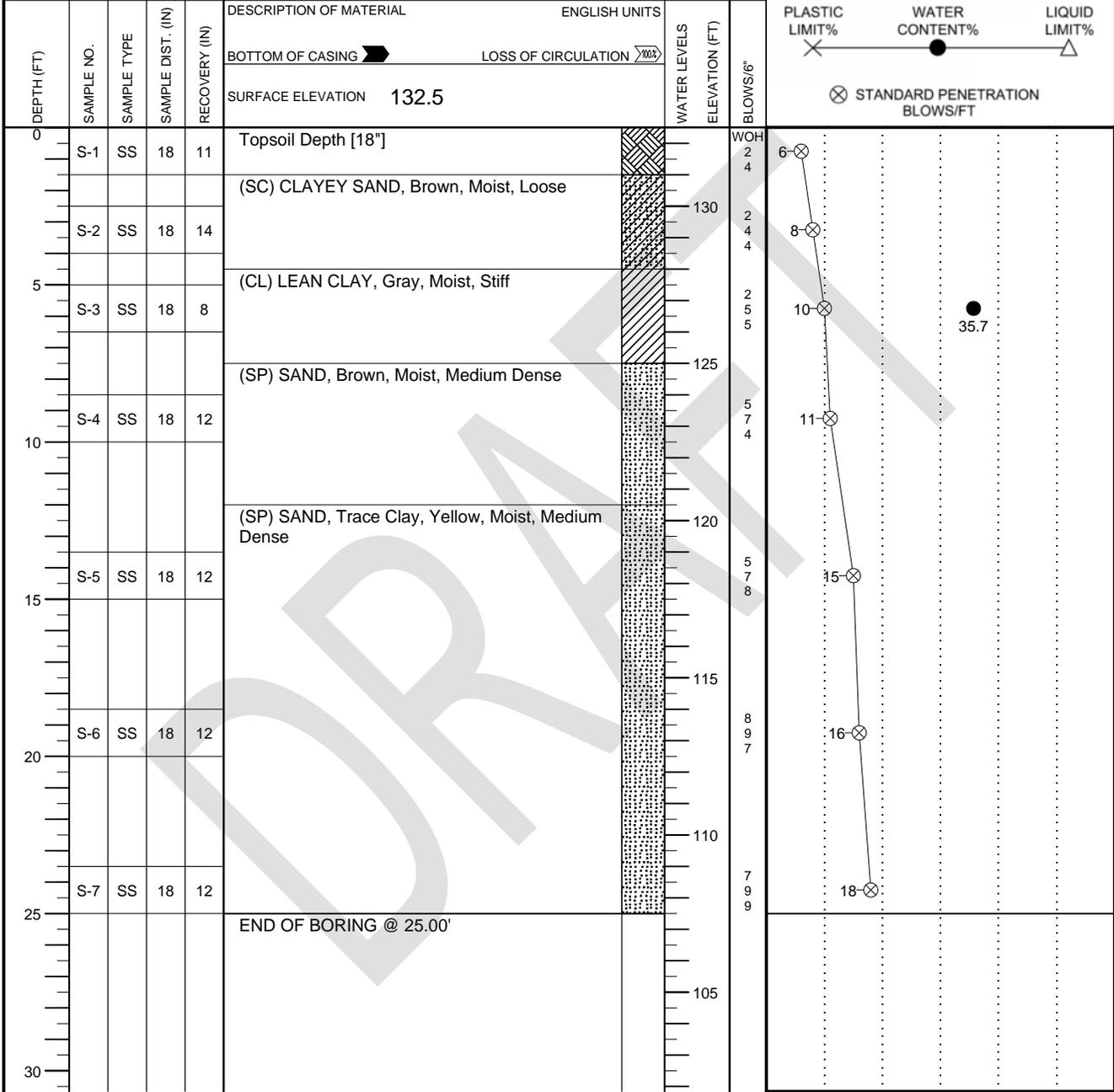
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL.

WL	WS <input type="checkbox"/>	WD <input type="checkbox"/>	BORING STARTED	03/09/15	
WL(BCR)	WL(ACR) <input checked="" type="checkbox"/>		BORING COMPLETED	03/09/15	CAVE IN DEPTH @ 17.50'
WL			RIG 750 ATV	FOREMAN DAVID MCLEAN	DRILLING METHOD 2.25 HSA

CLIENT Sorg & Associates	JOB # 24129	BORING # B-3	SHEET 1 OF 1	
PROJECT NAME Patrick Henry ES - Expansion/Renovation	ARCHITECT-ENGINEER			

SITE LOCATION
4643 Taney Avenue, Alexandria, City of Alexandria

NORTHING	EASTING	STATION
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THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL.

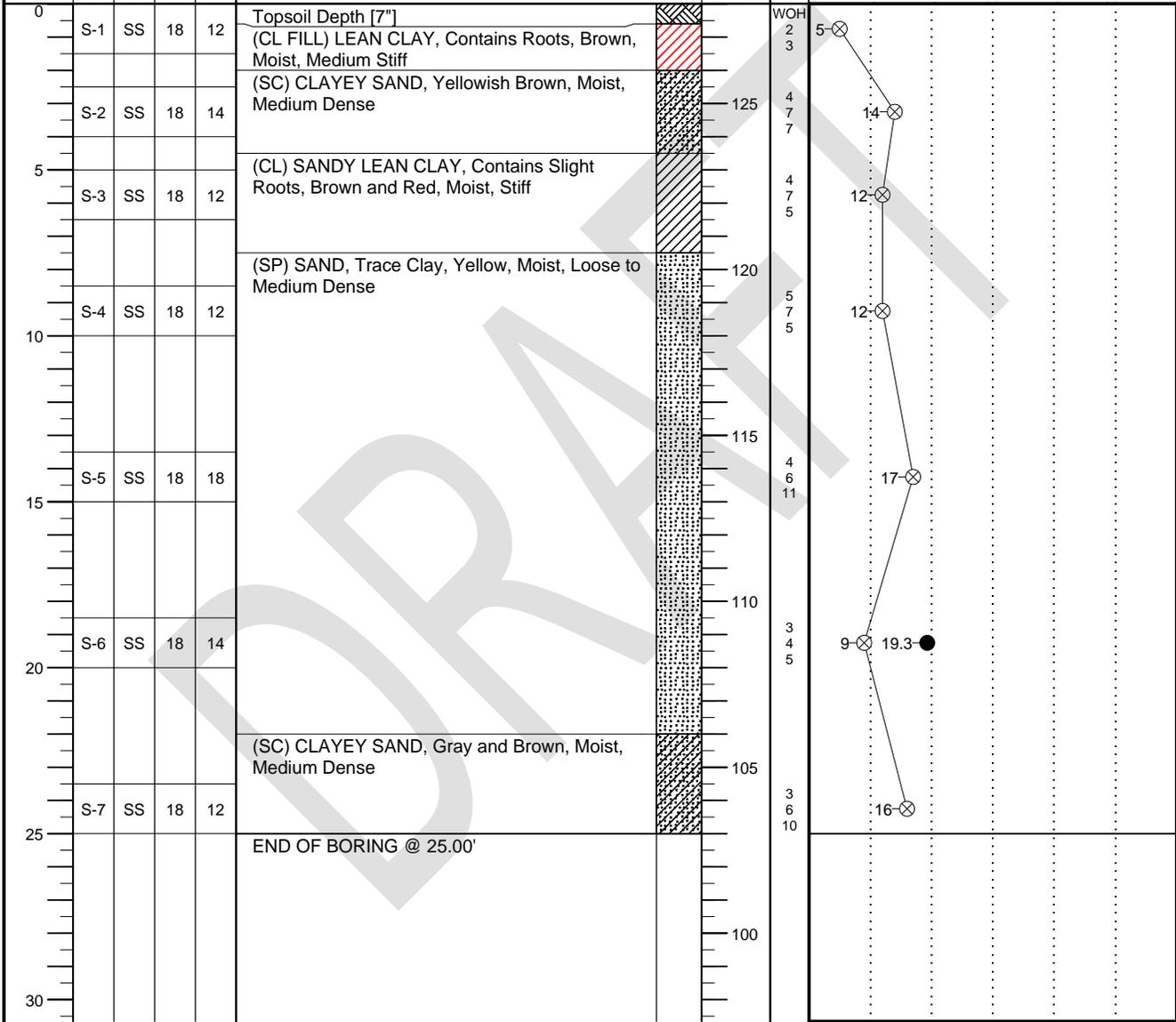
WL	WS	WD	BORING STARTED	03/09/15	
WL(BCR)	WL(ACR)		BORING COMPLETED	03/09/15	CAVE IN DEPTH @ 19.70'
WL			RIG 750 ATV	FOREMAN DAVID MCLEAN	DRILLING METHOD 2.25 HSA

CLIENT Sorg & Associates	JOB # 24129	BORING # B-4	SHEET 1 OF 1	
PROJECT NAME Patrick Henry ES - Expansion/Renovation	ARCHITECT-ENGINEER			

SITE LOCATION
4643 Taney Avenue, Alexandria, City of Alexandria

NORTHING	EASTING	STATION
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DEPTH (FT)	SAMPLE NO.	SAMPLE TYPE	SAMPLE DIST. (IN)	RECOVERY (IN)	DESCRIPTION OF MATERIAL	ENGLISH UNITS	WATER LEVELS	ELEVATION (FT)	BLOWS/6"
					BOTTOM OF CASING	LOSS OF CIRCULATION			
					SURFACE ELEVATION	128			



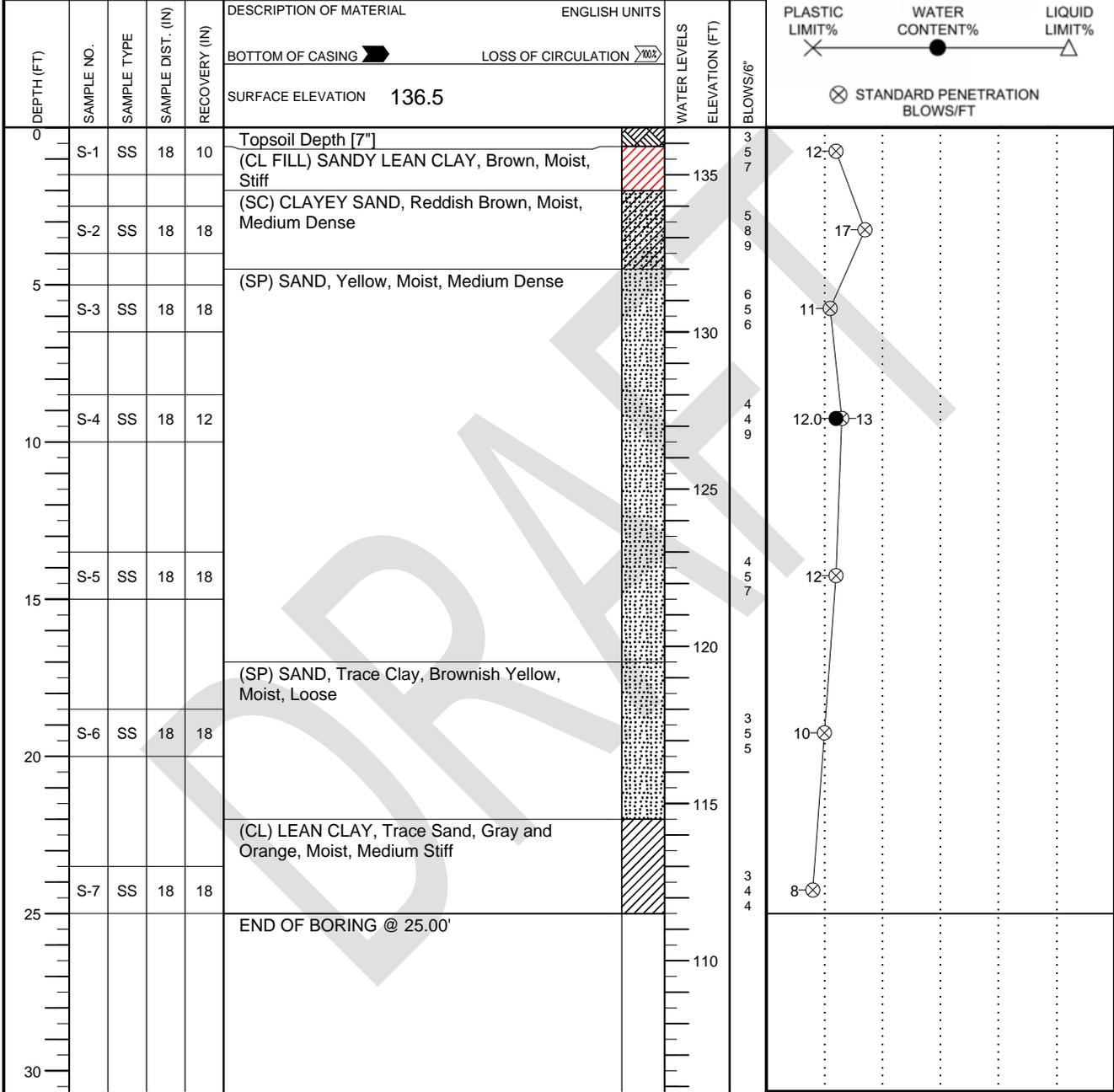
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL.

WL	WS <input type="checkbox"/>	WD <input type="checkbox"/>	BORING STARTED	03/09/15	
WL(BCR)	WL(ACR) <input type="checkbox"/>		BORING COMPLETED	03/09/15	CAVE IN DEPTH @ 18.00'
WL			RIG 750 ATV	FOREMAN DAVID MCLEAN	DRILLING METHOD 2.25 HSA

CLIENT Sorg & Associates	JOB # 24129	BORING # B-5	SHEET 1 OF 1	
PROJECT NAME Patrick Henry ES - Expansion/Renovation	ARCHITECT-ENGINEER			

SITE LOCATION
4643 Taney Avenue, Alexandria, City of Alexandria

NORTHING	EASTING	STATION
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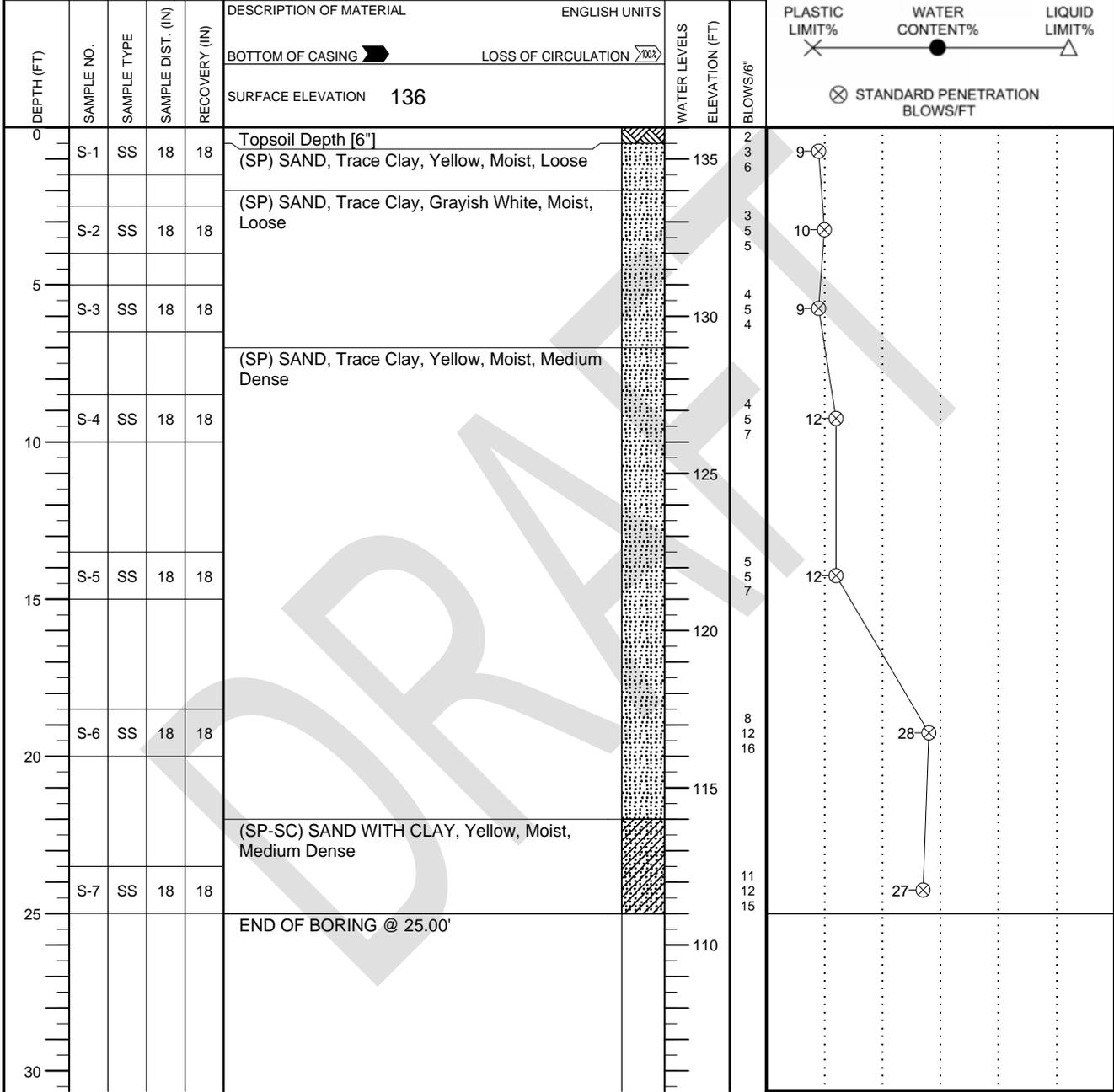
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WL	WS <input type="checkbox"/>	WD <input type="checkbox"/>	BORING STARTED	03/06/15	
WL(BCR)	WL(ACR) <input checked="" type="checkbox"/>		BORING COMPLETED	03/06/15	CAVE IN DEPTH @ 18.00'
WL			RIG 750 ATV	FOREMAN DAVID MCLEAN	DRILLING METHOD 2.25 HSA

CLIENT Sorg & Associates	JOB # 24129	BORING # B-6	SHEET 1 OF 1	
PROJECT NAME Patrick Henry ES - Expansion/Renovation	ARCHITECT-ENGINEER			

SITE LOCATION
4643 Taney Avenue, Alexandria, City of Alexandria

NORTHING	EASTING	STATION
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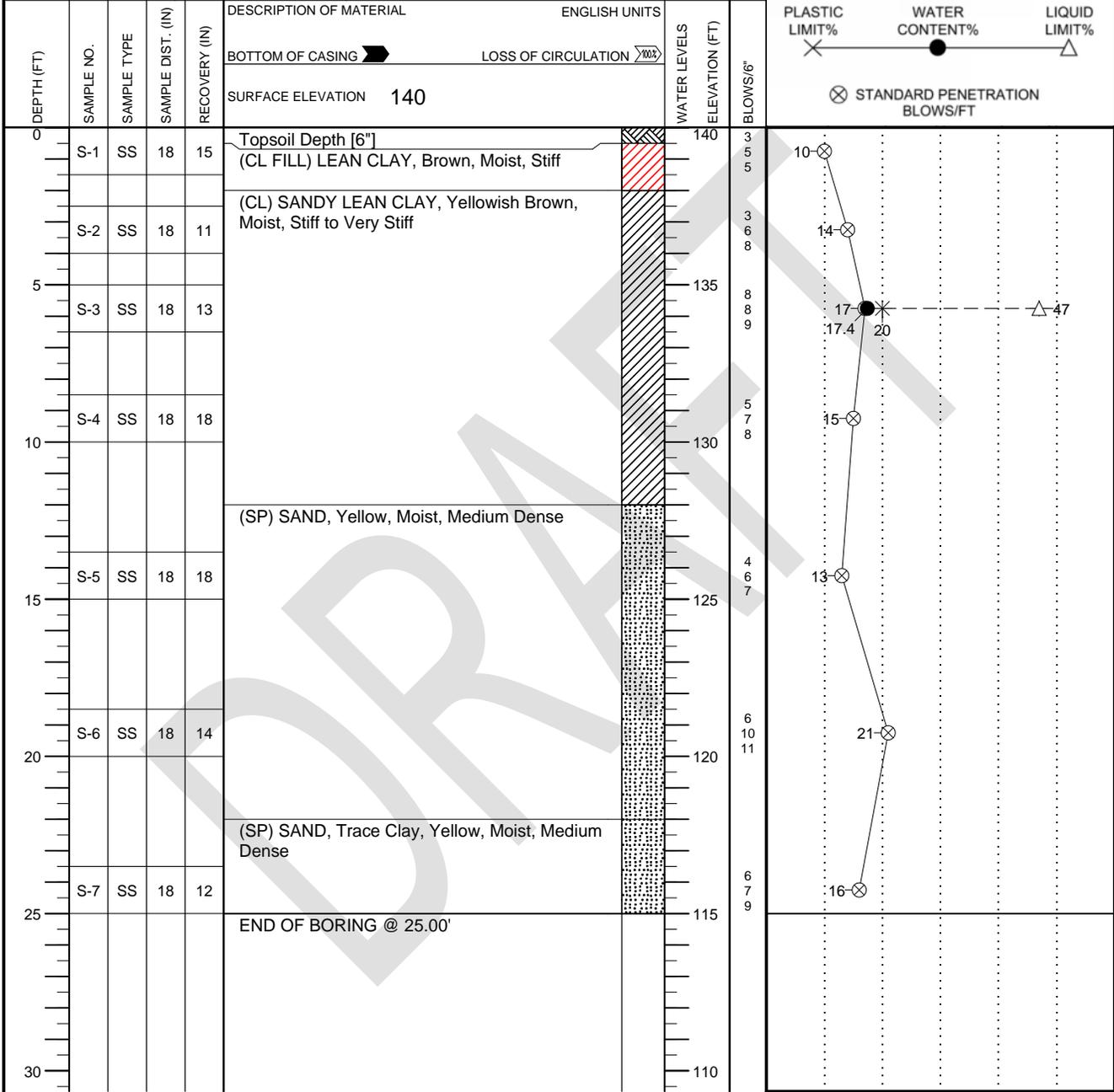
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WL	WS <input type="checkbox"/>	WD <input type="checkbox"/>	BORING STARTED	03/06/15	
WL(BCR)	WL(ACR)		BORING COMPLETED	03/06/15	CAVE IN DEPTH @ 18.00'
WL			RIG 750 ATV	FOREMAN DAVID MCLEAN	DRILLING METHOD 2.25 HSA

CLIENT Sorg & Associates	JOB # 24129	BORING # B-7	SHEET 1 OF 1	
PROJECT NAME Patrick Henry ES - Expansion/Renovation	ARCHITECT-ENGINEER			

SITE LOCATION
4643 Taney Avenue, Alexandria, City of Alexandria

NORTHING	EASTING	STATION
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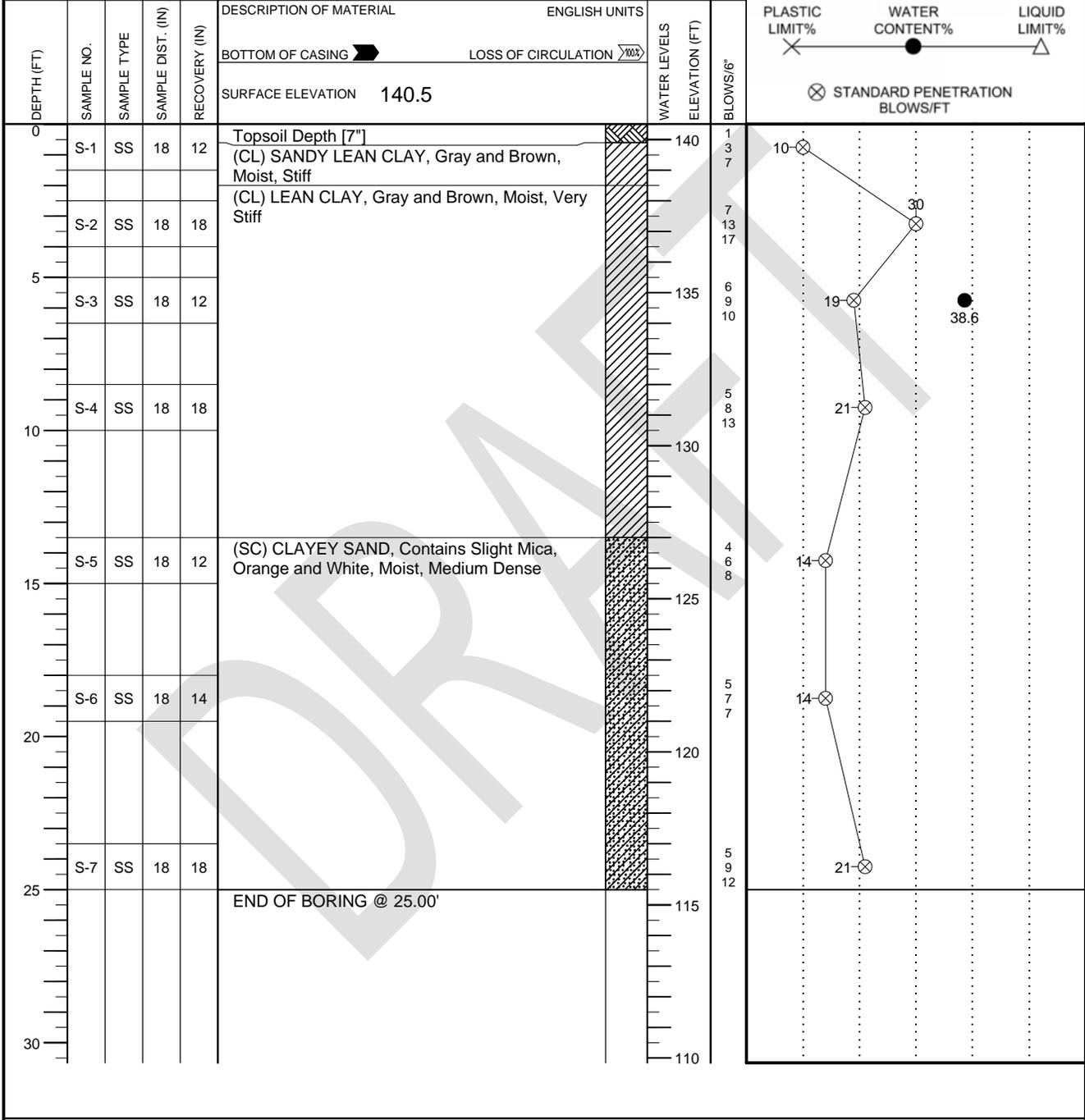
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WL(BCR)	WL(ACR)	BORING COMPLETED	03/06/15	CAVE IN DEPTH @ 17.20'
WL		RIG 750 ATV	FOREMAN DAVID MCLEAN	DRILLING METHOD 2.25 HSA

CLIENT Sorg & Associates	JOB # 24129	BORING # B-8	SHEET 1 OF 1	
PROJECT NAME Patrick Henry ES - Expansion/Renovation	ARCHITECT-ENGINEER			

SITE LOCATION
4643 Taney Avenue, Alexandria, City of Alexandria

NORTHING	EASTING	STATION
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THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL.

WL	WS	WD	BORING STARTED	03/09/15	
WL(BCR)	WL(ACR)		BORING COMPLETED	03/09/15	CAVE IN DEPTH @ 19.00'
WL			RIG 750 ATV	FOREMAN DAVID MCLEAN	DRILLING METHOD 2.25 HSA

Laboratory Testing Summary

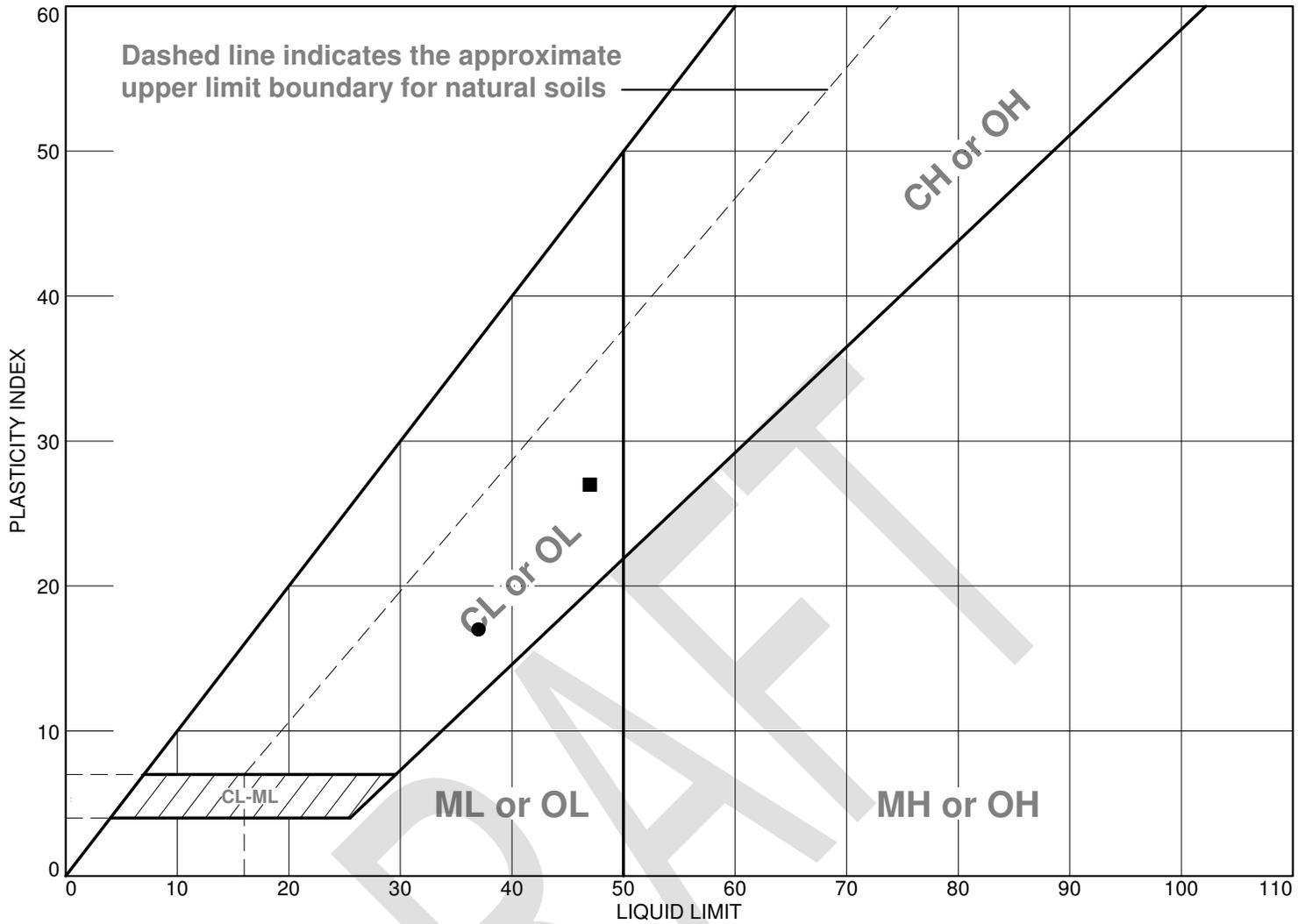
Sample Source	Sample Number	Depth (feet)	MC ¹ (%)	Soil Type ²	Atterberg Limits ³			Percent Passing No. 200 Sieve ⁴	Moisture - Density (Corr.) ⁵		CBR Value ⁶	Other
					LL	PL	PI		Maximum Density (pcf)	Optimum Moisture (%)		
B-1	S-5	13.50 - 15.00	11.6									
B-2	S-4	8.50 - 10.00	16.7	SC	37	20	17	40.0				
B-3	S-3	5.00 - 6.50	35.7									
B-4	S-6	18.50 - 20.00	19.3									
B-5	S-4	8.50 - 10.00	12.0									
B-7	S-3	5.00 - 6.50	17.4	CL	47	20	27	51.8				
B-8	S-3	5.00 - 6.50	38.6									

Notes: 1. ASTM D 2216, 2. ASTM D 2487, 3. ASTM D 4318, 4. ASTM D 1140, 5. See test reports for test method, 6. See test reports for test method
Definitions: MC: Moisture Content, Soil Type: USCS (Unified Soil Classification System), LL: Liquid Limit, PL: Plastic Limit, PI: Plasticity Index, CBR: California Bearing Ratio, OC: Organic Content (ASTM D 2974)

Project No. 24129
Project Name: Patrick Henry ES - Expansion/Renovation
PM: Andy Tao
PE: Bryan C. Layman
Printed On: Wednesday, March 25, 2015



LIQUID AND PLASTIC LIMITS TEST REPORT



	MATERIAL DESCRIPTION	LL	PL	PI	%<#40	%<#200	USCS
●	Clayey Sand Yellowish Brown(SC)	37	20	17	83.2	40.0	SC
■	Sandy Lean Clay Yellowish Brown(CL)	47	20	27	70.6	51.8	CL

Project No. 24129 **Client:** Sorg & Associates

Project: Patrick Henry ES - Expansion/Renovation

● **Source of Sample:** B-2 **Depth:** 8.50-10.00 **Sample Number:** S-4

■ **Source of Sample:** B-7 **Depth:** 5.00-6.50 **Sample Number:** S-3

Remarks:

● Data Entered: 3/16/15



ECS MID-ATLANTIC, LLC

14026 Thunderbolt Place, Suite 100
Chantilly, VA 20151-3232

Phone: (703) 471-8400
Fax: (703) 834-5527

Figure

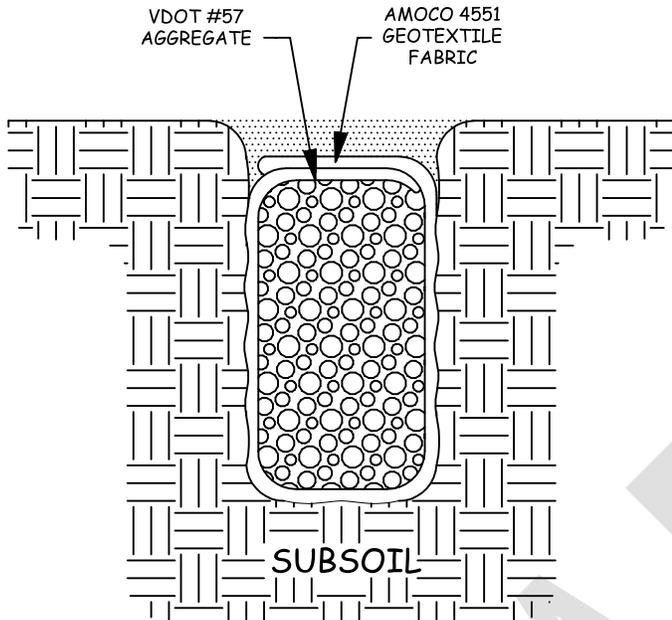
Tested By: HNT

Checked By: DVT

FRENCH DRAIN INSTALLATION PROCEDURE

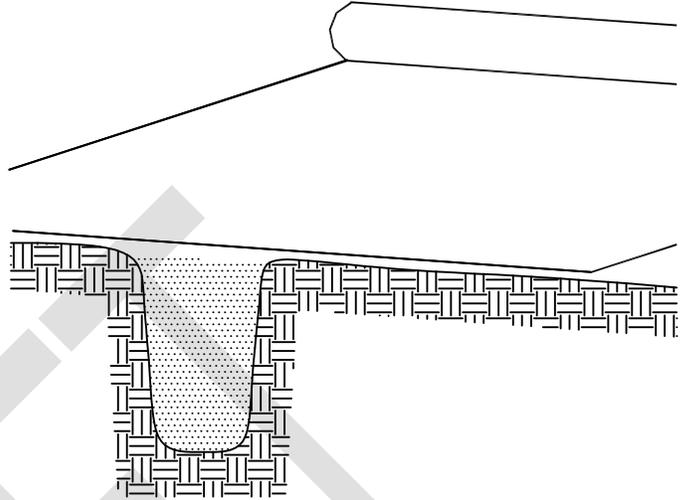
NOT TO SCALE

FINAL CONFIGURATION



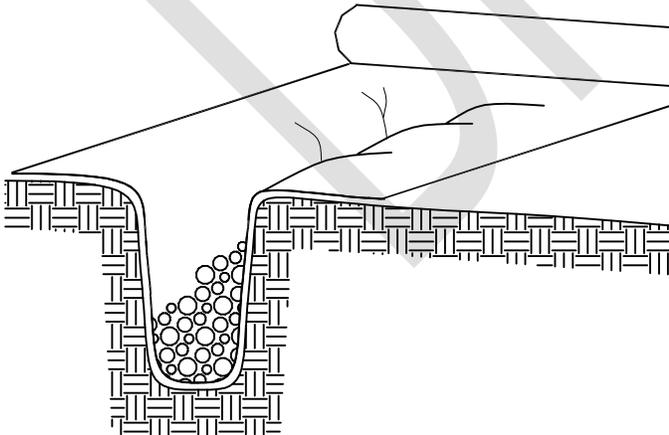
SUBDRAIN USING FILTER FABRIC

STEP 1



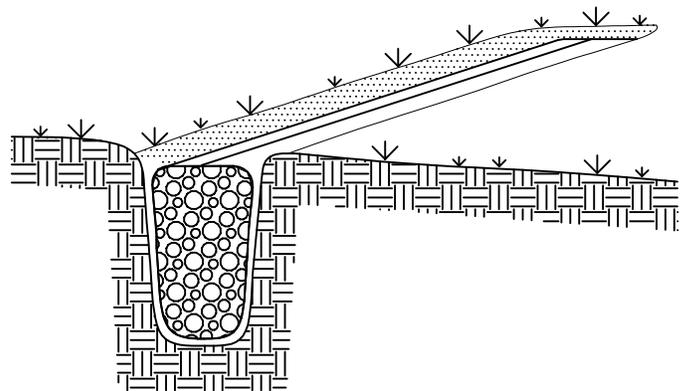
FABRIC IS UNROLLED DIRECTLY OVER TRENCH

STEP 2



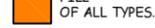
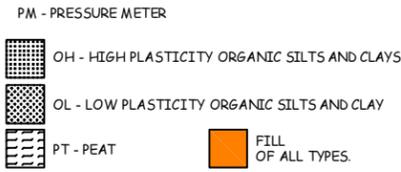
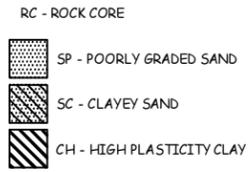
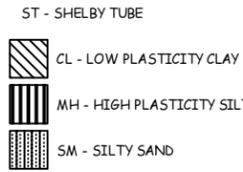
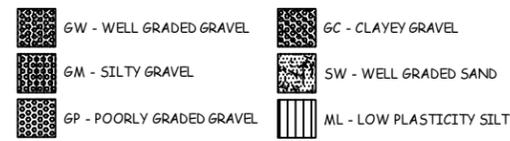
THE TRENCH IS FILLED WITH AGGREGATE

STEP 3

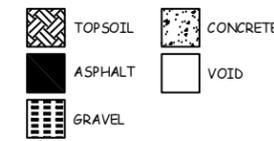


THE FABRIC IS LAPPED CLOSED AND
COVERED WITH BASE STONE

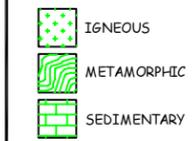
SOIL CLASSIFICATION LEGEND



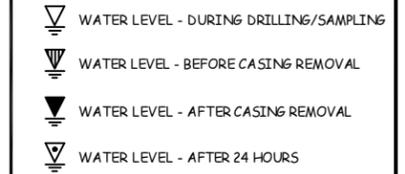
SURFACE MATERIALS



ROCK TYPES



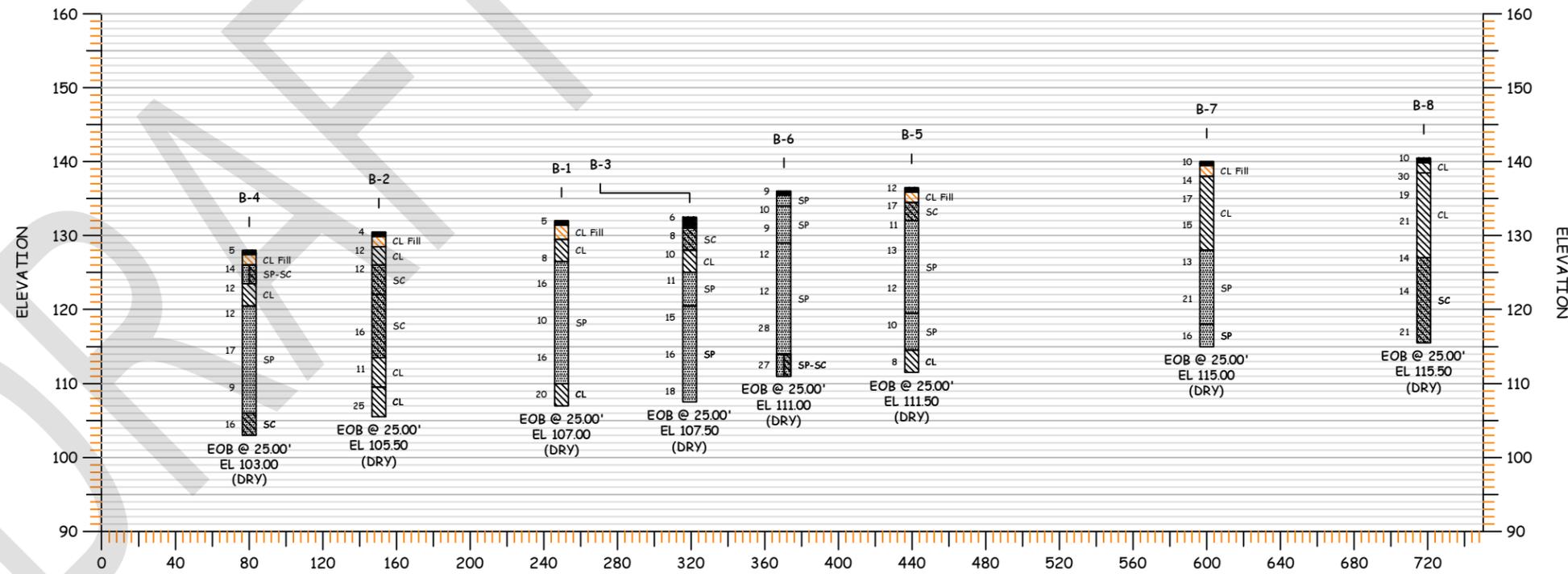
SYMBOL LEGEND



**PATRICK HENRY
ELEMENTARY SCHOOL
ALEXANDRIA, VA**



**CROSS SECTION
A-A**



SCALE
VERTICAL SCALE 1"=20'
HORIZONTAL SCALE 1"=80'

ECS REVISIONS

ENGINEER AT	DRAFTING RAC
----------------	-----------------

SCALE
AS NOTED

PROJECT NO.
24129

SHEET
2 OF 2

DATE
03-26-15

Sorg Architects

**Expansion/Renovation and
Reconfiguration of Existing
Patrick Henry School**

MEP Master Plan

Draft 2 | 8 April 2015

DRAFT

This report takes into account the particular instructions and requirements of our client.

It is not intended for and should not be relied upon by any third party and no responsibility is undertaken to any third party.

Job number 240471-00

Arup USA, Inc
1120 Connecticut Ave N.W.
Washington
DC 20036
United States of America
www.arup.com

ARUP

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3	Recreation Center	10
3.1	Mechanical	10
3.2	Plumbing	11
3.3	Fire Protection Systems	11
3.4	Electrical	12
3.5	Sustainability	12

1 MEP Systems Introduction

The existing Alexandria City Patrick Henry School has approximately 63,000 square feet of program space. The proposed program area is approximately 132,000 square feet of program space for the school and approximately 25,000 square feet of Recreation Center program space. There are three options for the project. One option includes renovation and expansion of the existing school. Two options are comprised of a new facility in different configurations on the site. All options include a new Recreation Center on the site.

The MEP systems will be designed and installed to provide the best value, proper functionality, support sustainable practices and the space plan provided for the school and the recreation center appropriately.

2 Elementary School

2.1 Mechanical

2.1.1 Existing Systems

The current mechanical systems are comprised of various types of equipment for the building. The original steam boiler with local classroom unit ventilators has long been removed. Roof Top DX packaged units are located on the original building built in 1952. These packaged units were installed within the past 10-15 years and each unit serves various classrooms. Various split system dx units also serve areas within the original structure. The new addition built in 2011, utilizes air cooled heat pumps for the classrooms. Each classroom has its own dedicated air handling unit in a closet. Cabinet unit heaters are strategically located at each exterior door.

The existing mechanical equipment is nearing the end of the service life. This equipment will soon need replacement. With this new replacement, the equipment will be more energy efficient and energy costs will be reduced.

One of the major factors associated with the existing HVAC systems is zone control. Currently the existing systems are zoned with large expansive areas being grouped together regardless of utilization. The expansion in 2011 provided the appropriate class room zone control, but the systems being utilized provide minimal operating cost savings.

2.1.2 New Systems

2.1.2.1 Renovation

If the school is renovated, the mechanical equipment should be replaced with DX Rooftop Package units. In addition to providing new roof top equipment, new local zone terminal units should be installed to provide the appropriate zoning per classroom. With this zoning, a variable volume system will provide accurate conditioning of the spaces and demand control ventilation can be utilized. New

DX Rooftop Packaged units utilize multiple compressors with variable load capacity reduction.

The existing ductwork can remain and terminal boxes will be installed for each classroom. Local temperature sensors in each classroom will provide accurate control of each terminal box and provide feedback to each Rooftop Unit. Each Rooftop Unit will be controlled appropriately by polling terminal zones. With duct mounted CO² sensors, demand control ventilation will be utilized and outdoor air will be modulated as needed.

2.1.2.2 New Building

With new technologies and systems to control HVAC systems, there are numerous opportunities to save energy, reduce operating cost, provide accurate conditioning of spaces and control systems based on usage, occupancy and appropriate utilization.

With new technologies including systems with variable airflow, variable speed compressors and overall increase in system efficiencies, these new systems provide alternative ways of reducing energy and providing the indoor air quality and comfort required per zone as needed.

High Efficient options for this type of school are:

- DX Rooftop Packaged Cooling with Natural Gas Heat
- Water Cooled Chillers with Natural Gas Boilers
- Water Source Heat Pumps with Geothermal Ground Source Piping

2.1.3 Life Cycle Cost Analysis

With the space types and square footage of both the School and the Recreation Center, various system types present different opportunities for energy reduction and environmental conditions. A 25 year life cycle cost analysis has been performed comparing the three system options for the mechanical equipment. This life cycle cost analysis has taken into account initial installation costs, maintenance costs, operating costs and any replacement costs during this 25 year life cycle.

Per the cost estimate provided by the Forella Group, the initial cost of the systems are broken down as:

DX Rooftop Packaged Units	- \$5,855,850
Chillers and Boiler Package	- \$6,084,000
Water Source Heat Pumps w/Geothermal	- \$7,605,000

The life cycle costs analysis takes into account an energy code compliant building and compares mechanical costs only. The envelope, lighting, and miscellaneous electrical loads have all been assumed and remain constant throughout each system type. The LCCA utilizes a cost of \$0.08 per kilowatt hour for all systems and the minimum, code compliant, Energy Efficiency Ratio is utilized for each system type. The total energy costs per system type are calculated per year, in

addition to the maintenance costs per year for each system type. A 5% escalation of both energy and costs are utilized per year for the Life Cycle Cost Analysis.

The American Society of Heating, Refrigeration and Air Conditioning Engineers provides service life estimates for all equipment types. The life cycle takes into account these estimates in service life for the each option. Packaged Roof Top Units and Water Source Heat Pumps have a 15 year service life. The chilled water/boiler water system has a greater than 25 year service life. Replacement costs, at the end of the 15 year service life, have been included in the LCCA for the Packaged Roof Top Unit option and the Water Source Heat Pump option.

For a 25 year span, the total Life Cycle Costs for each system are as follows:

DX Rooftop Packaged Units	- \$8,370,928
Chillers and Boiler Package	- \$7,683,312
Water Source Heat Pumps w/Geothermal	- \$10,099,783

This life cycle cost does not take into account the 15 yr replacement of the DX Roof Top Unit option and the Water Source Heat Pump option. With these replacement costs, only the equipment, controls and miscellaneous costs are included in the price.

The 15 yr replacement costs are as follows:

DX Rooftop Packaged Units	-\$4,175,145
Chillers and Boiler Package	-\$3,802,500*
Water Source Heat Pumps w/Geothermal	-\$5,171,400

*(30 Year Replacement cost not taken into account in the 25 yr life cycle costs)

These replacement costs will need to be considered and taken into account (added to the 25yr costs) when comparing each system during a 25 yr life cycle. These costs are shown in the attached documents provided for the Life Cycle Costs Analysis.

2.1.3.1 Recommendations

Space differences in regards to each option may play a role in selection. For the DX Rooftop Packaged units, this option requires the least amount of space within the building. Equipment is located on the roof, ductwork is distributed through the school and terminal boxes are located above the ceiling. The chiller/boiler package will required a central mechanical room, and mechanical rooms to house Air Handling Units. Terminal boxes and ductwork will be located above the ceilings. For the Water Source Heat Pump option, the heat pumps will be located above the ceiling with the ductwork. There will be a small mechanical room housing the pumps and piping manifolds.

The Life Cycle Cost Analysis (LCCA) has provide valuable economic data to make an educated recommendation for the HVAC systems. The recommendation for the system is based on sustainability objectives, first costs, and operating costs. Based on the LCCA, Arup views the DX Rooftop Packaged Units as the most economical, sustainable, and overall best package from a value standpoint. This system will provide the required zoning, the best temperature reset, and also

provide the energy usage and savings for the school. If high efficient DX Roof Top Units are selected with energy recovery, step down capacity reduction and variable speed fans, these units will provide reliable and dependable systems for the facility.

2.2 Plumbing

2.2.1 Existing Systems

The existing plumbing systems are based on the existing floor plan and provide adequate support for bathroom plumbing fixtures and kitchen appliances. Roof drain leaders provide a pathway for storm water to be dispersed through the building and out to the underground storm sewer system. A domestic water heater is located in the main mechanical room. The domestic water heater was installed in 2011. Natural gas piping routed on the roof supports the kitchen, the domestic water heater and comfort heating furnaces in all packaged roof top units.

2.2.2 New Systems

New efficient plumbing systems shall be utilized to preserve resources and reduce energy usage. Those include but are not limited to:

- Low flow fixtures
- High Efficient Water Heaters
- Domestic Hot Water Circulation Loops

2.3 Fire Protection Systems

2.3.1 Existing System

The existing school building is not sprinklered and not provided with a standpipe system. There is an existing 3-inch domestic water service to the building.

2.3.2 New System

For all options (since the renovation will be a Level 3 alteration), fire areas greater than 20,000 SF are required to be provided with sprinkler protection for the Educational occupancy building. Therefore, it is expected the building will be fully sprinklered. Class I standpipes are required for buildings where the highest floor level is more than 30 feet above the lowest level of fire department response. Therefore, it is expected that standpipes will be provided in the four-story, Option 2 building and will not be provided in the three-story, Option 1 building.

A hydrant flow test will be required from the two hydrants nearest the site to determine flow and pressure characteristics of the existing water service. If the existing flow and pressure are not sufficient to provide the required pressure at the hydraulically most remote point in the building, a fire pump will be required. A fire department connection will be required to be connected to the automatic

sprinkler system, and standpipe system if provided. A fire hydrant must be provided within 100 feet of these connections.

A minimum 6-inch water service is expected to be required. Alexandria requires all sprinkler systems to have a testable double check detector backflow prevention device.

2.4 Electrical

2.4.1 Existing Systems

Dominion utility power provides 120/208V, 3phase, 4wire system via utility pole mounted. The electrical service has been routed underground from utility pole mounted transformer to the main electrical room of school. The initial electrical distribution system was built in 1952 and then renovated in 2011. The original electrical distribution design included CT cabinet and main distribution panel sized as 600A. The building has been renovated after that and then the existing electrical system was upgraded into a main 2500A, 120/208V, 3phase, 4wire switchboard with new utility meter. Power is distributed throughout the school from this switchboard via lighting and receptacle panels located in electrical closets throughout the building. It is not clear that the whether building has a dedicated grounding system.

There is not generator for this this building and lighting fixtures for egress path have been provided with battery backup and also there is emergency discharge lighting fixtures at the exit doors.

There are fluorescent lighting fixtures throughout the building which some of them have reached to their life time. There is not centralized lighting control system for the school and each area is controlled individually.

There is existing fire alarm system and also, during the renovation the new fire alarm system had been added and connected to the existing fire alarm system. The system includes horns, bell, strobes, pull stations and audio/ visual devices.

2.4.2 New Systems

According to National Electric Code (NEC), the maximum demand data for a 1-year period for school will be required for all design approaches. If this information is not available, then the maximum demand (measure of average power demand over a 15-minute period) continuously recorded over a minimum 30-day period using power meter connected to the highest loaded phase of the feeder or service will be required.

It is recommended to have the lightning risk assessment for existing and new building. A complete grounding system in accordance with the National Electric Code will be provided. Ground conductors will be run with all feeders, motors and lighting and receptacle branch circuits.

It is recommended to provide LED lighting fixtures everywhere which means retrofit existing fluorescent lighting fixtures and replace them with LED lighting fixtures. A complete system of artificial interior and exterior lighting will be provided for all areas. In general, all interior and exterior lighting will be LED.

Illumination levels will be designed to meet LEED goals by conforming to ASHRAE 90.1. Emergency and exit lighting will be provided for all paths of egress from the buildings with either battery backup or a centralized inverter and batteries.

A complete interior programmable lighting control system including occupancy sensors, switches, time clock and daylight dimming controls where daylight harvesting can contribute to energy saving will be provided to enhance energy efficiency. Exterior lighting will be controlled via a programmable lighting control system.

A new fire alarm system will be required for new building (extension) and also modification and extension to the existing fire alarm system for addition approaches. It is recommended to demolish the older fire alarm system and provide new system connected into the upgraded system. The new fire alarm system will consist of multiple control panels alarmed to a central location via a common communication bus link. Strategically located field processing units will be installed to provide coverage and flexibility needed for such a system.

2.5 Sustainability

The new or renovated Patrick Henry School is envisioned to be a high performance sustainable building. Integrated design will be used throughout the process to create an exemplary green building. The Leadership in Energy and Environmental Design (LEED) for Schools 2009 rating system will be used to provide a recognized benchmark for sustainable achievement. Sustainability goals for the building include meeting the functional requirements of advanced technology while creating a building that has a low carbon footprint, is net zero energy, and is healthy and pleasant to be in. This section discusses the main components we will pursue for achieving a high performance building.

The building will be designed using energy-efficient technologies so that the building's energy demand will be drastically reduced compared to a conventional building design. It is estimated that a new construction building could reduce energy demand by 30% using a combination of technologies including ground-source heat pumps (i.e. geothermal). A renovated building could achieve a higher energy reduction of 40% simply because the baseline building comparison is the existing building¹, which is assumed to have a poor energy performance.

Net Zero Energy

To achieve a goal of net zero energy, the remainder of energy needed for the building should be supplied by on-site renewable generation. In this region, solar photovoltaics is the only technology that should be considered from a cost and efficiency standpoint. The rooftop and site provides a good opportunity for PV. An energy model will be needed to determine the exact amount of PV needed, though some space for PVs will likely be needed on-site in addition to the rooftop. In order to keep accessible green space, a PV canopy parking system would be recommended.

¹ The existing building envelope is used for the baseline, though all mechanical systems will be new in the baseline

2.5.1 Demand Reduction – Building Envelope

2.5.1.1 Increased Insulation

Wall insulation prevents heat from flowing inside or outside the building. In this climate, it is important to provide ample roof, wall and floor insulation to reduce heating and cooling needs.

2.5.1.2 Cool Roof

In a hot climate, preventing solar heat gain is critical. Typical roofs can absorb the sun's energy during the day, turning it to heat which can then enter the building. Cool roofs use light, solar reflective colors to reflect solar radiation rather than absorbing it at the roof. Cool roofs can be used in combination with solar rooftop photovoltaics (described below).

2.5.1.3 Window to Wall Ratio

Windows are generally the poorest-performing part of a façade. In a hot, humid climate, they contribute to solar heat gains, which must then be removed by the cooling system. Therefore, minimizing the ratio of window to wall area on the façade can help to lower cooling energy usage by minimizing solar heat gains and conduction heat gains.

2.5.1.4 High Performance Glazing

Where glass is specified, it will be also designed to reduce solar heat gains. Tinted glass with a low solar heat gain value will be specified to reject solar heat gain (from the infrared spectrum) while allowing some visible light through for daylighting and views.

2.5.1.5 Shading

The proposed building design will use external shading where possible to further control direct solar heat gains. When possible, the shading can be designed to allow the sun to enter in the winter to provide passive solar heating.

2.5.2 Demand Reduction - Lighting

2.5.2.1 Lighting Efficiency

The use of efficient lighting technology can greatly decrease the demand for electrical energy, as lighting is one of the highest energy demands for commercial buildings. It is proposed that the building will use all-LED lighting (or similar performance).

2.5.2.2 Daylight Sensors

When ample daylight conditions are detected by the daylight sensors (greater than 30 fc), perimeter lights near glazing are automatically dimmed. Lights typically 15-25 ft away can be controlled.

2.5.2.3 Lighting Occupancy Sensors

Lighting can additionally be controlled using occupancy sensors in certain areas such as classrooms, offices, cafeteria, and bathrooms. When spaces are unoccupied for a set amount of time, lighting output can be reduced or turned off.

2.5.3 Plug and Process Loads

2.5.3.1 Energy Efficient Computers

Computers represent a high energy user for schools. If energy-efficient office equipment such as low-energy desktops and LED monitors are procured, the overall equipment power density can be reduced by 10% or greater.

2.5.3.2 Energy-Star Equipment

All other equipment such as commercial appliances, televisions, display screens, etc. will meet the US Energy Star guidelines for low powered and standby energy use. This can significantly reduce equipment plug loads.

2.5.4 Demand Reduction - HVAC

2.5.4.1 Demand Control Ventilation

Ventilation air only needs to be supplied when occupants are physically present in a space. Cooling and heating energy is often needed when outside ventilation air is introduced since it can be warmer or cooler than is desirable. Demand control ventilation uses occupancy or CO₂ sensors to regulate the amount of ventilation air needed for each space at a particular time to minimum allowable by code to save energy.

2.5.5 Energy Recovery Ventilator

Cooling and heating energy is used to cool and heat incoming ventilation air. Normally when that air is exhausted, the energy used to provide cooling/heating is lost. Energy recovery uses an enthalpy wheel or similar technology to pre-cool or pre-heat the incoming air with the exhaust air, without exchanging airstreams. Some energy is therefore “recovered”, lowering the amount of cooling and heating needed.

2.5.6 Temperature Setpoints Controlled by Occupancy Sensors

Occupancy sensors which are installed to control lighting can also be tied to increase the temperature in the room to provide less cooling when occupants are not there. This can save on fan and cooling/heating energy by turning off airflow to spaces that are unoccupied. This is only effective in spaces with transient occupancy during the day and are physically separated from partitions.

2.5.7 Site Sustainable Initiatives

2.5.7.1 Solar Photovoltaics

Virginia provides a good climate to use solar photovoltaic technology for electricity generation. A grid-tied system is recommended so that excess energy is exported to the grid when not needed and vice versa.

The rooftop and site provides a good opportunity for PV. An energy model will be needed to determine the exact amount of PV needed, though some space for PVs will likely be needed on-site in addition to the rooftop. In order to keep accessible green space, a PV canopy parking system would be recommended.

2.5.7.2 Ground-Source Heat Pumps

Ground-source heat pumps (GSHPs, often called geothermal) provide high-efficiency electrical heating in the winter, and cooling in the summer. The technology eliminates the need for natural gas or oil for heat, both greenhouse gases. Since the coefficient of performance (COP) of GSHPs can be three or greater, compared to a boiler efficiency of up to 90%, they are a core strategy for heating energy reduction, which is a large contributor to overall building energy reduction.

The climate of Virginia is well-suited for ground-source heat pumps. Boring tests down to 600 feet are currently underway to determine the suitability of ground conditions for the technology.

2.5.8 LEED Certification

Two schemes were evaluated to determine the preliminary LEED scorecards for the project:

1. Renovation/Expansion
2. New Construction

Both schemes will use the LEED 2009 for K-12 School Projects rating system. Scheme 1 performs slightly better with earning LEED credits since the building structure reuse credits are available, and it should be easier to achieve energy reductions and points with the energy model. Under both schemes, however, more information will be needed to move credits from the 'maybe' categories into either a 'yes' or a 'no'. These are early conservative estimates which need to be confirmed as we get further into the design stages.

3 Recreation Center

3.1 Mechanical

3.1.1 Existing Systems

Currently, Roof Top DX packaged units condition the recreation center. This existing mechanical equipment is in fair shape, but is nearing the end of the service life and will soon need replacement. With this new replacement, the efficiency of the equipment will be more energy efficient and energy costs will be reduced. New Systems

3.1.1.1 New Building

The recreation center provides options that may be consistent with systems provided in the school. Depending on the system type, the recreation center can either be connected to the Elementary School mechanical system or can be totally independent.

For the DX Packaged Roof Top Units, the recreation center would be totally separate from the Elementary School system. This system would be independently controlled and operated as needed based on occupancy schedules.

A water cooled chiller with natural gas boiler option would provide energy savings and reduce annual energy costs. The recreation center would be connected to this central energy plant via direct buried piping. Air Handling Units within the Recreation Center would provide local zone control as needed based on occupancy schedules. This system would turn down during unoccupied modes and provide minimal conditioning as needed to maintain unoccupied set points.

Ground Source heat pumps not only provide local zone control, but the connectivity to a ground loop can be separate from the Elementary School or combined for a cost effective Geothermal circuit and to provide energy recovery between the two buildings.

With new technologies including systems with variable airflow, variable speed compressors and overall increase in system efficiencies, these new systems provide alternative ways of reducing energy and providing the indoor air quality and comfort required per zone as needed.

High Efficient options for this Recreation Center similar to the schools are:

- DX Rooftop Packaged Cooling with Natural Gas Heat
- Water Cooled Chillers with Natural Gas Boilers
- Water Source Heat Pumps with Geothermal Ground Source Piping

For the indoor soccer field within the Recreation Center, various options exist based on the environment required in the space. With the proper heat resistant envelope, large propeller fans, and radiant heating, the space can be maintained at a temperature between 80°F - 85°F in the summer and 70 °F in the winter. To achieve a summer environment within the mid 70°F temperature range, HVAC systems similar to the school and the remainder of the Recreation Center should

be utilized. User and owner preference will be a key factor in the direction and selection of HVAC equipment for the indoor soccer field.

3.1.1.2 Renovation

If the recreation center is renovated, the mechanical equipment should be replaced with DX Rooftop Package units. New DX Rooftop Packaged units utilize multiple compressors with variable load capacity reduction. Demand control ventilation will be utilized to provide energy savings associated with conditioning the large quantities of air that are required for a recreation center. This new DX Rooftop Packaged unit will utilize energy recovery to reduce energy usage and to precondition the outdoor air ahead of the heating and cooling coil. The energy recovery module will utilize exhaust air to transfer energy/heat to the outdoor air in the winter or transfer energy/heat from the outdoor air in the summer.

For the indoor soccer field, large propeller fans and radiant heating should be utilized to maintain a temperature between 80°F - 85°F in the summer and 70 °F in the winter. A DX Packaged Rooftop unit will provide ventilation for the soccer field. With a direct digital control package this system will reduce operating costs during unoccupied periods.

3.2 Plumbing

3.2.1 New Systems

New efficient plumbing systems shall be utilized to preserve resources and reduce energy usage. Those include but are not limited to:

- Low flow fixtures
- High Efficient Water Heaters
- Domestic Hot Water Circulation Loops

3.3 Fire Protection Systems

Fire areas greater than 12,000 SF are required to be provided with sprinkler protection for the Assembly occupancy building. Therefore, it is expected the building will be fully sprinklered. Class I standpipes are required for buildings where the highest floor level is more than 30 feet above the lowest level of fire department response. Therefore, it is not expected that standpipes will be provided in the two-story building.

A hydrant flow test will be required from the two hydrants nearest the site to determine flow and pressure characteristics of the existing water service. If the existing flow and pressure are not sufficient to provide the required pressure at the hydraulically most remote point in the building, a fire pump will be required. A fire department connection will be required to be connected to the automatic sprinkler system. A fire hydrant must be provided within 100 feet of these connections.

A minimum 6-inch water service is expected to be required. Alexandria requires all sprinkler systems to have a testable double check detector backflow prevention device.

3.4 Electrical

3.4.1 New Systems

It is recommended to have the lightning risk assessment for the building. A complete grounding system in accordance with the National Electric Code will be provided. Ground conductors will be run with all feeders, motors and lighting and receptacle branch circuits.

It is recommended to provide LED lighting fixtures everywhere. A complete system of artificial interior and exterior lighting will be provided for all areas. In general, all interior and exterior lighting will be LED. Illumination levels will be designed to meet LEED goals by conforming to ASHRAE 90.1. Emergency and exit lighting will be provided for all paths of egress from the buildings with either battery backup or a centralized inverter and batteries.

A complete interior programmable lighting control system including occupancy sensors, switches, time clock and daylight dimming controls where daylight harvesting can contribute to energy saving will be provided to enhance energy efficiency. Exterior lighting will be controlled via a programmable lighting control system.

A new fire alarm system will be required for the building. The new fire alarm system will consist of multiple control panels alarmed to a central location via a common communication bus link. Strategically located field processing units will be installed to provide coverage and flexibility needed for such a system.

3.5 Sustainability

The Leadership in Energy and Environmental Design (LEED) for New Construction and Major Renovations 2009 rating system will be used to provide a recognized benchmark for sustainable achievement. Sustainability goals for the building include meeting the functional requirements of advanced technology while creating a building that has a low carbon footprint, is net zero energy, and is healthy and pleasant to be in. This section discusses the main components we will pursue for achieving a high performance building.

The building will be designed using energy-efficient technologies so that the building's energy demand will be drastically reduced compared to a conventional building design. It is estimated that a new construction building could reduce energy demand by 30% using a combination of technologies including ground-source heat pumps (i.e. geothermal). A renovated building could achieve a higher energy reduction of 40% simply because the baseline building comparison is the existing building², which is assumed to have a poor energy performance.

² The existing building envelope is used for the baseline, though all mechanical systems will be new in the baseline

Net Zero Energy

To achieve a goal of net zero energy, the remainder of energy needed for the building should be supplied by on-site renewable generation. In this region, solar photovoltaics is the only technology that should be considered from a cost and efficiency standpoint. The rooftop and site provides a good opportunity for PV. An energy model will be needed to determine the exact amount of PV needed, though some space for PVs will likely be needed on-site in addition to the rooftop. In order to keep accessible green space, a PV canopy parking system would be recommended.

3.5.1 Demand Reduction – Building Envelope

3.5.1.1 Increased Insulation

Wall insulation prevents heat from flowing inside or outside the building. In this climate, it is important to provide ample roof, wall and floor insulation to reduce heating and cooling needs.

3.5.1.2 Cool Roof

In a hot climate, preventing solar heat gain is critical. Typical roofs can absorb the sun's energy during the day, turning it to heat which can then enter the building. Cool roofs use light, solar reflective colors to reflect solar radiation rather than absorbing it at the roof. Cool roofs can be used in combination with solar rooftop photovoltaics (described below).

3.5.1.3 Window to Wall Ratio

Windows are generally the poorest-performing part of a façade. In a hot, humid climate, they contribute to solar heat gains, which must then be removed by the cooling system. Therefore, minimizing the ratio of window to wall area on the façade can help to lower cooling energy usage by minimizing solar heat gains and conduction heat gains.

3.5.1.4 High Performance Glazing

Where glass is specified, it will be also designed to reduce solar heat gains. Tinted glass with a low solar heat gain value will be specified to reject solar heat gain (from the infrared spectrum) while allowing some visible light through for daylighting and views.

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The proposed building design will use external shading where possible to further control direct solar heat gains. When possible, the shading can be designed to allow the sun to enter in the winter to provide passive solar heating.

3.5.2 Demand Reduction - Lighting

3.5.2.1 Lighting Efficiency

The use of efficient lighting technology can greatly decrease the demand for electrical energy, as lighting is one of the highest energy demands for commercial buildings. It is proposed that the building will use all-LED lighting (or similar performance).

3.5.2.2 Daylight Sensors

When ample daylight conditions are detected by the daylight sensors (greater than 30 fc), perimeter lights near glazing are automatically dimmed. Lights typically 15-25 ft away can be controlled.

3.5.2.3 Lighting Occupancy Sensors

Lighting can additionally be controlled using occupancy sensors in certain areas such as the gymnasium, locker rooms, and media center. When spaces are unoccupied for a set amount of time, lighting output can be reduced or turned off.

3.5.3 Plug and Process Loads

3.5.3.1 Energy-Star Equipment

All other equipment such as commercial appliances, televisions, display screens, etc. will meet the US Energy Star guidelines for low powered and standby energy use. This can significantly reduce equipment plug loads.

3.5.4 Demand Reduction - HVAC

3.5.4.1 Demand Control Ventilation

Ventilation air only needs to be supplied when occupants are physically present in a space. Cooling and heating energy is often needed when outside ventilation air is introduced since it can be warmer or cooler than is desirable. Demand control ventilation uses occupancy or CO₂ sensors to regulate the amount of ventilation air needed for each space at a particular time to minimum allowable by code to save energy.

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3.5.7 Site Sustainable Initiatives

3.5.7.1 Solar Photovoltaics

Virginia provides a good climate to use solar photovoltaic technology for electricity generation. A grid-tied system is recommended so that excess energy is exported to the grid when not needed and vice versa.

The rooftop and site provides a good opportunity for PV. An energy model will be needed to determine the exact amount of PV needed, though some space for PVs will likely be needed on-site in addition to the rooftop. In order to keep accessible green space, a PV canopy parking system would be recommended.

3.5.7.2 Ground-Source Heat Pumps

Ground-source heat pumps (GSHPs, often called geothermal) provide high-efficiency electrical heating in the winter, and cooling in the summer. The technology eliminates the need for natural gas or oil for heat, both greenhouse gases. Since the coefficient of performance (COP) of GSHPs can be three or greater, compared to a boiler efficiency of up to 90%, they are a core strategy for heating energy reduction, which is a large contributor to overall building energy reduction.

The climate of Virginia is well-suited for ground-source heat pumps. Boring tests down to 600 feet are currently underway to determine the suitability of ground conditions for the technology.

3.5.8 LEED Certification

LEED certification for the Recreation Center will be similar to the school. If it is desired to register the building as a separate project, it will likely use the LEED 2009 for New Construction and Major Renovation rating system. Since the projects are both on the same site, they could use the LEED Campus Certification scheme.

The LEED Campus Certification option is intended to reduce costs and streamline the certification process for projects that share a site and are owned by a single entity. As part of a Campus Certification, certain prerequisites and credits are reviewed and pre-approved as campus credits. All prerequisites and credits earned as part of the campus “master site” can be claimed by all LEED projects within that master site, thereby reducing documentation requirements, saving time and costs. It is important to note that the master site will not receive a LEED Certification: only individual projects within the master site earn certifications.

There are additional costs for the Campus Certification approach, but individual projects located within the master site receive discounts off of standard fees.

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8.2 Cost Estimate Report

ESTIMATE OF PROBABLE COST

FEASIBILITY STUDY

JUNE 8, 2015

PATRICK HENRY RECREATION CENTER

4625 Taney Avenue
Alexandria, VA 22304

DRAFT

ARCHITECT:



ARCHITECTS

COST CONSULTANT:



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Fairfax, VA 22031-4713
703.560.2200
www.forellagroup.com

Project Cost Narrative

Forella Group LLC has prepared the following cost estimate for the proposed Patrick Henry Recreation Center based on the proposed building options A, B, and C. This estimate has been prepared using the Unifomat system which groups building components by system and assembly rather than by trades as in the CSI format. This format is standard for this early phase of the project because it allows for all building components to be captured in the estimate given the level of detail at this stage. Options A and B include an indoor multi-purpose turf area surrounded by an indoor running track along with other multi-purpose spaces flanking this turf area. Option C includes a flex court surrounded by an indoor running track in lieu of the multi-purpose turf area and similar additional multi-purpose spaces flanking the flex court.

The following cost estimate includes a pre-engineered metal building structure housing the multi-purpose area in options A and B along with a conventional building structure that houses the remaining spaces. Option C is based upon a conventional building structure with a wide span system for the flex court area.

Option A:	28,156 total square feet 17,856 square feet - pre-engineered metal building portion
Option B:	36,972 total square feet 17,856 square feet – pre-engineered metal building portion
Option C:	17,116 total square feet

Contingencies

This cost estimate is being prepared based on feasibility study site and building drawings. These drawings represent an early conceptual version of what the final buildings will look like. In order to capture some potential costs associated with drawings that are prepared at this early stage of the project a number of contingencies have been included.

Design Contingency – This number will account for any increase in project costs due to the progression of the design beyond the feasibility study level.

Construction Contingency – This will account for any change orders that occur during construction.

Cost Escalation – At this early stage of the project the construction schedule has not yet been determined. This factor will account for an increase in the project cost based on an escalation of construction costs over time. The project cost escalation will include the following:

- 2 Years from June 2015 – 6% Add
- 3 Years from June 2015 – 9.2% Add
- 4 Years from June 2015 – 12.5% Add
- 5 Years from June 2015 – 15.9% Add

Site Construction Cost

In addition to the proposed recreation center this site will also house a new 130,000 sf Pre-K through 8th Grade school building. The costs associated with this school building are contained in a separate report and include a cost for the development of the overall site. This site cost will be shared between the new school building and the new recreation center. Many of the factors that influence the cost of the site work cannot be distinguished between the school and the recreation center at this early stage, the cost estimate is designating 70% of the total site cost for the school and 30% for the recreation center. This figure is based upon the relative building sizes for the school and the recreation center. The site construction cost estimate includes all portions of the site work including storm water management strategies, required site grading, excavation for new construction, and landscaping. The three proposed site options do not vary greatly from one another in terms of the scope of work required, and as such the number included for site work is the same for each of the three options.

Basis of Building Cost Estimates

Pre-Engineered Metal Building Portion:

Foundation:

- Conventional spread footing design, slab on grade based on geotech report dated March 26, 2015.

Building Structure:

- Pre-engineered metal building system with rigid frame and long span truss roof framing.

Roof:

- Standing seam metal roof with insulation below.

Exterior Skin:

- Exterior insulated metal wall panel system.

Interior Finishes:

- Indoor Turf Field: 2 – ½" synthetic infill turf, organic infill, 1" shock pad/impact board
- Running Track: Athletic flooring
- Basic interior finishes and accessories.

Mechanical System:

- Propeller fan system

Conventional Building Portion:

Foundation:

- Conventional spread footing design, based on geotech report dated March 26, 2015.

Building Structure:

- Steel frame, perimeter columns, composite beam assembly, and metal roof deck.

Roof:

- Roofing membrane, cover board, polyisocyanurate insulation

Exterior Skin:

- Uninsulated metal panels on masonry back-up, insulation, and vapor barrier.
- Storefront glazing.

Interior Finishes:

- Flooring: Athletic flooring in flex court and fitness area, tile in bathrooms, decorative tile in lobby, resilient flooring elsewhere.
- Walls: Partial height wall tiles in bathrooms, painted walls throughout.
- Ceilings: Exposed structure in flex court, drywall ceilings in lobby and bathrooms, ACT ceilings elsewhere.

Mechanical System:

- DX Rooftop package cooling with natural gas fired heat, plenum returns, and DDC controls.

Value Management Options

The cost estimate includes some potential value management options that can be explored to reduce the overall cost for each option.

- | | |
|---|-----------------|
| • Option A – Include 50'x50' flex court in pre-engineered building area | (\$87,500) |
| • Option B – Include 50'x50' flex court in pre-engineered building area | (\$87,500) |
| • All Options - Eliminate the running track. | (\$155,500) |
| • Option C – Flex court portion to be a pre-engineered building. | (\$189,500) |
| • All Options- Reduce the number of parking spaces on site. | (\$2,800/space) |

Exclusions

The following elements are excluded from the proposed cost estimate.

- Hazardous materials abatement
- Offsite costs
- Offsite traffic control measures
- Temporary utilities during construction
- It is assumed that there will be no swing space requirement during construction of the new recreation center.



DRAFT

Submission Date:
06/05/2015

Introduction

Project Summary:

Patrick Henry K-8 School & Recreation Center Feasibility Study

Feasibility study of three options to expand Patrick Henry K-8 School & Recreation Center

GSF: n/a

Site: 13 acres

Overview

We are pleased to provide the enclosed estimate of probable cost for the feasibility study of the Patrick Henry K-8 School & Recreation Center project located in Alexandria, VA. Our work is based on the Feasibility Study Documents and information provided by Sorg Architects. If there are any questions, please do not hesitate to contact Mr. Aguero, at (703) 560-2200 or Israel@forellagroup.com

The subject submission provides estimates of hard construction costs. Given the progress nature of the information provided, technical scope interpretations have been made in order to account for all of the costs necessary to deliver a completed facility.

We have assumed construction operations will be confined to the contract limits of the subject property. We have added a *Design Contingency* to budget for unidentified scope requirements not yet delineated. There are numerous *soft costs* and *secondary scope* issues with cost implications, for further information on these, please refer to the Additional Notes and Clarifications section of this report.

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PATRICK HENRY

Section B

Building Cost Summary



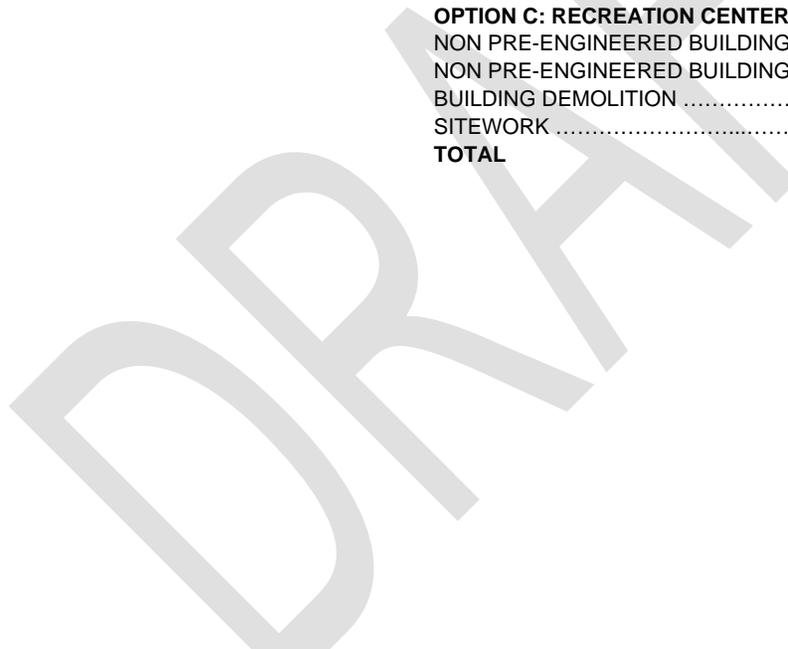
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PROGRAMMATIC COST STUDY Proj Name: Patric Henry K-8 and Rec Center Project Owner: Alexandria City Public School Project Location: Alexandria, VA		FORELLA GROUP, LLC 9495 Silver King Court Suite A Fairfax, VA 22031-4713	Prepared By:	pf/atb	Job #:		Revisions
			Approved By:	pf	File:	Feasibility Study	6/5/2015
				Email:	Design		
				Report Date:	Reference Date Stamp		

Scheme and Location Ref	Unifomat System Heading	Item Code #	Descriptions, Specification & Notes	Computed Quantity [US]	Unit of Meas	Mat+Lab+Equip Loaded Unit	Line Extension	Subtotals
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EXECUTIVE SUMMARY

OPTION A: RECREATION CENTER		
PRE-ENGINEERED BUILDING AREA		3,069,279.64
NON PRE-ENGINEERED BUILDING AREA		2,658,757.33
BUILDING DEMOLITION		86,283.00
SITWORK		1,664,453.66
TOTAL		7,478,773.63
OPTION B: RECREATION CENTER		
PRE-ENGINEERED BUILDING AREA: FLEX COURT & TRACK		3,516,095.07
NON PRE-ENGINEERED BUILDING AREA: OTHER SPACES		3,398,613.93
BUILDING DEMOLITION		86,283.00
SITWORK		1,664,453.66
TOTAL		8,665,445.66
OPTION C: RECREATION CENTER		
NON PRE-ENGINEERED BUILDING AREA: FLEX COURT & TRACK		1,599,265.97
NON PRE-ENGINEERED BUILDING AREA: OTHER SPACES		2,472,163.01
BUILDING DEMOLITION		86,283.00
SITWORK		1,664,453.66
TOTAL		5,822,165.66



Estimate of Probable Cost



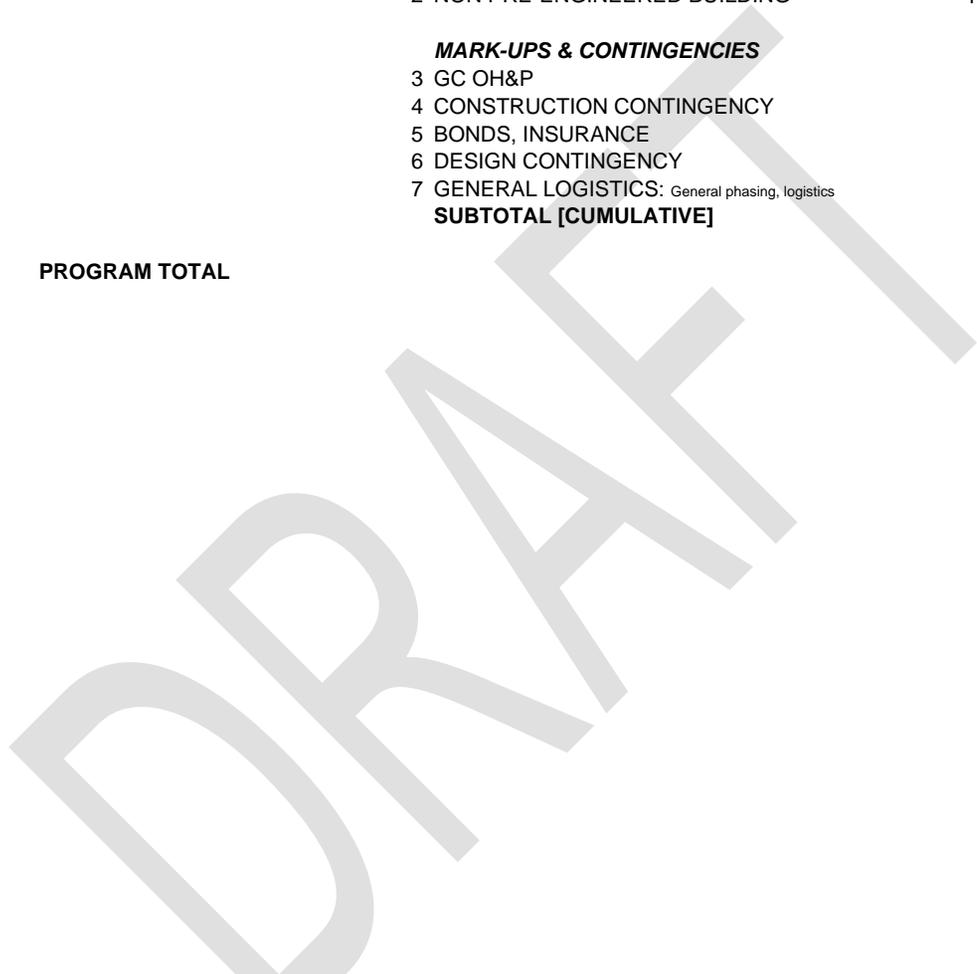
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PROGRAMMATIC COST STUDY Proj Name: Patric Henry K-8 and Rec Center Project Owner: Alexandria City Public School Project Location: Alexandria, VA		FORELLA GROUP, LLC 9495 Silver King Court Suite A Fairfax, VA 22031-4713	Prepared By:	pf/atb	Job #:		Revisions
			Approved By:	pf	File:	Feasibility Study	6/5/2015
				Email:		Design	
				Report Date:		Reference Date Stamp	

Scheme and Location Ref	Uniformal System Heading	Item Code #	Descriptions, Specification & Notes	Computed Quantity [US]	Unit of Meas	Mat+Lab+Equip Loaded Unit	Line Extension	Subtotals
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COST SUMMARY: OPTION A: RECREATION CENTER

HARD CONSTRUCTION COSTS	BUILDINGS	28,156	GSF				
	1 PRE-ENGINEERED BUILDING	17,856	GSF	146.29	2,612,129.48		4,874,881.40
	2 NON PRE-ENGINEERED BUILDING	10,300	GSF	219.68	2,262,751.92		
	MARK-UPS & CONTINGENCIES						
	3 GC OH&P		4.00%		194,995.26		5,069,876.66
	4 CONSTRUCTION CONTINGENCY		3.00%		152,096.30		5,221,972.96
	5 BONDS, INSURANCE		1.50%		78,329.59		5,300,302.55
	6 DESIGN CONTINGENCY		7.00%		371,021.18		5,671,323.73
	7 GENERAL LOGISTICS: <small>General phasing, logistics</small>		1.00%		56,713.24		5,728,036.97
	SUBTOTAL [CUMULATIVE]						5,728,036.97
PROGRAM TOTAL							5,728,036.97

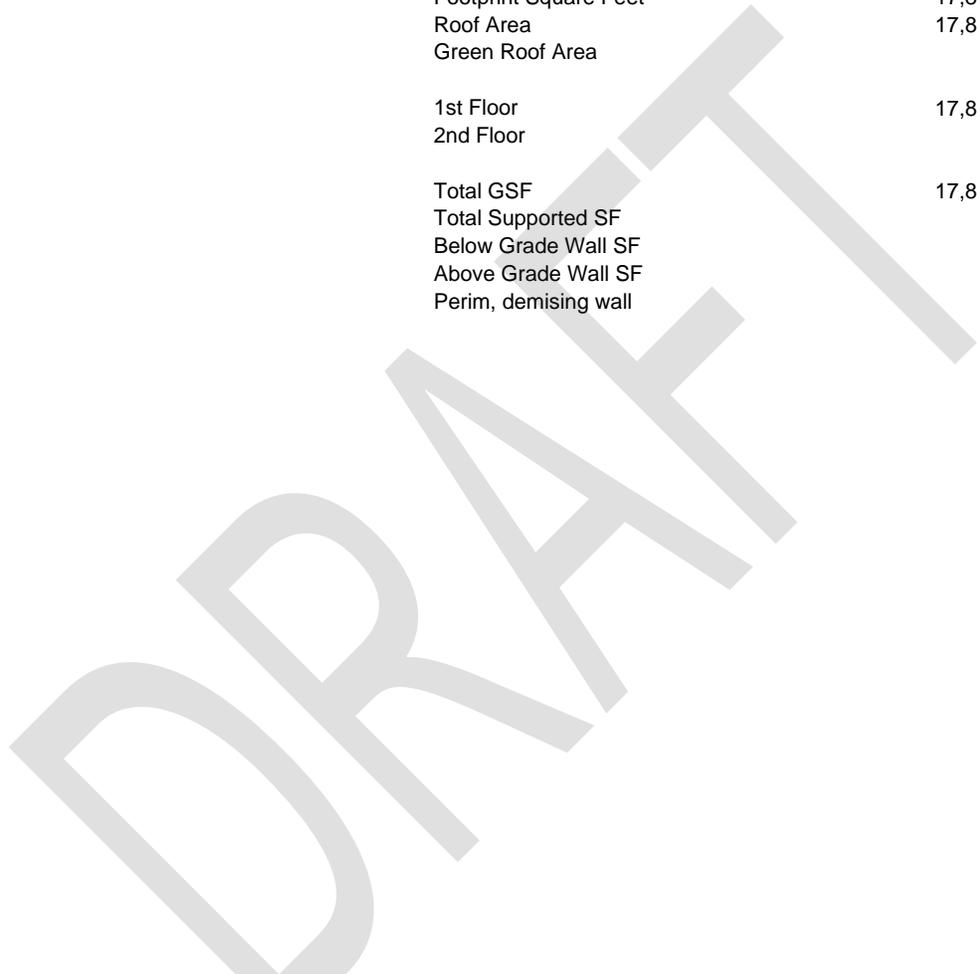


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Option A: PRE-ENGINEERED BUILDING AREA

0.00 PROJECT DATA	CATEGORIES	GSF	Ext. perim LF	Wall Ht	Wall Area
	Footprint Square Feet	17,856.00	392.00		
	Roof Area	17,856.00			
	Green Roof Area	0.00			
	1st Floor	17,856.00	392.00	30.00	11,760.00
	2nd Floor	0.00	150.00	14.00	2,100.00
	Total GSF	17,856.00			
	Total Supported SF	0.00			
	Below Grade Wall SF				0.00
	Above Grade Wall SF				13,860.00
	Perim, demising wall		150.00	16.00	2,400.00



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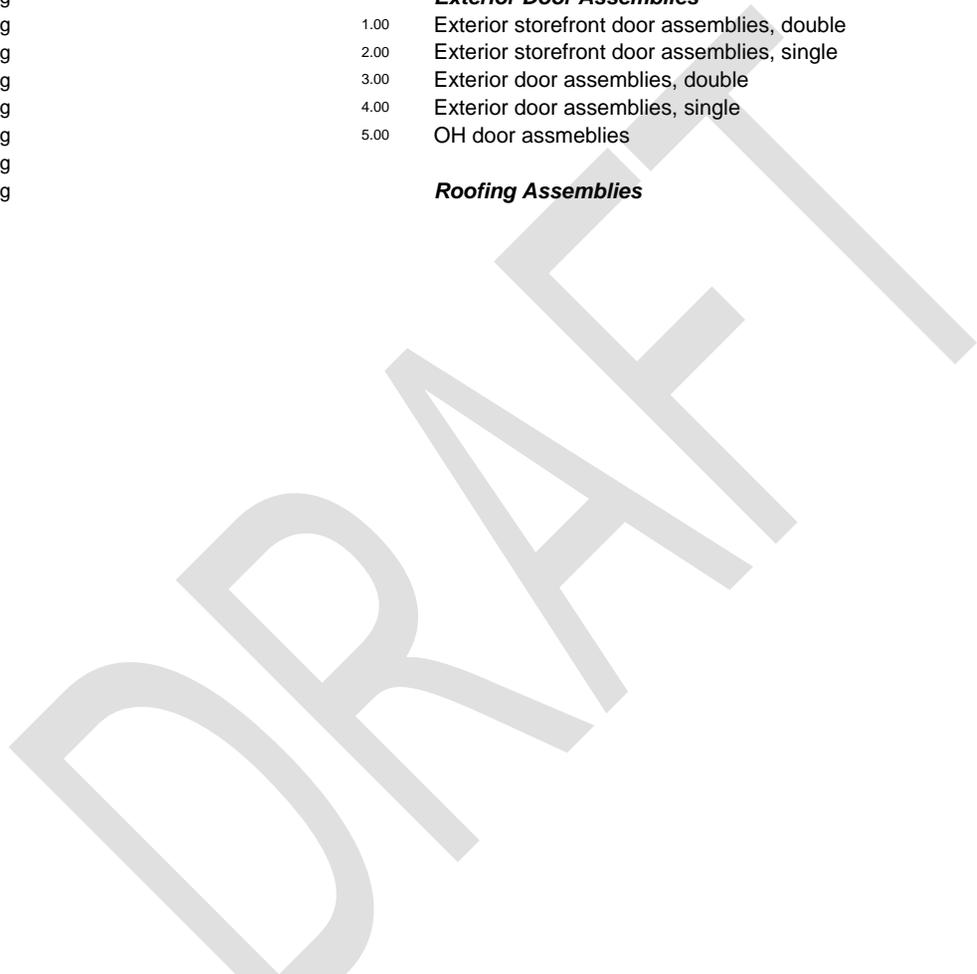
A	SUBSTRUCTURE	0.00	System Subtotal	17,856.00	GSF	11.87		211,970.67
			Lower Level Assemblies					
A. Pre-eng		1.00	Concrete spread footing foundations	165.33	CY	500.00	82,666.67	
A. Pre-eng		2.00	Special foundations		Excluded			
A. Pre-eng		3.00	Foundation drainage	431.20	LF	10.00	4,312.00	
A. Pre-eng		4.00	Concrete SOG std capacity	17,856.00	FLSF	7.00	124,992.00	
B	SHELL		System Subtotal	17,856.00		56.84		1,014,896.00
			Superstructure					
A. Pre-eng		1.00	Preengineered metal bldg	17,856.00	SF	50.00	892,800.00	
A. Pre-eng			Roof Structure					
A. Pre-eng								Included with pre-engineered metal building
A. Pre-eng			Miscellaneous Metals					
A. Pre-eng		1.00	Stairs					None indicated
A. Pre-eng		2.00	Miscellaneous fabrications	17,856.00	SF	2.00	35,712.00	
A. Pre-eng		3.00	Expansion jts assemblies & covers	17,856.00	SF	0.25	4,464.00	
A. Pre-eng			Exterior Enclosure Assemblies					
A. Pre-eng		1.00	Exterior Metal wall panel system					Included with pre-engineered metal building
A. Pre-eng		2.00	Canopy at entrance					Ref site

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B SHELL, continued

			Exterior Fenestration Assemblies					
A. Pre-eng	1.00		Storefront glazing	1,200.00	SF	55.00	66,000.00	
A. Pre-eng	2.00		Curtain Wall		None per direction	Sorg		
A. Pre-eng								
A. Pre-eng			Exterior Door Assemblies					
A. Pre-eng	1.00		Exterior storefront door assemblies, double	0.00	Pair	5,500.00	0.00	
A. Pre-eng	2.00		Exterior storefront door assemblies, single	0.00	Leaf	2,750.00	0.00	
A. Pre-eng	3.00		Exterior door assemblies, double	2.00	Pair	2,700.00	5,400.00	
A. Pre-eng	4.00		Exterior door assemblies, single	1.00	Leafs	1,400.00	1,400.00	
A. Pre-eng	5.00		OH door assmeblies	2.00	EA	4,560.00	9,120.00	
A. Pre-eng								
A. Pre-eng			Roofing Assemblies					
								Included with pre-engineered metal building



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C	INTERIORS		System Subtotal	17,856.00	GSF	11.91		212,609.60
			Partition Assemblies					
A. Pre-eng		1.00	Interior partitions	2,040.00	WallSF	10.00	20,400.00	
A. Pre-eng		2.00	GWB + mtl furring, int face ext walls					
A. Pre-eng		3.00	Interior glass	360.00	WallSF	40.00	14,400.00	
			Door, Frame & Hardware Assemblies					
A. Pre-eng		1.00	Door assm, single	4.00	Leaf	1,400.00	5,600.00	
A. Pre-eng		2.00	Door assm, dbl	2.00	Pair	2,700.00	5,400.00	
A. Pre-eng		3.00	storefront door assemblies, single	0.00	Leaf	2,750.00	0.00	
A. Pre-eng		4.00	storefront door assemblies, double	0.00	Pair	5,500.00	0.00	
			Finish Assemblies					
A. Pre-eng		1.00	Floor finishes: multipurpose field	12,000.00	SF	8.50	102,000.00	
A. Pre-eng		2.00	Floor finishes: track	5,856.00	SF	10.00	58,560.00	
A. Pre-eng		3.00	Ceilings		Exposed			
			Stairs & Railings Assemblies		None indicated			
			Miscellaneous Specialties					
A. Pre-eng		1.00	Toil acc., entr mats & frames, signage, fire ext., jan acc., etc.	17,856.00	GSF	0.15	2,678.40	
A. Pre-eng		2.00	Rough carpentry	17,856.00	GSF	0.20	3,571.20	

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D	SERVICES		System Subtotal	17,856.00	SF	51.10		912,486.24
			Conveying Systems			None indicated		
			HVAC Field			Propeller fan system		
A. Pre-eng		1.00	Equipment	17,856.00	GSF	9.50	169,632.00	
A. Pre-eng		2.00	Air Distribution	17,856.00	GSF	4.75	84,816.00	
A. Pre-eng		3.00	Piping	17,856.00	GSF	2.09	37,319.04	
A. Pre-eng		4.00	Controls (sole sourced)	17,856.00	GSF	2.85	50,889.60	
A. Pre-eng		5.00	Miscellaneous	17,856.00	GSF	1.66	29,685.60	
A. Pre-eng			Plumbing					
A. Pre-eng		1.00	Floor drainage	17,856.00	GSF	1.00	17,856.00	
A. Pre-eng			Fire Protection					
A. Pre-eng		1.00	Fire protection, excl fire pump	17,856.00	GSF	3.00	53,568.00	
A. Pre-eng			Electrical Systems					
A. Pre-eng		1.00	Service & distribution	17,856.00	GSF	7.00	124,992.00	
A. Pre-eng		2.00	Lighting & controls, incl LED	17,856.00	GSF	10.00	178,560.00	
A. Pre-eng		3.00	Branch power devices & wiring	17,856.00	GSF	1.75	31,248.00	
A. Pre-eng		4.00	AV/Communication RI only	17,856.00	GSF	2.00	35,712.00	
A. Pre-eng		5.00	Fire alarm	17,856.00	GSF	2.50	44,640.00	
A. Pre-eng		6.00	Security system	17,856.00	GSF	3.00	53,568.00	

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E	EQ & FURNISHINGS		System Subtotal	17,856.00 SF		5.00		89,280.00
A.	Pre-eng							
A.	Pre-eng		Special					
A.	Pre-eng	1.00	Athletic equipment allowance	17,856.00 SF		5.00	89,280.00	
F	SPECIAL		System Subtotal		Not used			
Z	GENERAL		Subtotal A-F					2,441,242.51
A.	Pre-eng	1.00	Field Overhead				170,886.98	
A.	Pre-eng	2.00	Subtotal				2,612,129.48	
A.	Pre-eng	3.00	GC OH&P, ref Summary				0.00	
A.	Pre-eng	4.00	Subtotal				2,612,129.48	
A.	Pre-eng	5.00	Bonds & Insurance, ref Summary				0.00	
A.	Pre-eng		SUBTOTAL	17,856.00 GSF		146.29		2,612,129.48

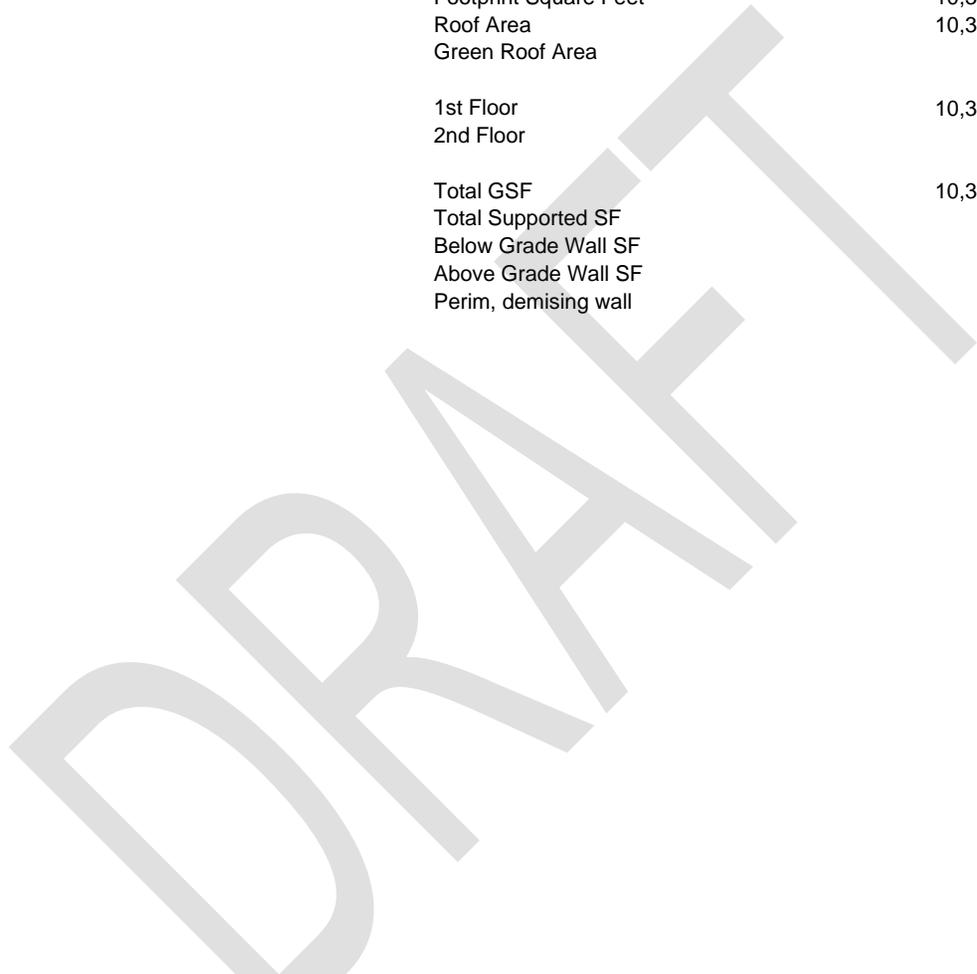
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Option A: NON PRE-ENGINEERED BUILDING AREA

0.00 PROJECT DATA	CATEGORIES	GSF	Ext. perim LF	Wall Ht	Wall Area
	Footprint Square Feet	10,300.00	360.00		
	Roof Area	10,300.00			
	Green Roof Area	0.00			
	1st Floor	10,300.00	360.00	16.00	5,760.00
	2nd Floor	0.00	0.00	0.00	0.00
	Total GSF	10,300.00			
	Total Supported SF	0.00			
	Below Grade Wall SF				0.00
	Above Grade Wall SF				5,760.00
	Perim, demising wall	0.00		0.00	0.00



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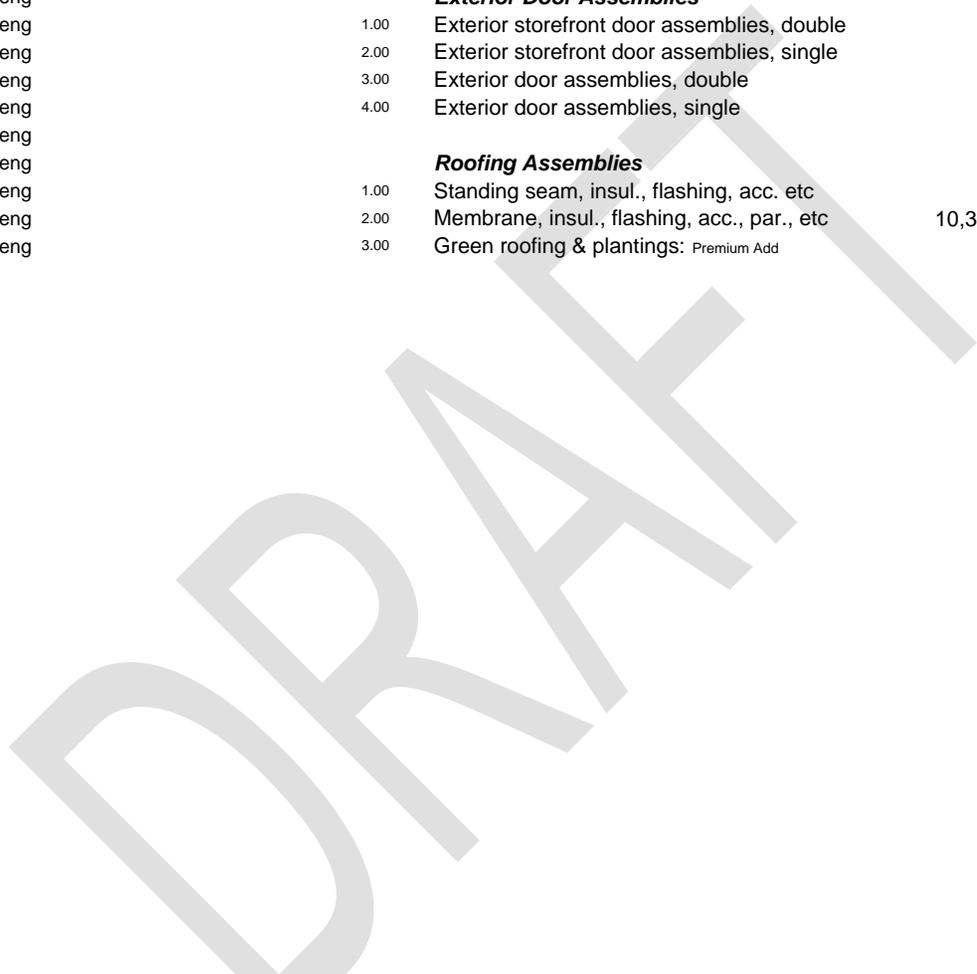
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A	SUBSTRUCTURE	0.00	System Subtotal	10,300.00	GSF	13.09		134,866.22
			Lower Level Assemblies					
A. Non Pre-eng		1.00	Concrete spread footing foundations	114.44	CY	500.00	57,222.22	
A. Non Pre-eng		2.00	Special foundations		Excluded			
A. Non Pre-eng		3.00	Foundation drainage	396.00	LF	14.00	5,544.00	
A. Non Pre-eng		4.00	Concrete SOG std capacity	10,300.00	FLSF	7.00	72,100.00	
B	SHELL		System Subtotal	10,300.00		57.71		594,430.00
			Supported Floor Structure		None indicated			
			Roof Structure					
A. Non Pre-eng		1.00	Roof: Horizontal steel framing & columns	36.05	Tons	3,000.00	108,150.00	
A. Non Pre-eng		2.00	Metal deck assembly	10,300.00	SF	2.25	23,175.00	
A. Non Pre-eng		3.00	Fireproofing, limited	10,300.00	GSF	0.70	7,210.00	
			Miscellaneous Metals					
A. Non Pre-eng		1.00	Stairs		None indicated			
A. Non Pre-eng		2.00	Miscellaneous fabrications	10,300.00	SF	2.00	20,600.00	
A. Non Pre-eng		3.00	Expansion jts assemblies & covers	10,300.00	SF	0.25	2,575.00	
			Exterior Enclosure Assemblies					
A. Non Pre-eng		1.00	Uninsul metal panels, cmu bu incl insul, & vb	3,960.00	Wall SF	47.00	186,120.00	
A. Non Pre-eng		2.00	Canopy at entrance		Ref site			

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B SHELL, continued			Exterior Fenestration Assemblies					
A. Non Pre-eng	1.00		Storefront glazing	1,800.00	SF	55.00	99,000.00	
A. Non Pre-eng	2.00		Curtain Wall		None per direction	Sorg		
A. Non Pre-eng			Exterior Door Assemblies					
A. Non Pre-eng	1.00		Exterior storefront door assemblies, double	1.00	Pair	5,500.00	5,500.00	
A. Non Pre-eng	2.00		Exterior storefront door assemblies, single	0.00	Leaf	2,750.00	0.00	
A. Non Pre-eng	3.00		Exterior door assemblies, double	2.00	Pair	2,700.00	5,400.00	
A. Non Pre-eng	4.00		Exterior door assemblies, single	2.00	Leafs	1,400.00	2,800.00	
A. Non Pre-eng			Roofing Assemblies					
A. Non Pre-eng	1.00		Standing seam, insul., flashing, acc. etc	0.00	RSF	35.00	0.00	
A. Non Pre-eng	2.00		Membrane, insul., flashing, acc., par., etc	10,300.00	RSF	13.00	133,900.00	
A. Non Pre-eng	3.00		Green roofing & plantings: Premium Add	0.00	RSF	38.00	0.00	



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C	INTERIORS		System Subtotal	10,300.00	GSF	45.53		468,920.20
			Partition Assemblies					
A. Non Pre-eng		1.00	Interior partitions	10,660.50	WallSF	10.00	106,605.00	
A. Non Pre-eng		2.00	GWB + mtl furring, int face ext walls		Ref Ext Enclosure Assemblies			
A. Non Pre-eng		3.00	Interior glass	1,184.50	WallSF	40.00	47,380.00	
			Door, Frame & Hardware Assemblies					
A. Non Pre-eng		1.00	Door assm, single	30.00	Leaf	1,400.00	42,000.00	
A. Non Pre-eng		2.00	Door assm, dbl	6.00	Pair	2,700.00	16,200.00	
A. Non Pre-eng		3.00	storefront door assemblies, single	3.00	Leaf	2,750.00	8,250.00	
A. Non Pre-eng		4.00	storefront door assemblies, double	2.00	Pair	5,500.00	11,000.00	
			Finish Assemblies					
A. Non Pre-eng		1.00	Floor finishes: multipurpose, fitness	2,700.00	SF	12.00	32,400.00	
A. Non Pre-eng		2.00	Floor finishes: other areas	7,600.00	SF	6.00	45,600.00	
A. Non Pre-eng		3.00	Ceilings	10,300.00	SF	6.00	61,800.00	
A. Non Pre-eng		4.00	GWB bulkheads	10,300.00	SF	0.75	7,725.00	
A. Non Pre-eng		5.00	CT walls	2,132.10	WSF	12.00	25,585.20	
A. Non Pre-eng		6.00	Paint, interior & exterior	10,300.00	SF	2.25	23,175.00	
			Stairs & Railings Assemblies		None indicated			
			Miscellaneous Specialties					
A. Non Pre-eng		1.00	Toil acc., entr mats & frames, signage, fire ext., jan acc., etc.	10,300.00	GSF	3.00	30,900.00	
A. Non Pre-eng		2.00	Rough carpentry	10,300.00	GSF	1.00	10,300.00	

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D	SERVICES		System Subtotal	10,300.00	SF	80.75		831,725.00
			Conveying Systems		None indicated			
			HVAC					
			DX rooftop package cooling with natural gas fired heat					
A. Non Pre-eng		1.00	Equipment	10,300.00	GSF	17.00	175,100.00	
A. Non Pre-eng		2.00	Air Distribution	10,300.00	GSF	11.00	113,300.00	
A. Non Pre-eng		3.00	Piping	10,300.00	GSF	4.00	41,200.00	
A. Non Pre-eng		4.00	Controls (sole sourced)	10,300.00	GSF	5.00	51,500.00	
A. Non Pre-eng		5.00	Miscellaneous	10,300.00	GSF	1.50	15,450.00	
			Plumbing					
A. Non Pre-eng		1.00	Bathroom fixtures, water, san sewer serv.,	10,300.00	GSF	11.00	113,300.00	
A. Non Pre-eng		2.00	floor & roof drainage					
			Fire Protection					
A. Non Pre-eng		1.00	Fire protection, excl fire pump	10,300.00	GSF	3.00	30,900.00	
			Electrical Systems					
A. Non Pre-eng		1.00	Service & distribution	10,300.00	GSF	7.00	72,100.00	
A. Non Pre-eng		2.00	Lighting & controls, incl LED	10,300.00	GSF	12.00	123,600.00	
A. Non Pre-eng		3.00	Branch power devices & wiring	10,300.00	GSF	1.75	18,025.00	
A. Non Pre-eng		4.00	AV/Communication RI only	10,300.00	GSF	2.00	20,600.00	
A. Non Pre-eng		5.00	Fire alarm	10,300.00	GSF	2.50	25,750.00	
A. Non Pre-eng		6.00	Security system	10,300.00	GSF	3.00	30,900.00	

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			Approved By:	pf	File:	Feasibility Study	6/5/2015
				Email:	Design		
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Scheme and Location Ref	Unifomat System Heading	Item Code #	Descriptions, Specification & Notes	Computed Quantity [US]	Unit of Meas	Mat+Lab+Equip Loaded Unit	Line Extension	Subtotals
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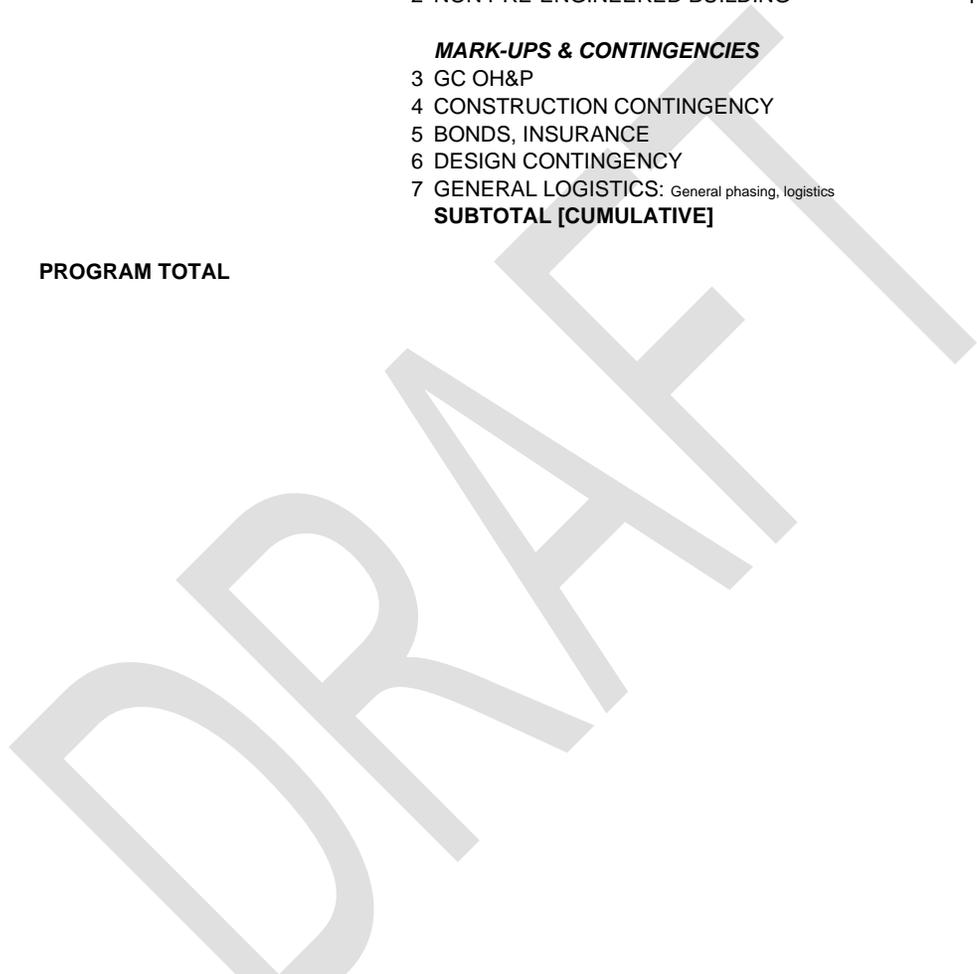
E	EQ & FURNISHINGS		System Subtotal	10,300.00	SF	8.23		84,780.00
	A. Non Pre-eng		Basics					
	A. Non Pre-eng	1.00	Lockers / cubbies	100.00	EA	250.00	25,000.00	
	A. Non Pre-eng	2.00	Casework, millwork, etc.	10,300.00	GSF	3.00	30,900.00	
	A. Non Pre-eng	3.00	Drymarker boards and tack boards	2.00	Rooms	1,440.00	2,880.00	
	A. Non Pre-eng		Special					
	A. Non Pre-eng	1.00	Athletic equipment allowance	5,200.00	Gym SF	5.00	26,000.00	
F	SPECIAL		System Subtotal				Not used	
Z	GENERAL		Subtotal A-F					2,114,721.42
	A. Non Pre-eng	1.00	Field Overhead				148,030.50	
	A. Non Pre-eng	2.00	Subtotal				2,262,751.92	
	A. Non Pre-eng	3.00	GC OH&P, ref Summary				0.00	
	A. Non Pre-eng	4.00	Subtotal				2,262,751.92	
	A. Non Pre-eng	5.00	Bonds & Insurance, ref Summary				0.00	
	A. Non Pre-eng		SUBTOTAL	10,300.00	GSF	219.68		2,262,751.92

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Scheme and Location Ref	Uniformal System Heading	Item Code #	Descriptions, Specification & Notes	Computed Quantity [US]	Unit of Meas	Mat+Lab+Equip Loaded Unit	Line Extension	Subtotals
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COST SUMMARY: OPTION B: RECREATION CENTER

HARD CONSTRUCTION COSTS	BUILDINGS	31,126	GSF				
	1 PRE-ENGINEERED BUILDING	17,856	GSF	167.58	2,992,394.53	5,884,806.00	
	2 NON PRE-ENGINEERED BUILDING	13,270	GSF	217.97	2,892,411.47		
	MARK-UPS & CONTINGENCIES						
	3 GC OH&P		4.00%		235,392.24	6,120,198.24	
	4 CONSTRUCTION CONTINGENCY		3.00%		183,605.95	6,303,804.19	
	5 BONDS, INSURANCE		1.50%		94,557.06	6,398,361.25	
	6 DESIGN CONTINGENCY		7.00%		447,885.29	6,846,246.54	
	7 GENERAL LOGISTICS: General phasing, logistics		1.00%		68,462.47	6,914,709.00	
	SUBTOTAL [CUMULATIVE]					6,914,709.00	
PROGRAM TOTAL						6,914,709.00	

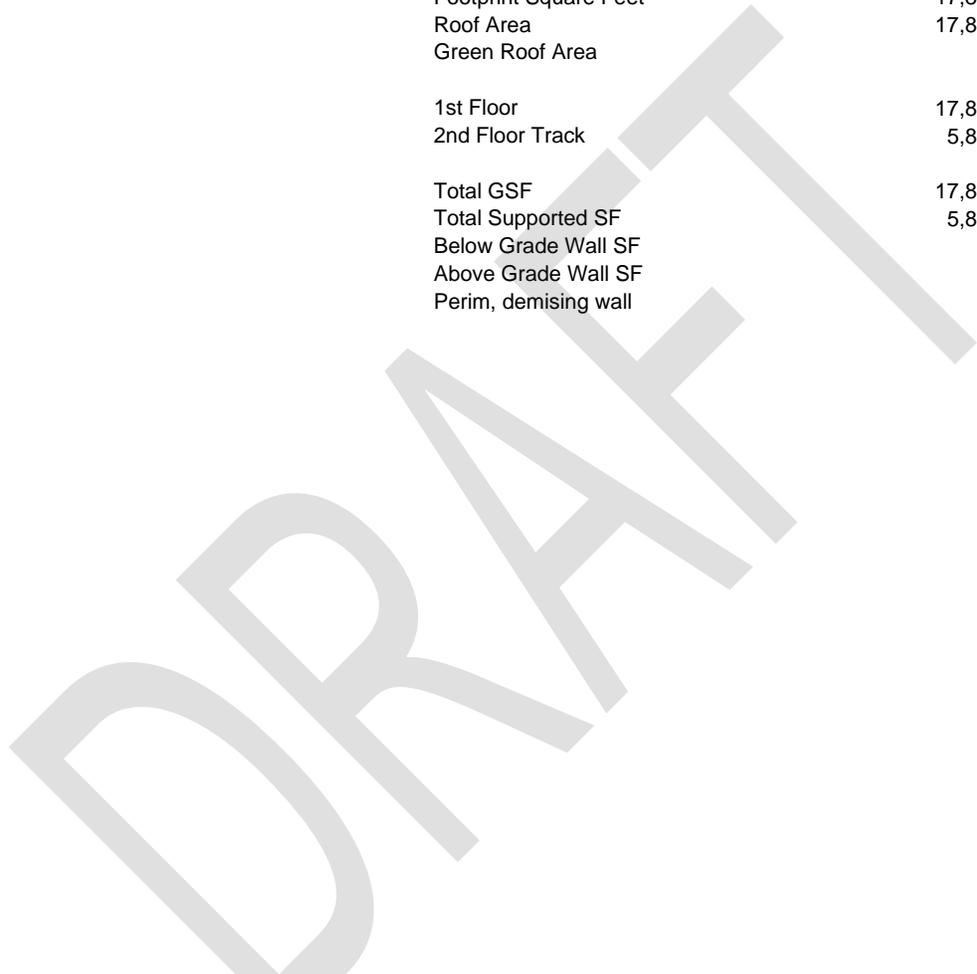


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Option B: PRE-ENGINEERED BUILDING AREA

0.00 PROJECT DATA	CATEGORIES	GSF	Ext. perim LF	Wall Ht	Wall Area
	Footprint Square Feet	17,856.00	400.00		
	Roof Area	17,856.00			
	Green Roof Area	0.00			
	1st Floor	17,856.00	400.00	24.00	9,600.00
	2nd Floor Track	5,856.00	400.00	16.00	6,400.00
	Total GSF	17,856.00			
	Total Supported SF	5,856.00			
	Below Grade Wall SF				0.00
	Above Grade Wall SF				16,000.00
	Perim, demising wall		150.00	16.00	2,400.00



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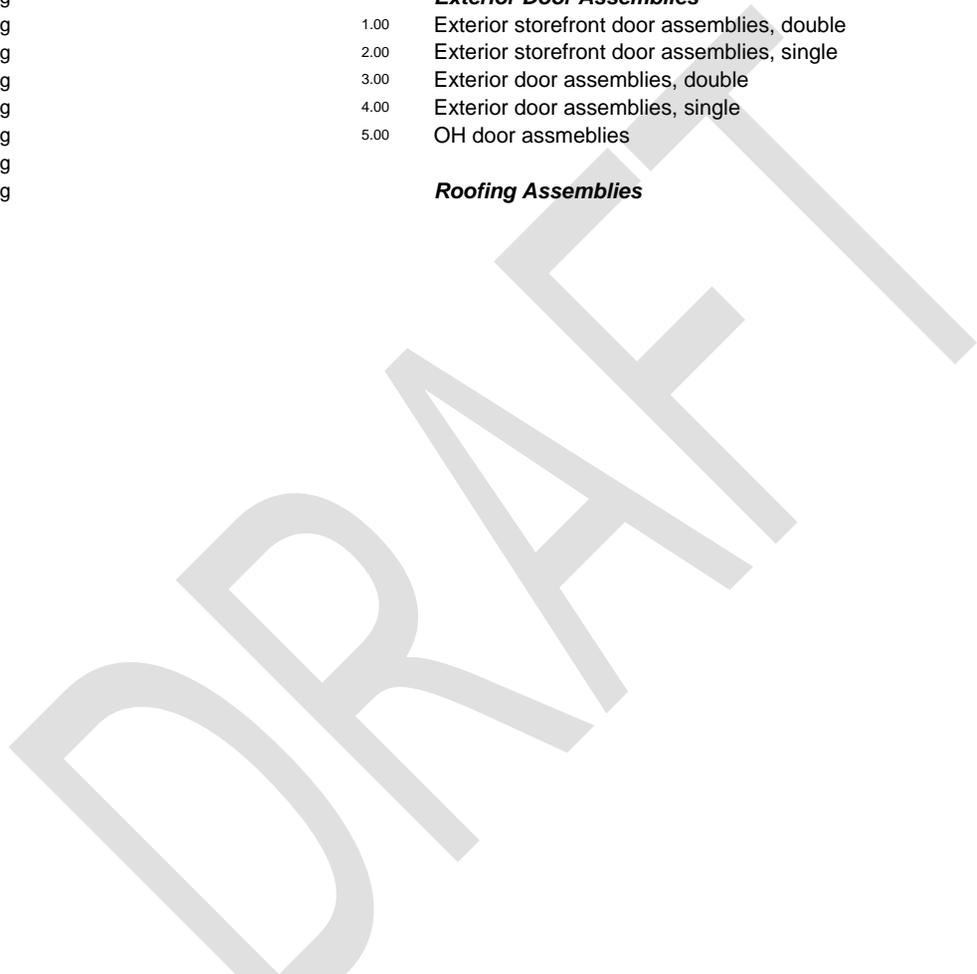
A	SUBSTRUCTURE	0.00	System Subtotal	17,856.00	GSF	12.25		218,672.00
			Lower Level Assemblies					
B. Pre-eng		1.00	Concrete spread footing foundations	178.56	CY	500.00	89,280.00	
B. Pre-eng		2.00	Special foundations		Excluded			
B. Pre-eng		3.00	Foundation drainage	440.00	LF	10.00	4,400.00	
B. Pre-eng		4.00	Concrete SOG std capacity	17,856.00	FLSF	7.00	124,992.00	
B	SHELL		System Subtotal	17,856.00		8.73		155,808.00
			Superstructure					
B. Pre-eng		1.00	Preengineered metal bldg, incl track	17,856.00	SF	60.00	1,071,360.00	
B. Pre-eng			Roof Structure				Included with pre-engineered metal building	
B. Pre-eng			Miscellaneous Metals					
B. Pre-eng		1.00	Stairs				Included with pre-engineered metal building	
B. Pre-eng		2.00	Miscellaneous fabrications	23,712.00	SF	2.00	47,424.00	
B. Pre-eng		3.00	Expansion jts assemblies & covers	17,856.00	SF	0.25	4,464.00	
B. Pre-eng			Exterior Enclosure Assemblies					
B. Pre-eng		1.00	Exterior Metal wall panel system				Included with pre-engineered metal building	
B. Pre-eng		2.00	Canopy at entrance				Ref site	

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B SHELL, continued

			Exterior Fenestration Assemblies					
B. Pre-eng	1.00		Storefront glazing	1,600.00	SF	55.00	88,000.00	
B. Pre-eng	2.00		Curtain Wall			None per direction Sorg		
B. Pre-eng			Exterior Door Assemblies					
B. Pre-eng	1.00		Exterior storefront door assemblies, double	0.00	Pair	5,500.00	0.00	
B. Pre-eng	2.00		Exterior storefront door assemblies, single	0.00	Leaf	2,750.00	0.00	
B. Pre-eng	3.00		Exterior door assemblies, double	2.00	Pair	2,700.00	5,400.00	
B. Pre-eng	4.00		Exterior door assemblies, single	1.00	Leafs	1,400.00	1,400.00	
B. Pre-eng	5.00		OH door assmeblies	2.00	EA	4,560.00	9,120.00	
B. Pre-eng			Roofing Assemblies			Included with pre-engineered metal building		



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C	INTERIORS		System Subtotal	17,856.00	GSF	14.81		264,435.20
			Partition Assemblies					
B. Pre-eng		1.00	Interior partitions	2,040.00	WallSF	10.00	20,400.00	
B. Pre-eng		2.00	GWB + mtl furring, int face ext walls		Ref Ext Enclosure Assemblies			
B. Pre-eng		3.00	Interior glass	360.00	WallSF	40.00	14,400.00	
			Door, Frame & Hardware Assemblies					
B. Pre-eng		1.00	Door assm, single	4.00	Leaf	1,400.00	5,600.00	
B. Pre-eng		2.00	Door assm, dbl	2.00	Pair	2,700.00	5,400.00	
B. Pre-eng		3.00	Storefront door assemblies, single	0.00	Leaf	2,750.00	0.00	
B. Pre-eng		4.00	Storefront door assemblies, double	0.00	Pair	5,500.00	0.00	
			Finish Assemblies					
B. Pre-eng		1.00	Floor finishes: multipurpose field	17,856.00	SF	8.50	151,776.00	
B. Pre-eng		2.00	Floor finishes: track	5,856.00	SF	10.00	58,560.00	
B. Pre-eng		3.00	Ceilings		Exposed			
			Stairs & Railings Assemblies		None indicated			
			Miscellaneous Specialties					
B. Pre-eng		1.00	Toil acc., entr mats & frames, signage, fire ext., jan acc., etc.	23,712.00	GSF	0.15	3,556.80	
B. Pre-eng		2.00	Rough carpentry	23,712.00	GSF	0.20	4,742.40	

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D	SERVICES		System Subtotal	17,856.00 SF		54.20		967,795.20
			Conveying Systems		None indicated			
			HVAC Field		Propeller fan system			
B. Pre-eng		1.00	Equipment	17,856.00 GSF		10.00	178,560.00	
B. Pre-eng		2.00	Air Distribution	17,856.00 GSF		5.00	89,280.00	
B. Pre-eng		3.00	Piping	17,856.00 GSF		2.20	39,283.20	
B. Pre-eng		4.00	Controls (sole sourced)	17,856.00 GSF		3.00	53,568.00	
B. Pre-eng		5.00	Miscellaneous	17,856.00 GSF		1.75	31,248.00	
B. Pre-eng			Plumbing					
B. Pre-eng		1.00	Floor drainage	17,856.00 GSF		1.00	17,856.00	
B. Pre-eng			Fire Protection					
B. Pre-eng		1.00	Fire protection, excl fire pump	17,856.00 GSF		3.00	53,568.00	
B. Pre-eng			Electrical Systems					
B. Pre-eng		1.00	Service & distribution	17,856.00 GSF		7.00	124,992.00	
B. Pre-eng		2.00	Lighting & controls, incl LED	17,856.00 GSF		12.00	214,272.00	
B. Pre-eng		3.00	Branch power devices & wiring	17,856.00 GSF		1.75	31,248.00	
B. Pre-eng		4.00	AV/Communication RI only	17,856.00 GSF		2.00	35,712.00	
B. Pre-eng		5.00	Fire alarm	17,856.00 GSF		2.50	44,640.00	
B. Pre-eng		6.00	Security system	17,856.00 GSF		3.00	53,568.00	

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E	EQ & FURNISHINGS		System Subtotal	17,856.00	SF	6.64		118,560.00
B. Pre-eng								
B. Pre-eng								
B. Pre-eng								
			Special					
		1.00	Athletic equipment allowance	23,712.00	SF	5.00	118,560.00	
F	SPECIAL		System Subtotal		Not used			
Z	GENERAL		Subtotal A-F					2,796,630.40
B. Pre-eng		1.00	Field Overhead				195,764.13	
B. Pre-eng		2.00	Subtotal				2,992,394.53	
B. Pre-eng		3.00	GC OH&P, ref Summary				0.00	
B. Pre-eng		4.00	Subtotal				2,992,394.53	
B. Pre-eng		5.00	Bonds & Insurance, ref Summary				0.00	
B. Pre-eng			SUBTOTAL	17,856.00	GSF	167.58		2,992,394.53

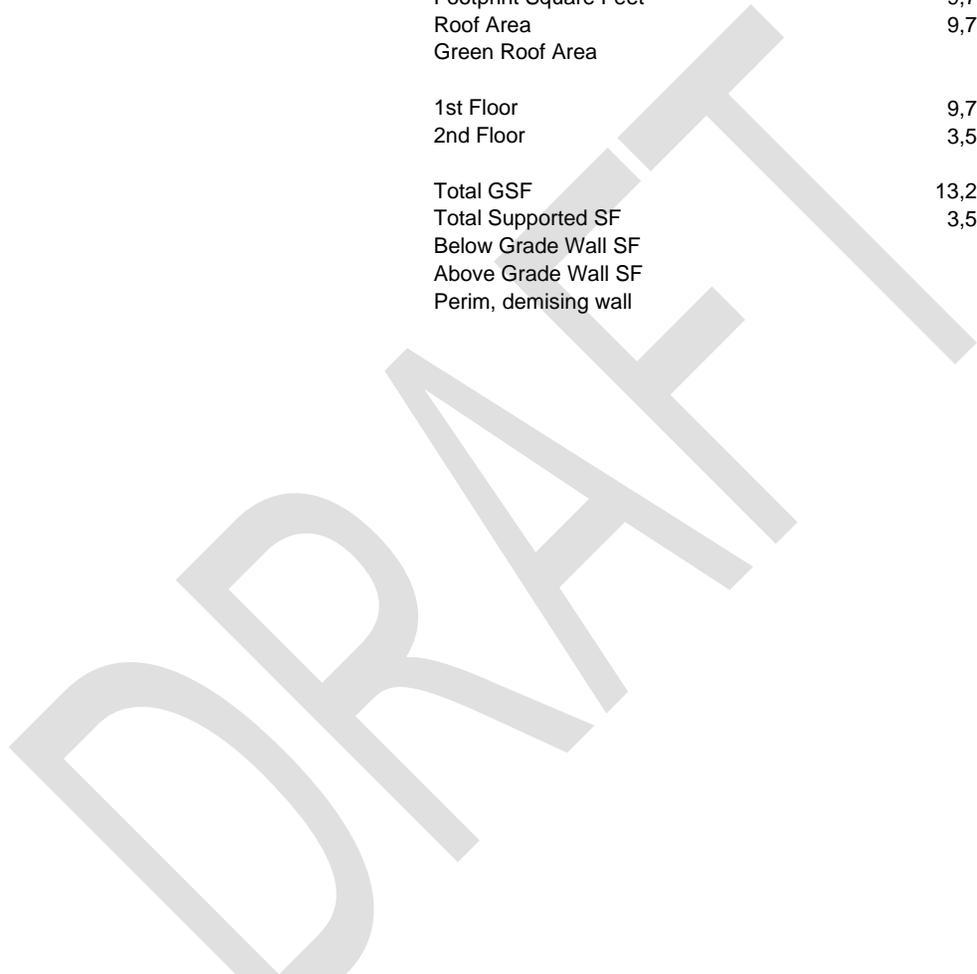
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Option B: NON PRE-ENGINEERED BUILDING AREA

0.00 PROJECT DATA	CATEGORIES	GSF	Ext. perim LF	Wall Ht	Wall Area
	Footprint Square Feet	9,740.00	360.00		
	Roof Area	9,740.00			
	Green Roof Area	0.00			
	1st Floor	9,750.00	334.00	16.00	5,344.00
	2nd Floor	3,520.00	264.00	16.00	4,224.00
	Total GSF	13,270.00			
	Total Supported SF	3,520.00			
	Below Grade Wall SF				0.00
	Above Grade Wall SF				9,568.00
	Perim, demising wall		150.00	16.00	2,400.00



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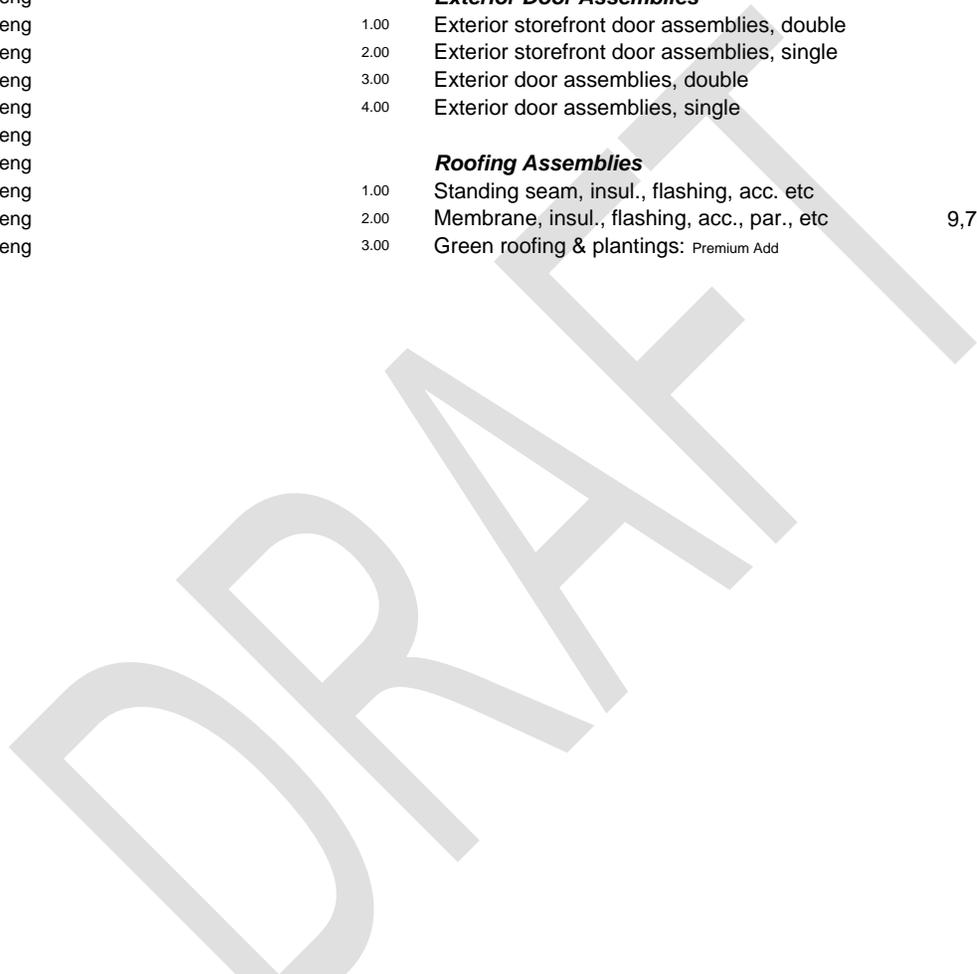
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A	SUBSTRUCTURE	0.00	System Subtotal	13,270.00	GSF	9.63		127,835.11
			Lower Level Assemblies					
B. Non Pre-eng		1.00	Concrete spread footing foundations	108.22	CY	500.00	54,111.11	
B. Non Pre-eng		2.00	Special foundations		Excluded			
B. Non Pre-eng		3.00	Foundation drainage	396.00	LF	14.00	5,544.00	
B. Non Pre-eng		4.00	Concrete SOG std capacity	9,740.00	FLSF	7.00	68,180.00	
B	SHELL		System Subtotal	13,270.00		65.26		865,996.50
			Supported Floor Structure		None indicated			
B. Non Pre-eng		1.00	Supported floor frame & columns	21.12	Tons	3,000.00	63,360.00	
B. Non Pre-eng		2.00	Metal deck assembly	3,520.00	SF	2.50	8,800.00	
B. Non Pre-eng		3.00	Slab on deck	3,520.00	SF	6.00	21,120.00	
B. Non Pre-eng		4.00	Fireproofing, limited	3,520.00	SF	0.75	2,640.00	
			Roof Structure					
B. Non Pre-eng		1.00	Roof: Horizontal steel framing & columns	34.09	Tons	3,000.00	102,270.00	
B. Non Pre-eng		2.00	Metal deck assembly	9,740.00	SF	2.25	21,915.00	
B. Non Pre-eng		3.00	Fireproofing, limited	9,740.00	GSF	0.70	6,818.00	
			Miscellaneous Metals					
B. Non Pre-eng		1.00	Stairs			Ref Stairs & Railings, below		
B. Non Pre-eng		2.00	Miscellaneous fabrications	13,270.00	SF	2.00	26,540.00	
B. Non Pre-eng		3.00	Expansion jts assemblies & covers	13,270.00	SF	0.25	3,317.50	
			Exterior Enclosure Assemblies					
B. Non Pre-eng		1.00	Uninsul metal panels, cmu bu incl insul, & vb	7,168.00	Wall SF	47.00	336,896.00	
B. Non Pre-eng		2.00	Canopy at entrance		Ref site			

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B SHELL, continued		Exterior Fenestration Assemblies						
B. Non Pre-eng	1.00	Storefront glazing	2,400.00	SF	55.00	132,000.00		
B. Non Pre-eng	2.00	Curtain Wall		None per direction	Sorg			
		Exterior Door Assemblies						
B. Non Pre-eng	1.00	Exterior storefront door assemblies, double	1.00	Pair	5,500.00	5,500.00		
B. Non Pre-eng	2.00	Exterior storefront door assemblies, single	0.00	Leaf	2,750.00	0.00		
B. Non Pre-eng	3.00	Exterior door assemblies, double	2.00	Pair	2,700.00	5,400.00		
B. Non Pre-eng	4.00	Exterior door assemblies, single	2.00	Leafs	1,400.00	2,800.00		
		Roofing Assemblies						
B. Non Pre-eng	1.00	Standing seam, insul., flashing, acc. etc	0.00	RSF	35.00	0.00		
B. Non Pre-eng	2.00	Membrane, insul., flashing, acc., par., etc	9,740.00	RSF	13.00	126,620.00		
B. Non Pre-eng	3.00	Green roofing & plantings: Premium Add	0.00	RSF	38.00	0.00		



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C	INTERIORS		System Subtotal	13,270.00	GSF	46.35		615,129.18
			Partition Assemblies					
B. Non Pre-eng		1.00	Interior partitions	13,734.45	WallSF	10.00	137,344.50	
B. Non Pre-eng		2.00	GWB + mtl furring, int face ext walls					
B. Non Pre-eng		3.00	Interior glass	1,526.05	WallSF	40.00	61,042.00	
			Door, Frame & Hardware Assemblies					
B. Non Pre-eng		1.00	Door assm, single	34.00	Leaf	1,400.00	47,600.00	
B. Non Pre-eng		2.00	Door assm, dbl	8.00	Pair	2,700.00	21,600.00	
B. Non Pre-eng		3.00	storefront door assemblies, single	5.00	Leaf	2,750.00	13,750.00	
B. Non Pre-eng		4.00	storefront door assemblies, double	3.00	Pair	5,500.00	16,500.00	
			Finish Assemblies					
B. Non Pre-eng		1.00	Floor finishes: multipurpose, fitness	2,700.00	SF	12.00	32,400.00	
B. Non Pre-eng		2.00	Floor finishes: other areas	10,570.00	SF	6.00	63,420.00	
B. Non Pre-eng		3.00	Ceilings	13,270.00	SF	6.00	79,620.00	
B. Non Pre-eng		4.00	GWB bulkheads	13,270.00	SF	0.75	9,952.50	
B. Non Pre-eng		5.00	CT walls	2,746.89	WSF	12.00	32,962.68	
B. Non Pre-eng		6.00	Paint, interior & exterior	13,270.00	SF	2.25	29,857.50	
			Stairs & Railings Assemblies					
B. Non Pre-eng		1.00	Conc on metal pan: Stairs, landgs, railings, basic finishes	1.00	Flts	16,000.00	16,000.00	
			Miscellaneous Specialties					
B. Non Pre-eng		1.00	Toil acc., entr mats & frames, signage, fire ext., jan acc., etc.	13,270.00	GSF	3.00	39,810.00	
B. Non Pre-eng		2.00	Rough carpentry	13,270.00	GSF	1.00	13,270.00	

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Scheme and Location Ref	Unifomat System Heading	Item Code #	Descriptions, Specification & Notes	Computed Quantity [US]	Unit of Meas	Mat+Lab+Equip Loaded Unit	Line Extension	Subtotals
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D	SERVICES		System Subtotal	13,270.00	SF	75.40		1,000,537.50
			Conveying Systems		None indicated			
B. Non Pre-eng		1.00	Hydraulic elevator	2.00	Stops	38,000.00	76,000.00	
			HVAC		DX rooftop package cooling with natural gas fired heat			
B. Non Pre-eng		1.00	Equipment	10,300.00	GSF	17.00	175,100.00	
B. Non Pre-eng		2.00	Air Distribution	10,300.00	GSF	11.00	113,300.00	
B. Non Pre-eng		3.00	Piping	10,300.00	GSF	4.00	41,200.00	
B. Non Pre-eng		4.00	Controls (sole sourced)	10,300.00	GSF	5.00	51,500.00	
B. Non Pre-eng		5.00	Miscellaneous	10,300.00	GSF	1.50	15,450.00	
			Plumbing					
B. Non Pre-eng		1.00	Bathroom fixtures, water, san sewer serv.,	10,300.00	GSF	11.00	113,300.00	
B. Non Pre-eng		2.00	floor & roof drainage					
			Fire Protection					
B. Non Pre-eng		1.00	Fire protection, excl fire pump	13,270.00	GSF	3.00	39,810.00	
			Electrical Systems					
B. Non Pre-eng		1.00	Service & distribution	13,270.00	GSF	7.00	92,890.00	
B. Non Pre-eng		2.00	Lighting & controls, incl LED	13,270.00	GSF	12.00	159,240.00	
B. Non Pre-eng		3.00	Branch power devices & wiring	13,270.00	GSF	1.75	23,222.50	
B. Non Pre-eng		4.00	AV/Communication RI only	13,270.00	GSF	2.00	26,540.00	
B. Non Pre-eng		5.00	Fire alarm	13,270.00	GSF	2.50	33,175.00	
B. Non Pre-eng		6.00	Security system	13,270.00	GSF	3.00	39,810.00	

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			Approved By:	pf	File:	Feasibility Study	6/5/2015
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Scheme and Location Ref	Unifomat System Heading	Item Code #	Descriptions, Specification & Notes	Computed Quantity [US]	Unit of Meas	Mat+Lab+Equip Loaded Unit	Line Extension	Subtotals
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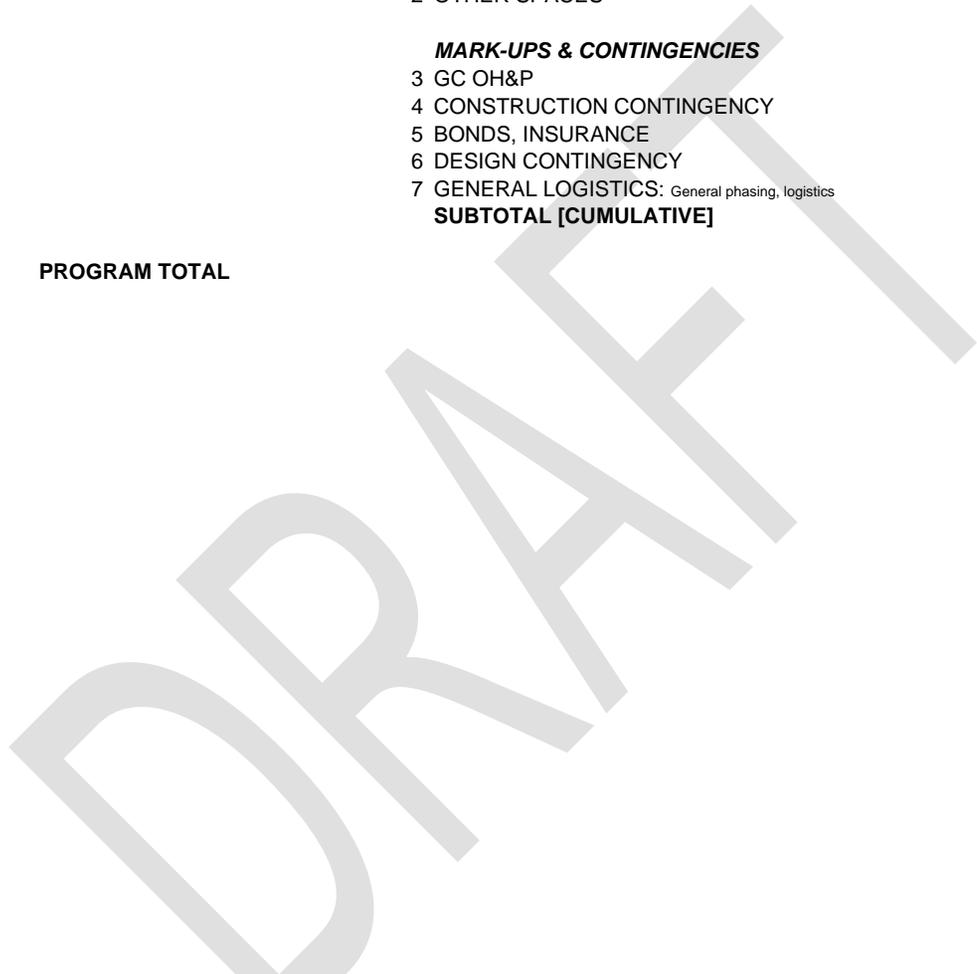
E	EQ & FURNISHINGS		System Subtotal	13,270.00	SF	7.06		93,690.00
			Basics					
	B. Non Pre-eng	1.00	Lockers / cubbies	100.00	EA	250.00	25,000.00	
	B. Non Pre-eng	2.00	Casework, millwork, etc.	13,270.00	GSF	3.00	39,810.00	
	B. Non Pre-eng	3.00	Drymarker boards and tack boards	2.00	Rooms	1,440.00	2,880.00	
	B. Non Pre-eng		Special					
	B. Non Pre-eng	1.00	Athletic equipment allowance	5,200.00	Gym SF	5.00	26,000.00	
F	SPECIAL		System Subtotal				Not used	
Z	GENERAL		Subtotal A-F					2,703,188.29
	B. Non Pre-eng	1.00	Field Overhead				189,223.18	
	B. Non Pre-eng	2.00	Subtotal				2,892,411.47	
	B. Non Pre-eng	3.00	GC OH&P, ref Summary				0.00	
	B. Non Pre-eng	4.00	Subtotal				2,892,411.47	
	B. Non Pre-eng	5.00	Bonds & Insurance, ref Summary				0.00	
	B. Non Pre-eng		SUBTOTAL	13,270.00	GSF	217.97		2,892,411.47

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COST SUMMARY: OPTION C: RECREATION CENTER

HARD CONSTRUCTION COSTS	BUILDINGS	17,116	GSF				
	1 FLEX COURT & TRACK	7,992	GSF	170.30	1,361,065.23	3,465,014.90	
	2 OTHER SPACES	9,124	GSF	230.60	2,103,949.67		
	MARK-UPS & CONTINGENCIES						
	3 GC OH&P		4.00%		138,600.60	3,603,615.50	
	4 CONSTRUCTION CONTINGENCY		3.00%		108,108.46	3,711,723.96	
	5 BONDS, INSURANCE		1.50%		55,675.86	3,767,399.82	
	6 DESIGN CONTINGENCY		7.00%		263,717.99	4,031,117.81	
	7 GENERAL LOGISTICS: <small>General phasing, logistics</small>		1.00%		40,311.18	4,071,428.99	
	SUBTOTAL [CUMULATIVE]					4,071,428.99	
PROGRAM TOTAL						4,071,428.99	

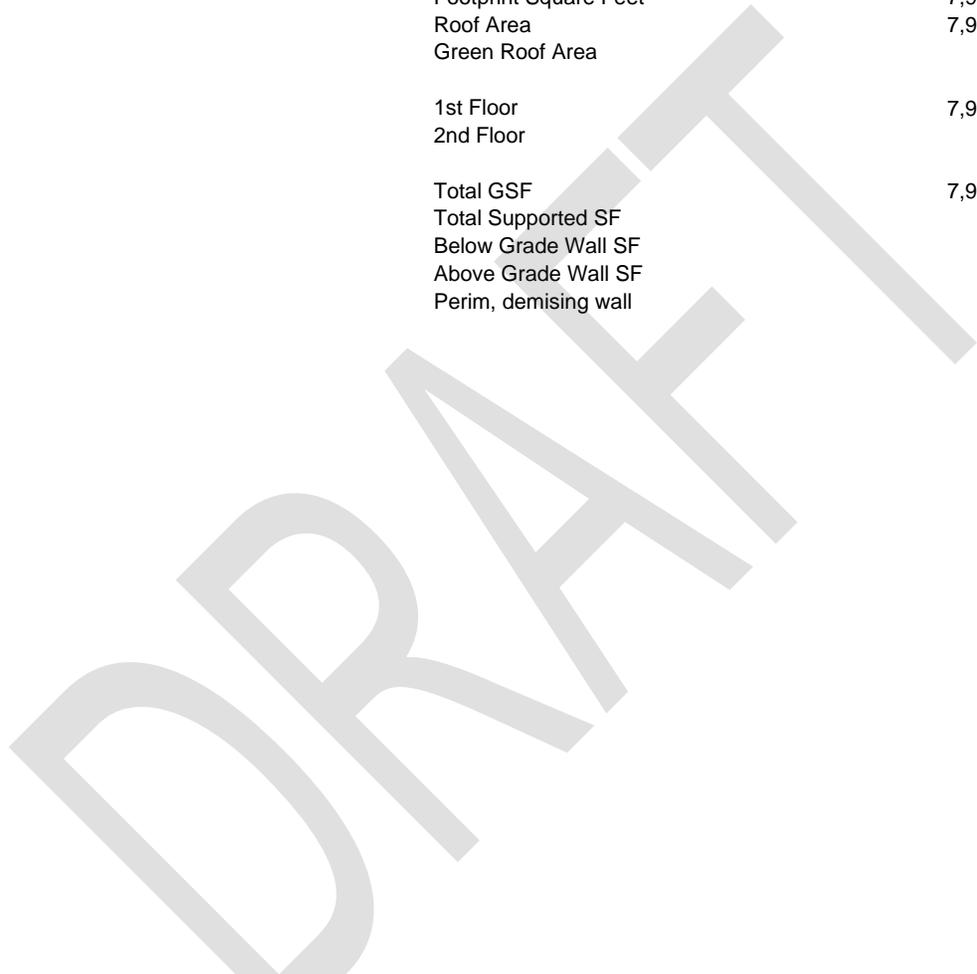


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Option C: NON PRE-ENGINEERED AREA: FLEX COURT & TRACK

0.00 PROJECT DATA	CATEGORIES	GSF	Ext. perim LF	Wall Ht	Wall Area
	Footprint Square Feet	7,992.00	232.00		
	Roof Area	7,992.00			
	Green Roof Area	0.00			
	1st Floor	7,992.00	232.00	24.00	5,568.00
	2nd Floor	0.00	140.00	8.00	1,120.00
	Total GSF	7,992.00			
	Total Supported SF	0.00			
	Below Grade Wall SF				0.00
	Above Grade Wall SF				6,688.00
	Perim, demising wall	0.00		0.00	0.00



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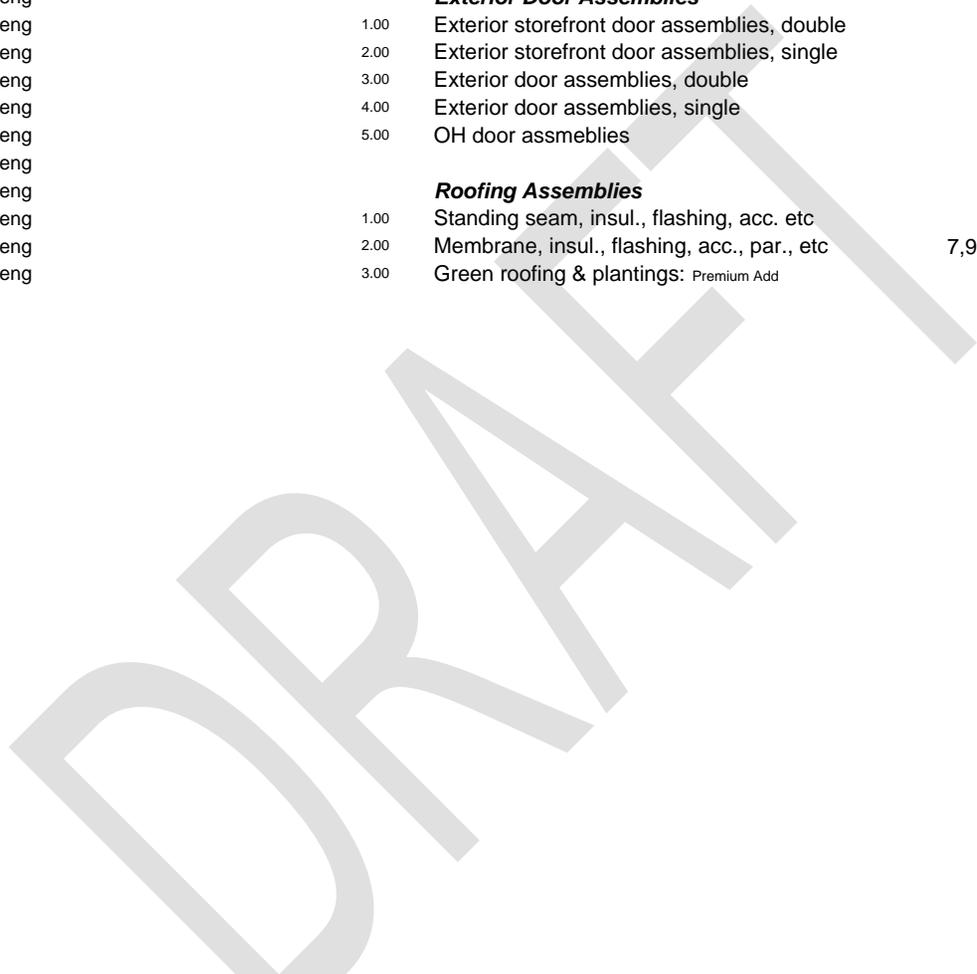
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A	SUBSTRUCTURE	0.00	System Subtotal	7,992.00	GSF	13.93		111,316.80
			Lower Level Assemblies					
C. Non Pre-eng		1.00	Concrete spread footing foundations	103.60	CY	500.00	51,800.00	
C. Non Pre-eng		2.00	Special foundations		Excluded			
C. Non Pre-eng		3.00	Foundation drainage	255.20	LF	14.00	3,572.80	
C. Non Pre-eng		4.00	Concrete SOG std capacity	7,992.00	FLSF	7.00	55,944.00	
B	SHELL		System Subtotal	7,992.00		75.15		600,630.40
			Supported Floor Structure		None indicated			
			Roof Structure					
C. Non Pre-eng		1.00	Roof: Horizontal steel framing & columns	39.96	Tons	3,000.00	119,880.00	
C. Non Pre-eng		2.00	Metal deck assembly	7,992.00	SF	2.25	17,982.00	
C. Non Pre-eng		3.00	Fireproofing, limited	7,992.00	GSF	0.70	5,594.40	
			Miscellaneous Metals					
C. Non Pre-eng		1.00	Stairs		None indicated			
C. Non Pre-eng		2.00	Miscellaneous fabrications	7,992.00	SF	2.00	15,984.00	
C. Non Pre-eng		3.00	Expansion jts assemblies & covers	7,992.00	SF	0.25	1,998.00	
			Exterior Enclosure Assemblies					
C. Non Pre-eng		1.00	Uninsul metal panels, cmu bu incl insul, & vb	5,488.00	Wall SF	47.00	257,936.00	
C. Non Pre-eng		2.00	Canopy at entrance		Ref site			

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B SHELL, continued		Exterior Fenestration Assemblies						
C. Non Pre-eng	1.00	Storefront glazing	1,200.00 SF		55.00		66,000.00	
C. Non Pre-eng	2.00	Curtain Wall			None per direction Sorg			
		Exterior Door Assemblies						
C. Non Pre-eng	1.00	Exterior storefront door assemblies, double	0.00 Pair		5,500.00		0.00	
C. Non Pre-eng	2.00	Exterior storefront door assemblies, single	0.00 Leaf		2,750.00		0.00	
C. Non Pre-eng	3.00	Exterior door assemblies, double	2.00 Pair		2,700.00		5,400.00	
C. Non Pre-eng	4.00	Exterior door assemblies, single	1.00 Leafs		1,400.00		1,400.00	
C. Non Pre-eng	5.00	OH door assmeblies	1.00 EA		4,560.00		4,560.00	
		Roofing Assemblies						
C. Non Pre-eng	1.00	Standing seam, insul., flashing, acc. etc	0.00 RSF		35.00		0.00	
C. Non Pre-eng	2.00	Membrane, insul., flashing, acc., par., etc	7,992.00 RSF		13.00		103,896.00	
C. Non Pre-eng	3.00	Green roofing & plantings: Premium Add	0.00 RSF		38.00		0.00	



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C	INTERIORS		System Subtotal	7,992.00	GSF	13.69		109,397.20
			Partition Assemblies					
C. Non Pre-eng		1.00	Interior partitions	1,904.00	WallSF	10.00	19,040.00	
C. Non Pre-eng		2.00	GWB + mtl furring, int face ext walls					
C. Non Pre-eng		3.00	Interior glass	336.00	WallSF	40.00	13,440.00	
			Door, Frame & Hardware Assemblies					
C. Non Pre-eng		1.00	Door assm, single	4.00	Leaf	1,400.00	5,600.00	
C. Non Pre-eng		2.00	Door assm, dbl	2.00	Pair	2,700.00	5,400.00	
C. Non Pre-eng		3.00	storefront door assemblies, single	0.00	Leaf	2,750.00	0.00	
C. Non Pre-eng		4.00	storefront door assemblies, double	0.00	Pair	5,500.00	0.00	
			Finish Assemblies					
C. Non Pre-eng		1.00	Floor finishes: flex court	4,200.00	SF	6.00	25,200.00	
C. Non Pre-eng		2.00	Floor finishes: track	3,792.00	SF	10.00	37,920.00	
C. Non Pre-eng		3.00	Ceilings		Exposed			
			Stairs & Railings Assemblies		None indicated			
			Miscellaneous Specialties					
C. Non Pre-eng		1.00	Toil acc., entr mats & frames, signage, fire ext., jan acc., etc.	7,992.00	GSF	0.15	1,198.80	
C. Non Pre-eng		2.00	Rough carpentry	7,992.00	GSF	0.20	1,598.40	

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D	SERVICES		System Subtotal	7,992.00	SF	51.39		410,719.18
			Conveying Systems		None indicated			
			HVAC		Propeller fan system			
C. Non Pre-eng		1.00	Equipment	7,992.00	GSF	9.50	75,924.00	
C. Non Pre-eng		2.00	Air Distribution	7,992.00	GSF	4.75	37,962.00	
C. Non Pre-eng		3.00	Piping	7,992.00	GSF	2.09	16,703.28	
C. Non Pre-eng		4.00	Controls (sole sourced)	7,992.00	GSF	2.85	22,777.20	
C. Non Pre-eng		5.00	Miscellaneous	7,992.00	GSF	1.66	13,286.70	
			Plumbing					
C. Non Pre-eng		1.00	Bathroom fixtures, water, san sewer serv.,	10,300.00	GSF	1.00	10,300.00	
C. Non Pre-eng		2.00	floor & roof drainage					
			Fire Protection					
C. Non Pre-eng		1.00	Fire protection, excl fire pump	7,992.00	GSF	3.00	23,976.00	
			Electrical Systems					
C. Non Pre-eng		1.00	Service & distribution	7,992.00	GSF	7.00	55,944.00	
C. Non Pre-eng		2.00	Lighting & controls, incl LED	7,992.00	GSF	10.00	79,920.00	
C. Non Pre-eng		3.00	Branch power devices & wiring	7,992.00	GSF	1.75	13,986.00	
C. Non Pre-eng		4.00	AV/Communication RI only	7,992.00	GSF	2.00	15,984.00	
C. Non Pre-eng		5.00	Fire alarm	7,992.00	GSF	2.50	19,980.00	
C. Non Pre-eng		6.00	Security system	7,992.00	GSF	3.00	23,976.00	

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E	EQ & FURNISHINGS		System Subtotal	7,992.00	SF	5.00		39,960.00
C. Non Pre-eng			Special					
C. Non Pre-eng		1.00	Athletic equipment allowance	7,992.00	Gym SF	5.00	39,960.00	
F	SPECIAL		System Subtotal				Not used	
Z	GENERAL		Subtotal A-F					1,272,023.58
C. Non Pre-eng		1.00	Field Overhead				89,041.65	
C. Non Pre-eng		2.00	Subtotal				1,361,065.23	
C. Non Pre-eng		3.00	GC OH&P, ref Summary				0.00	
C. Non Pre-eng		4.00	Subtotal				1,361,065.23	
C. Non Pre-eng		5.00	Bonds & Insurance, ref Summary				0.00	
C. Non Pre-eng			SUBTOTAL	7,992.00	GSF	170.30		1,361,065.23

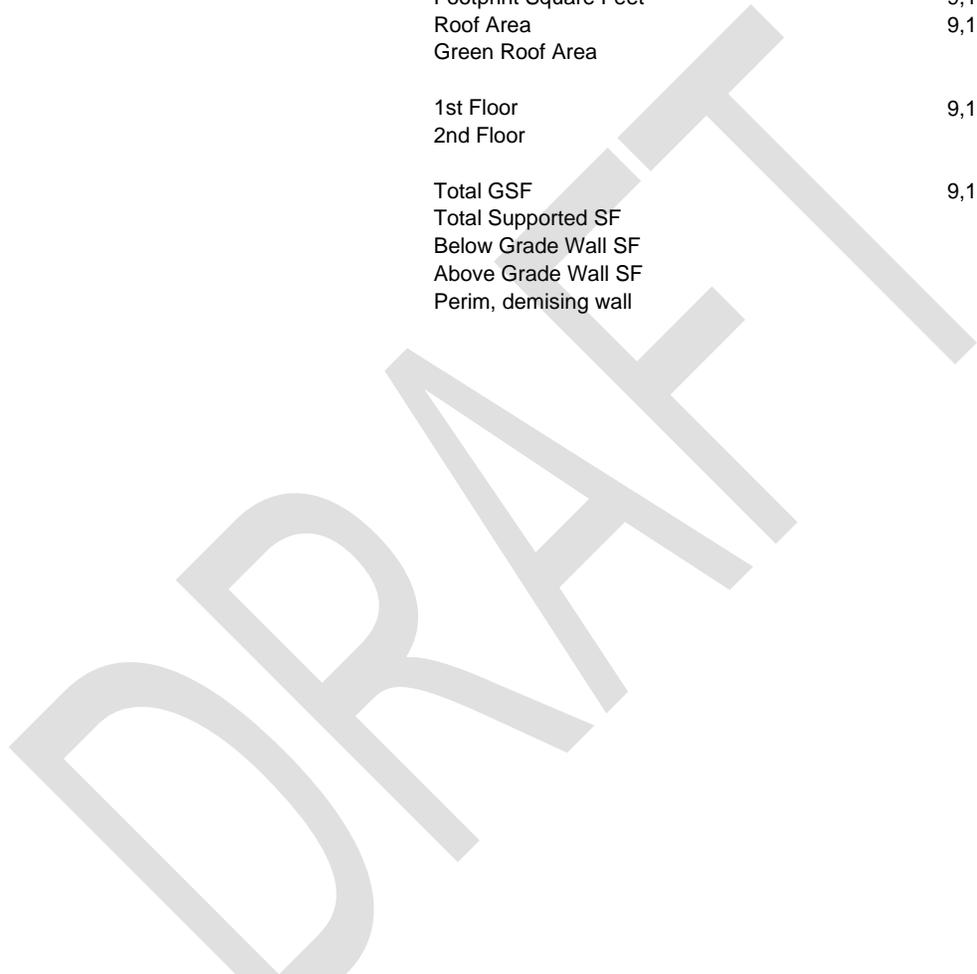
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Option C: NON PRE-ENGINEERED BUILDING AREA: OTHER SPACES

0.00 PROJECT DATA	CATEGORIES	GSF	Ext. perim LF	Wall Ht	Wall Area
	Footprint Square Feet	9,124.00	304.00		
	Roof Area	9,124.00			
	Green Roof Area	0.00			
	1st Floor	9,124.00	304.00	16.00	4,864.00
	2nd Floor	0.00	0.00	0.00	0.00
	Total GSF	9,124.00			
	Total Supported SF	0.00			
	Below Grade Wall SF				0.00
	Above Grade Wall SF				4,864.00
	Perim, demising wall	0.00		0.00	0.00



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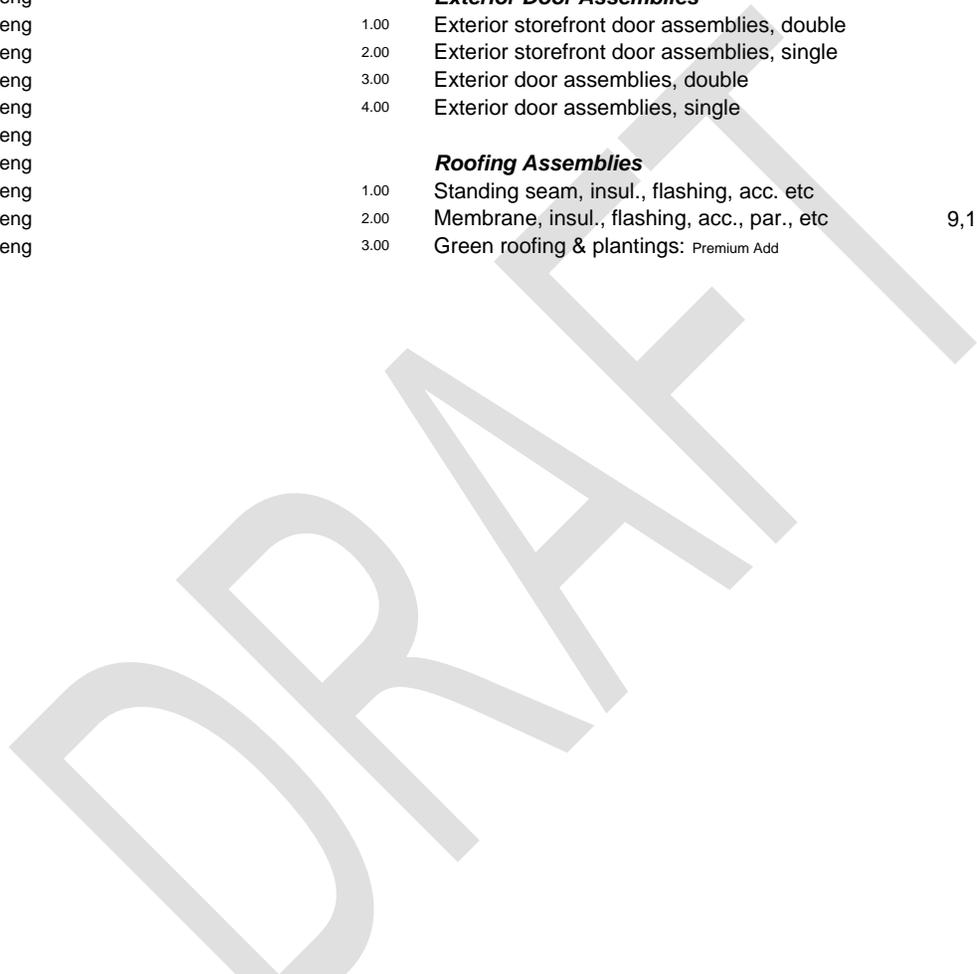
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A	SUBSTRUCTURE	0.00	System Subtotal	9,124.00	GSF	13.07		119,238.49
			Lower Level Assemblies					
C. Non Pre-eng		1.00	Concrete spread footing foundations	101.38	CY	500.00	50,688.89	
C. Non Pre-eng		2.00	Special foundations		Excluded			
C. Non Pre-eng		3.00	Foundation drainage	334.40	LF	14.00	4,681.60	
C. Non Pre-eng		4.00	Concrete SOG std capacity	9,124.00	FLSF	7.00	63,868.00	
B	SHELL		System Subtotal	9,124.00		57.64		525,928.80
			Supported Floor Structure		None indicated			
			Roof Structure					
C. Non Pre-eng		1.00	Roof: Horizontal steel framing & columns	31.93	Tons	3,000.00	95,802.00	
C. Non Pre-eng		2.00	Metal deck assembly	9,124.00	SF	2.25	20,529.00	
C. Non Pre-eng		3.00	Fireproofing, limited	9,124.00	GSF	0.70	6,386.80	
			Miscellaneous Metals					
C. Non Pre-eng		1.00	Stairs		None indicated			
C. Non Pre-eng		2.00	Miscellaneous fabrications	9,124.00	SF	2.50	22,810.00	
C. Non Pre-eng		3.00	Expansion jts assemblies & covers	9,124.00	SF	0.25	2,281.00	
			Exterior Enclosure Assemblies					
C. Non Pre-eng		1.00	Uninsul metal panels, cmu bu incl insul, & vb	3,064.00	Wall SF	47.00	144,008.00	
C. Non Pre-eng		2.00	Canopy at entrance		Ref site			

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B SHELL, continued			Exterior Fenestration Assemblies					
C. Non Pre-eng	1.00	Storefront glazing	1,800.00 SF		55.00	99,000.00		
C. Non Pre-eng	2.00	Curtain Wall			None per direction Sorg			
C. Non Pre-eng								
C. Non Pre-eng								
C. Non Pre-eng	1.00	Exterior storefront door assemblies, double	2.00 Pair		5,500.00	11,000.00		
C. Non Pre-eng	2.00	Exterior storefront door assemblies, single	0.00 Leaf		2,750.00	0.00		
C. Non Pre-eng	3.00	Exterior door assemblies, double	1.00 Pair		2,700.00	2,700.00		
C. Non Pre-eng	4.00	Exterior door assemblies, single	2.00 Leafs		1,400.00	2,800.00		
C. Non Pre-eng								
C. Non Pre-eng								
C. Non Pre-eng	1.00	Standing seam, insul., flashing, acc. etc	0.00 RSF		35.00	0.00		
C. Non Pre-eng	2.00	Membrane, insul., flashing, acc., par., etc	9,124.00 RSF		13.00	118,612.00		
C. Non Pre-eng	3.00	Green roofing & plantings: Premium Add	0.00 RSF		38.00	0.00		



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C	INTERIORS		System Subtotal	9,124.00	GSF	46.76		426,673.82
			Partition Assemblies					
C. Non Pre-eng		1.00	Interior partitions	9,443.34	WallSF	10.00	94,433.40	
C. Non Pre-eng		2.00	GWB + mtl furring, int face ext walls					
C. Non Pre-eng		3.00	Interior glass	1,049.26	WallSF	40.00	41,970.40	
			Door, Frame & Hardware Assemblies					
C. Non Pre-eng		1.00	Door assm, single	30.00	Leaf	1,400.00	42,000.00	
C. Non Pre-eng		2.00	Door assm, dbl	6.00	Pair	2,700.00	16,200.00	
C. Non Pre-eng		3.00	storefront door assemblies, single	3.00	Leaf	2,750.00	8,250.00	
C. Non Pre-eng		4.00	storefront door assemblies, double	2.00	Pair	5,500.00	11,000.00	
			Finish Assemblies					
C. Non Pre-eng		1.00	Floor finishes: multipurpose, fitness	2,800.00	SF	12.00	33,600.00	
C. Non Pre-eng		2.00	Floor finishes: other areas	6,324.00	SF	6.00	37,944.00	
C. Non Pre-eng		3.00	Ceilings	9,124.00	SF	6.00	54,744.00	
C. Non Pre-eng		4.00	GWB bulkheads	9,124.00	SF	0.75	6,843.00	
C. Non Pre-eng		5.00	CT walls	1,888.67	WSF	12.00	22,664.02	
C. Non Pre-eng		6.00	Paint, interior & exterior	9,124.00	SF	2.25	20,529.00	
			Stairs & Railings Assemblies		None indicated			
			Miscellaneous Specialties					
C. Non Pre-eng		1.00	Toil acc., entr mats & frames, signage, fire ext., jan acc., etc.	9,124.00	GSF	3.00	27,372.00	
C. Non Pre-eng		2.00	Rough carpentry	9,124.00	GSF	1.00	9,124.00	

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			Approved By: pf	File:	Feasibility Study	6/5/2015
				Email:	Design	
				Report Date:	Reference Date Stamp	

Scheme and Location Ref	Uniformal System Heading	Item Code #	Descriptions, Specification & Notes	Computed Quantity [US]	Unit of Meas	Mat+Lab+Equip Loaded Unit	Line Extension	Subtotals
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D	SERVICES		System Subtotal	9,124.00	SF	87.13		794,975.00
			Conveying Systems			None indicated		
			HVAC			DX rooftop package cooling with natural gas fired heat		
C. Non Pre-eng		1.00	Equipment	10,300.00	GSF	17.00	175,100.00	
C. Non Pre-eng		2.00	Air Distribution	10,300.00	GSF	11.00	113,300.00	
C. Non Pre-eng		3.00	Piping	10,300.00	GSF	4.00	41,200.00	
C. Non Pre-eng		4.00	Controls (sole sourced)	10,300.00	GSF	5.00	51,500.00	
C. Non Pre-eng		5.00	Miscellaneous	10,300.00	GSF	1.50	15,450.00	
			Plumbing					
C. Non Pre-eng		1.00	Bathroom fixtures, water, san sewer serv.,	10,300.00	GSF	11.00	113,300.00	
C. Non Pre-eng		2.00	floor & roof drainage					
			Fire Protection					
C. Non Pre-eng		1.00	Fire protection, excl fire pump	9,124.00	GSF	3.00	27,372.00	
			Electrical Systems					
C. Non Pre-eng		1.00	Service & distribution	9,124.00	GSF	7.00	63,868.00	
C. Non Pre-eng		2.00	Lighting & controls, incl LED	9,124.00	GSF	12.00	109,488.00	
C. Non Pre-eng		3.00	Branch power devices & wiring	9,124.00	GSF	1.75	15,967.00	
C. Non Pre-eng		4.00	AV/Communication RI only	9,124.00	GSF	2.00	18,248.00	
C. Non Pre-eng		5.00	Fire alarm	9,124.00	GSF	2.50	22,810.00	
C. Non Pre-eng		6.00	Security system	9,124.00	GSF	3.00	27,372.00	

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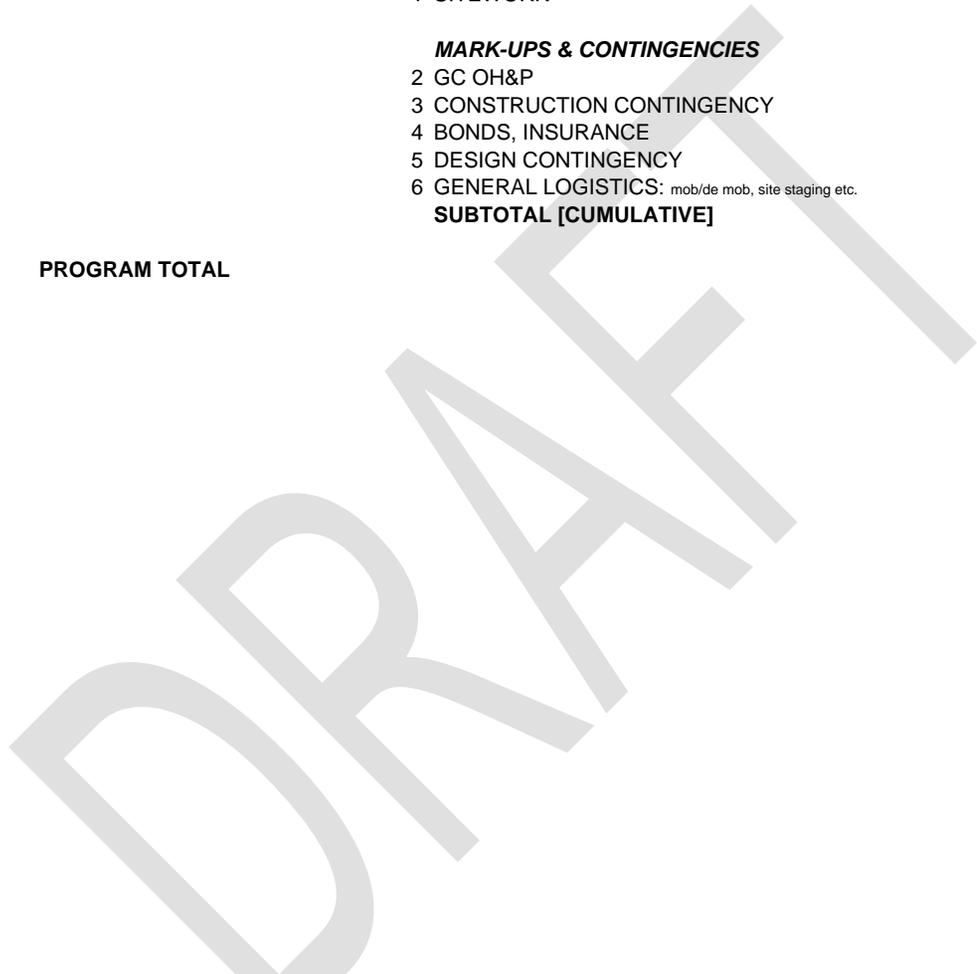
E	EQ & FURNISHINGS		System Subtotal	9,124.00	SF	10.90		99,492.00
			Basics					
	C. Non Pre-eng	1.00	Lockers / cubbies	100.00	EA	250.00	25,000.00	
	C. Non Pre-eng	2.00	Casework, millwork, etc.	9,124.00	GSF	3.00	27,372.00	
	C. Non Pre-eng	3.00	Foldable partition	480.00	SF	38.00	18,240.00	
	C. Non Pre-eng	4.00	Drymarker boards and tack boards	2.00	Rooms	1,440.00	2,880.00	
			Special					
	C. Non Pre-eng	1.00	Athletic equipment allowance	5,200.00	Gym SF	5.00	26,000.00	
F	SPECIAL		System Subtotal				Not used	
Z	GENERAL		Subtotal A-F					1,966,308.10
	C. Non Pre-eng	1.00	Field Overhead				137,641.57	
	C. Non Pre-eng	2.00	Subtotal				2,103,949.67	
	C. Non Pre-eng	3.00	GC OH&P, ref Summary				0.00	
	C. Non Pre-eng	4.00	Subtotal				2,103,949.67	
	C. Non Pre-eng	5.00	Bonds & Insurance, ref Summary				0.00	
	C. Non Pre-eng		SUBTOTAL	9,124.00	GSF	230.60		2,103,949.67

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COST SUMMARY: SITE ANALYSIS: ALL OPTIONS

HARD CONSTRUCTION COSTS	BUILDINGS							
	1 SITEWORK			1	LS		4,721,812.05	4,721,812.05
			MARK-UPS & CONTINGENCIES					
	2 GC OH&P				4.00%		188,872.48	4,910,684.53
	3 CONSTRUCTION CONTINGENCY				3.00%		147,320.54	5,058,005.06
	4 BONDS, INSURANCE				1.50%		75,870.08	5,133,875.14
	5 DESIGN CONTINGENCY				7.00%		359,371.26	5,493,246.40
	6 GENERAL LOGISTICS: mob/de mob, site staging etc.				1.00%		54,932.46	5,548,178.86
	SUBTOTAL [CUMULATIVE]							5,548,178.86
PROGRAM TOTAL								5,548,178.86



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SITWORK

0.00	DATA BOX		Site Statistics					
			Disturbed area work zones	509,652.00	SF		11.70 Acres	
			Total site [published area]	566,280.00	SF		13.00 Acres	
Sitework	ENVIRONMENTAL CONTROLS	1.00	Erosion, sedimentary cntrls, disturbed area	11.70	AC	5,500.00	64,350.00	64,350.00
Sitework	SITE DEMO	1.00	General site demolition	509,652.00	SF	0.50	254,826.00	254,826.00
Sitework	EARTHWORK	1.00	General grading	509,652.00	SF	0.55	280,308.60	280,308.60
Sitework		3.00	Rock or unsuitable soils		Excluded			
Sitework	PRIMARY UTILITIES: WET	1.00	Water, sanitary sewer	1.00	LS	409,500.00	409,500.00	409,500.00
Sitework	PRIMARY UTILITIES and SERVICES: DRY	1.00	Incoming service	1.00	LS	100,000.00	100,000.00	611,000.00
Sitework		2.00	Incoming communications ductbanks	1.00	LS	50,000.00	50,000.00	
Sitework		3.00	Incoming gas					Assume brought to meter by gas company
Sitework		4.00	Emrgncy generator, WP enclosure, fuel tank	1.00	LS	250,000.00	250,000.00	
Sitework		5.00	General site ltg lamp, pole, foundns, circuitg	42.00	EA	3,500.00	147,000.00	
Sitework		6.00	Athletic field night lighting		Excluded			
Sitework		7.00	Walking path lighting	32.00	EA	2,000.00	64,000.00	
Sitework	SWM	1.00	SWM: Detention / retention, piping, struc-	1.00	LS	877,500.00	877,500.00	877,500.00
Sitework		2.00	tures, filtration, bio swales					
Sitework	SITE CONCRETE	1.00	Sidewalks, new	47,108.25	SF	7.00	329,757.75	862,252.75
Sitework		2.00	Concrete ramps	4.00	LOC	5,000.00	20,000.00	
Sitework		3.00	Concrete steps	210.00	LF	50.00	10,500.00	
Sitework		4.00	Amphitheatre paving	13,965.00	SF	8.00	111,720.00	
Sitework		5.00	Amphitheatre tiered seating	472.50	LF	150.00	70,875.00	
Sitework		6.00	Outdoor classroom					Included with green roof
Sitework		7.00	Retaining wall allowance	1.00	LS	80,000.00	80,000.00	
Sitework		8.00	Walking paths, concrete	4,200.00	SF	7.00	29,400.00	
Sitework		9.00	Paved play area	21,000.00	SF	10.00	210,000.00	

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Sitework	BITUMINOUS PVMNT includin	1.00	Mill, overlay existing bituminous paving	0.00	SY	35.00	0.00	522,640.00
Sitework	C&G	2.00	New paving	10,066.00	SY	40.00	402,640.00	
Sitework		3.00	C&G	6,000.00	LF	20.00	120,000.00	
Sitework	LANDSCAPING	1.00	Plant materials	1.00	LS	120,000.00	120,000.00	202,679.06
Sitework		2.00	Seed and sod	23,722.86	SY	2.00	47,445.72	
Sitework		3.00	Soccer field	8,808.33	SY	4.00	35,233.33	
Sitework	MISCELLANEOUS	1.00	Canopy at entrance		None indicated			207,500.00
Sitework		2.00	Signage/bollards/misc, etc.	1.00	LS	50,000.00	50,000.00	
Sitework		3.00	Field equip	1.00	LS	40,000.00	40,000.00	
Sitework		4.00	Site furnishings: benches, bike racks, etc.	35.00	EA	2,000.00	70,000.00	
Sitework		5.00	Waste receptacles	10.00	EA	750.00	7,500.00	
Sitework		6.00	Play area equipment	1.00	LS	40,000.00	40,000.00	
Sitework		7.00	Offsite		Excluded			
Sitework		1.00	Subtotal					4,292,556.41
Sitework		2.00	Field Overhead Incl Phasing Premium				429,255.64	
Sitework		3.00	Subtotal				4,721,812.05	
Sitework		4.00	CM Fee, ref Summary				0.00	
Sitework		5.00	Subtotal				4,721,812.05	
Sitework		6.00	Bonds & Insurance, ref Summary				0.00	
Sitework		7.00	SUBTOTAL				1.10	4,721,812.05

NOTES

Important Note: Forella Group, LLC disclaims any warranties expressed or implied with respect to this estimate and any information or data contained herein.

Additional Notes and Clarifications



DRAFT

ADDITIONAL NOTES & EXCLUSIONS

There are numerous *soft costs* and *secondary scope* issues with cost implications associated with a construction project today. The following can be a useful way to help verify that all of your project costs have been addressed. Unless noted otherwise, none of the costs listed below have been included in our computations.

A. REAL ESTATE ACQUISITIONS & LEASING

- Due diligence fees and expenses
- Real estate acquisitions and/or leases, including those pertaining to any necessary easements and rights of way.
- Settlement charges, fees, taxes, transfer and/or recordation fees
- Brokerage commissions

B. PROJECT & CONTRACT MANAGEMENT

- Development fees
- Project / contract management costs and expenses
- Communications, telephones, cell phones, web services, facsimile expenses, e-mail, long distance telephone expenses, etc.
- Travel, parking, courier services, office equipment, office supplies, security fees and expenses
- Reprographics expenses
- Messenger and overnight expenses

C. FINANCIAL

- Financial feasibility analyses
- Construction and interim financing fees, expenses and interest
- Permanent financing fees, expenses, interest, bonds
- Fees and expenses related to special government programs
- Accounting both internal and external
- Appraisal fees
- Start-up working capital to cover initial operating deficit

D. INSURANCE

Insurance premiums purchased at appropriate limits for the following categories. Note that we recommend that the A.M. Best Company ratings be A [minus] or above.

- General liability insurance*
- Professional liability insurance*
- Excess liability or umbrella insurance
- Bonds, builder's risk insurance*
- Moving and storage insurance
- Title insurance
- Worker's compensation insurance*
- Auto insurance
- Pollution, hazardous materials liability insurance

* Construction Managers insurance has been included in our cost estimate

E. LEGAL

- Legal services related to acquisitions and title
- Legal services related to zoning, subdivision, use and proffers
- Legal services related to partnership and joint venture agreement preparations and reviews
- Legal services related to financing
- Legal services related to contract preparation and reviews
- Legal services related to leasing document preparation and reviews

F. REGULATORY PROCESSES

- Site and building permit fees & expenses
- Fees and expenses pertaining to special zoning and uses
- Primary water, sewer, gas, power, communications fees and expenses
- State and local highway fees, bonds
- On and off site improvements or contributions mandated by regulatory agencies as a condition of their approvals.

G. DESIGN FEES & EXPENSES

- Field surveys [note: lenders may have certification standards regarding field surveys]
- Civil engineering fees and expenses
- Architectural fees and expenses
- Structural engineering fees and expenses
- Mechanical engineering fees and expenses
- Electrical engineering fees and expenses
- Traffic consultant's fees and expenses
- Acoustical engineering fees and expenses
- Lighting consultant's fees and expenses
- Testing & inspections
- Permit expeditor

H. PROPERTY MANAGEMENT, OPERATIONS & MAINTENANCE

- Property management fees and expenses
- Operations and maintenance costs

I. MARKETING, PUBLIC RELATIONS & ADVERTISING

- Consultant's fees for market analyses, strategies, public relations, advertising and merchandizing
- Expenses related to promotional photography, graphics, artwork, reproduction, postage, signage, etc.
- Promotional events, hearings, fundraisers, etc.

J. MOVING & STORAGE COSTS

- Moving and storage fees and expenses
- Hauling and disposal expenses that can occur during and following a move

K. TEMPORARY FACILITIES

- Temporary owner/user office facility leases or purchases
- Temporary owner/user utilities fees and charges, etc.
- Temporary owner/user furniture, fixture & equipment

L. MISCELLANEOUS

Construction Contingency: This contingency budgets for change orders and / or additional costs charged by the contractor after the construction contract award.

Owner Paid Inspections and Testing: We have not included inspections and testing costs called for in the specifications. Owners can require additional inspections and testing over and above those required of the contractor.

Undelineated Issues: Unless noted otherwise, we have not included costs that have not be specified or delineated on the subject documents.

Existing Conditions: Unless noted otherwise, we have not included costs pertaining to wetland issues, geotechnical issues, archeological finds or hazardous materials.

Furniture, fixtures & equipment [F.F. and E.]: We have not included owner or user required items that are not permanently attached or fastened to the facility or part of the general contract for construction.

OPINION OF PROBABLE COST

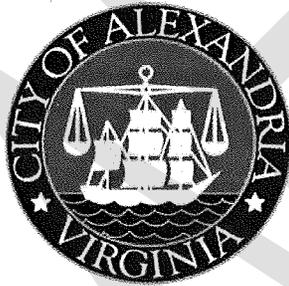
Controlling cost, schedule and quality requires on going processes that commence at the programmatic phase and continue through to final acceptance and building occupancy. It should be noted that we exercise no control over fluctuating market conditions. We have employed our best judgment in analyzing the subject project. We cannot, however, guarantee that actual costs will not vary from the opinions we have provided.

DRAFT

8.3 Previous Reports

- A. 2008 Patrick Henry Recreation Center Feasibility Study
- B. 2014 Patrick Henry Facility Survey
- C. RPCA Field House Study - Brailsford & Dunlavy

**PATRICK HENRY RECREATION CENTER
FEASIBILITY STUDY**



October 2008

THE LUKMIRE PARTNERSHIP/ S3E KLINGEMANN/ A. MORTON THOMAS

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- F. ELECTRICAL**
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VI. COST ESTIMATE

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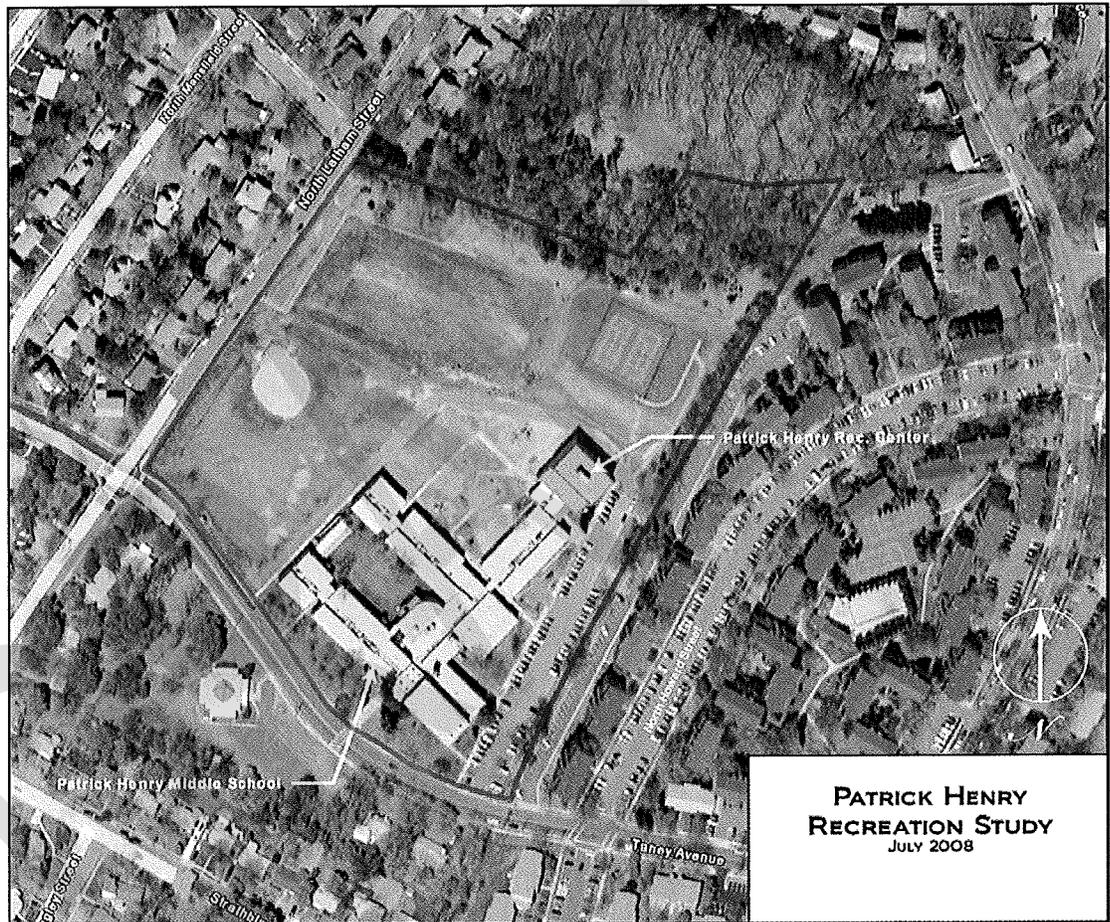
- SEPT. 2008 IDR MEETING MINUTES**
-

I. SCOPE OF WORK

A. BACKGROUND

The Arcadis/Lukmire Partnership team has been tasked with a feasibility study to determine the space needs and possibilities for expanding the Patrick Henry Recreation Center. The first part of that investigation is an analysis of the existing conditions of both the site and the recreation center.

Included in this report is that analysis along with Proposed Space Needs.



II. EXISTING CONDITIONS

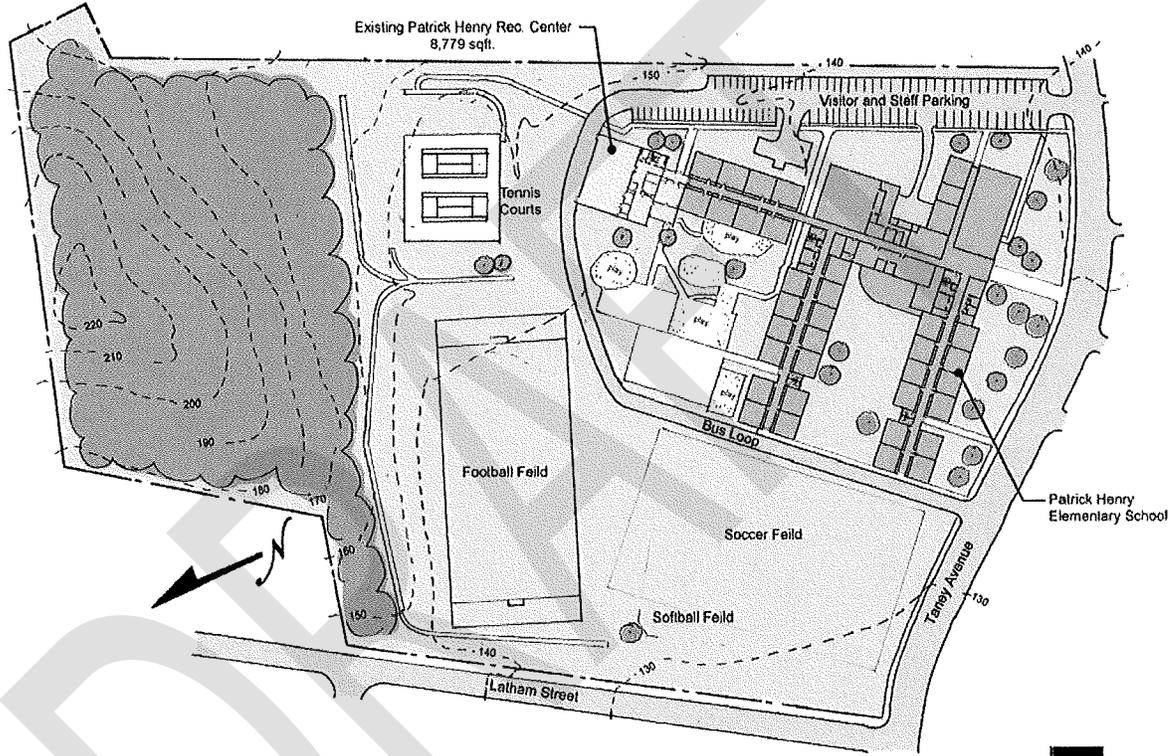
A. CIVIL

The Patrick Henry school and Recreation Center site is co-located on 14.9 acres between Taney Avenue and Latham Street in Alexandria. The site contains the Patrick Henry Elementary School originally constructed in 1953 and expanded in 1995 and the Patrick Henry Recreation Center, originally constructed in 1973 and added to in 1990. In addition there are two playing fields used by the school and public, a tennis court which is located on a hill to the north of the recreation center, and a playground located immediately adjacent to the school. A ring road wraps around the school providing a bus loop as well as access for emergency vehicles. An 89 car parking lot serving both the school and recreation center is located on the east side of the facility.

The school, recreation center, parking lot and playing fields occupy the entire "flat" area of the site, and the tennis court occupies a site 10' above the level of the recreation center. The remaining site area to the north is wooded and sloped and has been left in its natural state.

II. EXISTING CONDITIONS

A. CIVIL - continued



Patrick Henry Recreation Center
Existing Site Plan



II. EXISTING CONDITIONS

SITE PHOTOS



Gym & Hillside to tennis court



Tennis Court



Gym & School from tennis court



Gym & School looking from outdoor play area

II. EXISTING CONDITIONS

SITE PHOTOS



Parking Lot



Entry



School & Recreation Center looking from upper field



Upper (north) field

II. EXISTING CONDITIONS

A. CIVIL

A. Morton Thomas has reviewed the existing conditions at the Patrick Henry Recreation Center site to identify constraints associated with the future expansion of the facility.

1. Zoning: The site is currently zoned R-12. The bulk regulations for the R-12 District are as follows:

Yard Requirements:

Front Yard: Each use shall provide a front yard of at least 35 feet

Side Yards Each residential use shall provide two side yards, each based on a setback ratio of 1:2 and a minimum size of ten feet. Each other use shall provide two side yards, each based on a setback ratio of 1:1 and a minimum size of 25 feet.

Rear Yard Each residential use shall provide a rear yard based on a setback ratio of 1:1 and a minimum size of ten feet. Each other use shall provide a rear yard based on a setback ratio of 1:1 and a minimum size of 25 feet.

Floor Area Ratio:

The maximum permitted floor area ratio is 0.30

Height:

The maximum permitted height of a structure is 35 feet except for a church or school use in which case the maximum permitted height is 40 feet.

While Permitted uses in the R-12 district include a public park and a public school, a recreation center requires a Special Use Permit (SUP). This is significant due to the amount of time that will be added to the project to go through a review process associated with a SUP.

2. Stormwater management. Due to the fact that this project will result in an area of land disturbance that exceeds 2,500 square feet, it will be necessary to comply with the provisions of section 13-109 of the City of Alexandria Zoning Ordinance - General Performance Requirements for Chesapeake Bay Preservation Areas. This section requires that stormwater peak runoff rate from the site after development is equal to or less than that which existed prior to development. In order to address this requirement it will be necessary to provide some type of "stormwater detention" facility that stores or "detains" stormwater runoff and then releases it at a rate that is at or below the existing rate. There are several different types of facilities that could be used to meet this requirement. An above ground "detention pond" is generally considered the least
-

II. EXISTING CONDITIONS

A. CIVIL - continued

2. Stormwater Management - continued
expensive option, but it occupies a significant amount of site area that could be used for other purposes. Another option to address this requirement is an underground facility. There are several different types of underground facilities that can be used for this purpose. These facilities are more expensive, but they generally can be placed beneath site improvements and, therefore, do not reduce the developable area of the site. If the disturbed area of the site can be limited to one-half acre or less, it may be possible to get a waiver of this requirement.

This section of the Zoning Ordinance also requires that stormwater runoff quality controls be provided. These controls are generally referred to as BMPs which stands for Best Management Practices and there are a large number of different types available. They range from vegetative type practices such as a bioretention basin or filter (rain garden) to manufactured systems such as "storm filters" to combination systems such as "filtrerras". In order to comply with this requirement, it will be necessary to treat the entire water quality volume (WQV) from the site. The WQV is defined as the first one-half inch of runoff from the impervious areas on the site which is 1,816 cubic feet per impervious acre. This site contains 3.85 acres of impervious area which is comprised of 1.69 acres of buildings, 1.11 acres of parking lot and 1.05 acres of walks and play areas. The resultant WQV to be treated is approximately 7,000 cubic feet (1816 X 3.85). Based on our review of the available information, it appears that a combination of bioretention filters and an extended detention dry pond would be an economical way to address the requirements. The bioretention filters can be used to address WQV requirements and the extended detention dry pond can be used to address WQV and stormwater detention requirements. The size of each of the facilities will be determined by the size of the drainage area that they serve. A bioretention filter is limited to a ponding depth of one foot, so its surface area will be equal to the WQV for the drainage area that it serves. Therefore, if the entire WQV were to be treated by bioretention filters they would have a total surface area of 7,000 square feet. As noted above, the extended detention pond can serve two purposes, storage of the WQV and stormwater detention. Unlike the bioretention filters, there is no depth limitation for storing the WQV in the pond and, therefore, there is not a 1:1 relationship between the WQV and the surface area as there is with the bioretention filters which allows for a smaller surface area. However, the pond must also store the volume of stormwater necessary to reduce the flow rate enough to match the existing rate.

II. EXISTING CONDITIONS

A. CIVIL - continued

3. **Parking:** The City of Alexandria Zoning Ordinance does not list a parking requirement for recreation centers. However, it does list a parking requirement for "Amusement Enterprise (Indoor)". The requirement for this particular use is one (1) space for each two hundred (200) square feet of floor area. Based on a facility of approximately 17,000 SF, 85 parking spaces are required. The existing parking lot has 89 parking spaces that are used by both the elementary school as well as the recreation center. The requirement for a school is 1 space per 25 classroom seats. There are 32 classrooms, so 32 spaces are required. Therefore, without any shared usage, 117 parking spaces are required, or 28 more than exist in the parking lot.

Currently the school and recreation center are not open at the same time so the 89 parking spaces have met the needs of the recreation center as well as the school. However, there are plans to allow both facilities to be open at the same time which may trigger the need for 28 additional parking spaces. Alternatively, a parking study can be conducted to determine if there are sufficient spaces for shared use.

4. **Landscaping:** There are three distinct requirements relating to landscaping. The first of these is crown coverage area. The crown coverage area for the site must be equal to or greater than 25 percent of the total land area. The crown coverage can be made up of a combination of existing and planted vegetation. The second requirement is for street trees. Street trees are required within the exiting rights-of-way that abut the site. There needs to be one shade tree for every 30 linear feet of frontage and one ornamental tree for every 20 linear feet of frontage. The last requirement is for parking lot landscaping. There are 2 types of landscaping related to parking. The first is parking lot screening. Everywhere that there is parking adjacent to a street, there must be a 6-foot landscape strip between the parking lot and street right-of-way. The second is internal parking lot landscaping. There needs to be a landscape island at least equal to the size of a parking space for each ten parking spaces and approximately every one-hundred linear feet of parking row.
 5. **Soils:** Following is a soils map with descriptions.
-

II. EXISTING CONDITIONS

A. CIVIL - continued



 Site

Soils

 Hydric Soils

 Soils with Hydric Inclusions

 Non-hydric & Unknown-Hydric Soils

Soils Map
USDA Digital Data
Patrick Henry School/Rec Center
Scale: 1" = 200'

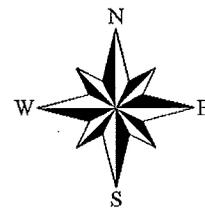


Photo Source: City of Alexandria Spring 2007 Natural Color Imagery

Wetland Studies and Solutions, Inc.

II. EXISTING CONDITIONS

A. CIVIL - continued

Map Unit Description

Alexandria City, Virginia

[Minor map unit components are excluded from this report]

Map unit: 47B - Grist Mill-Woodstown complex, 2 to 7 percent slopes

Component: Grist Mill (45%)

The Grist Mill component makes up 45 percent of the map unit. Slopes are 0 to 20 percent. This component is on marine terraces on coastal plains. The parent material consists of Earthy fill of fluviomarine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately low. Available water to a depth of 60 inches is moderate. Shrink-swell potential is moderate. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 40 inches during January, February, March, April, May, November, December. Organic matter content in the surface horizon is about 0 percent. Nonirrigated land capability classification is 2e. This soil does not meet hydric criteria.

Component: Woodstown (40%)

The Woodstown component makes up 40 percent of the map unit. Slopes are 2 to 7 percent. This component is on terraces on coastal plains. The parent material consists of fluviomarine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is moderately well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is moderate. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 30 inches during January, February, March, April. Organic matter content in the surface horizon is about 2 percent. Nonirrigated land capability classification is 2e. This soil does not meet hydric criteria.

Map unit: 66 - Kingstowne sandy clay loam, 0 to 45 percent slopes

Component: Kingstowne (100%)

The Kingstowne component makes up 100 percent of the map unit. Slopes are 0 to 45 percent. This component is on marine terraces on coastal plains. The parent material consists of Earthy fill of fluviomarine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately low. Available water to a depth of 60 inches is moderate. Shrink-swell potential is moderate. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 40 inches during January, February, March, April, May, November, December. Organic matter content in the surface horizon is about 0 percent. Nonirrigated land capability classification is 2e. This soil does not meet hydric criteria.

Map unit: 71C - Kingstowne-Sassafras-Marumsc complex, 7 to 15 percent slopes

Component: Kingstowne (45%)

The Kingstowne component makes up 45 percent of the map unit. Slopes are 0 to 20 percent. This component is on marine terraces on coastal plains. The parent material consists of Earthy fill of fluviomarine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately low. Available water to a depth of 60 inches is moderate. Shrink-swell potential is moderate. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 40 inches during January, February, March, April, May, November, December. Organic matter content in the surface horizon is about 0 percent. Nonirrigated land capability classification is 2e. This soil does not meet hydric criteria.

Component: Sassafras (23%)

The Sassafras component makes up 23 percent of the map unit. Slopes are 7 to 15 percent. This component is on coastal plains, terraces. The parent material consists of fluviomarine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is moderate. Shrink-swell potential is low. This soil is not flooded. It is not ponded. There is no zone of water saturation within a depth of 72 inches. Organic matter content in the surface horizon is about 2 percent. Nonirrigated land capability classification is 3e. This soil does not meet hydric criteria.

Component: Marumsc (22%)

The Marumsc component makes up 22 percent of the map unit. Slopes are 7 to 15 percent. This component is on terraces on coastal plains. The parent material consists of fluviomarine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is moderately well drained. Water movement in the most restrictive layer is moderately low. Available water to a depth of 60 inches is moderate. Shrink-swell potential is high. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 15 inches during January, February, March, November, December. Organic matter content in the surface horizon is about 2 percent.

II. EXISTING CONDITIONS

A. CIVIL - continued

Map Unit Description

Alexandria City, Virginia

Map unit: 71C - Kingstowne-Sassafras-Marumsko complex, 7 to 15 percent slopes

Component: Marumsko (22%)
Nonirrigated land capability classification is 3w. This soil does not meet hydric criteria.

Map unit: 95 - Urban land

Component: Urban Land (95%)

Generated brief soil descriptions are created for major soil components. The Urban Land is a miscellaneous area.

Map unit: 109B - Woodstown sandy loam, 2 to 7 percent slopes

Component: Woodstown (85%)

The Woodstown component makes up 85 percent of the map unit. Slopes are 2 to 7 percent. This component is on terraces on coastal plains. The parent material consists of fluviomarine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is moderately well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is moderate. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 30 inches during January, February, March, April. Organic matter content in the surface horizon is about 2 percent. Nonirrigated land capability classification is 2e. This soil does not meet hydric criteria.

II. EXISTING CONDITIONS

B. ARCHITECTURAL

The current Patrick Henry Recreation Center is located at 4643 Taney Ave., Alexandria Virginia. The facility is approximately 8,700 square feet and includes a meeting room, game room, kitchen, lounge, office, storage, and elementary size gym facility. The facility is located on the north western portion of the property and is directly attached to the Patrick Henry Elementary School. The entire facility is encircled by a fire access road that doubles as a bus loop for drop off and pick up of school age children.

The site contains two playing fields, one football field and a soccer field. Both fields serve both the school and groups within the community who use them for softball and lacrosse games. However, the playing fields are not available for use during the day by the recreation center since the school uses them for physical education.

To the north of the fields, there is a steep sloped area that separates a very heavily wooded area on the northern end of the site from the rest of the site. Within this area are tennis courts located on a hill behind the existing gym.

The existing gym is shared by the recreation center and the elementary school. Since the gym is used by the school, the recreation center can not offer programming in the space during school hours. The gym was sized for elementary school use which limits the activities that the recreation center can offer as there isn't a full size basketball court. The gym is also not divisible which would allow multiple functions to occur in the same space. The kitchen shares space with what is also currently the conference room. The space is not of adequate size for either function. The kitchen consists of an oven with range top, sink and refrigerator. There is also a lack of a computer center in the facility. The school has allowed the recreation center access to their computer room after school hours but requires staff to monitor activity. The overall flow of the building is not ideal and in some cases program space is only accessible by traversing through other program space.

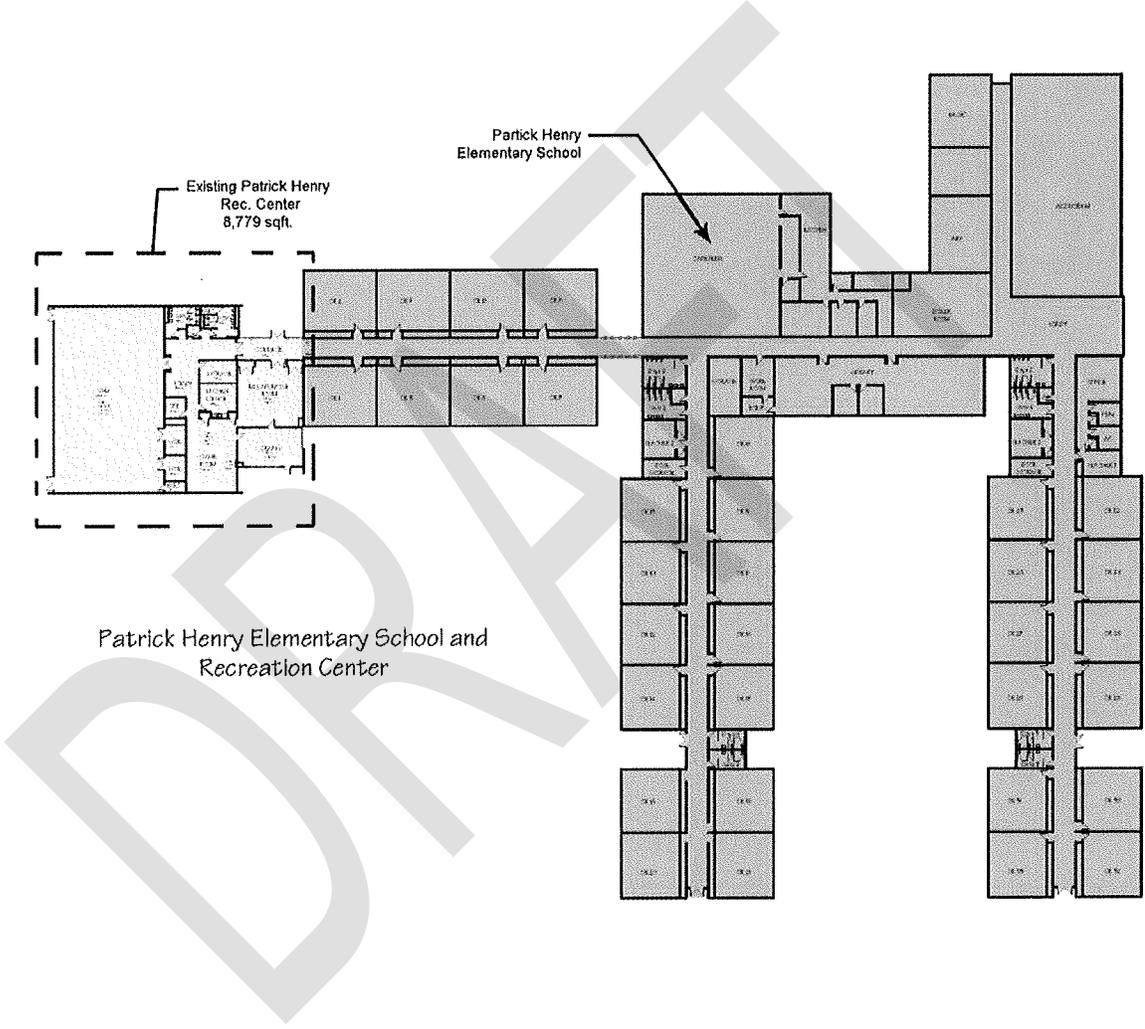
There is a concern in the existing facility over security as the main entrance is not visible from the main office and there is no sign in or reception desk. There is also a lack of a lobby area to allow for groups to gather before using the gym or other facilities. In addition, exterior site lighting is minimal and should be enhanced.

The current restrooms do not meet ADA (American with Disabilities Act) requirements as they do not have a handicap stall or meet code compliant clearances.

The facility has 8'-0" ceiling heights throughout with the gymnasium ceiling being 25'-0" in height. Interior finishes consist of painted CMU walls, acoustic lay-in ceiling grids with 2'x 4' florescent light fixtures, and vinyl floor tiles. The gymnasium has painted CMU walls with acoustic wall panels and an exposed painted structure with acoustic treatment.

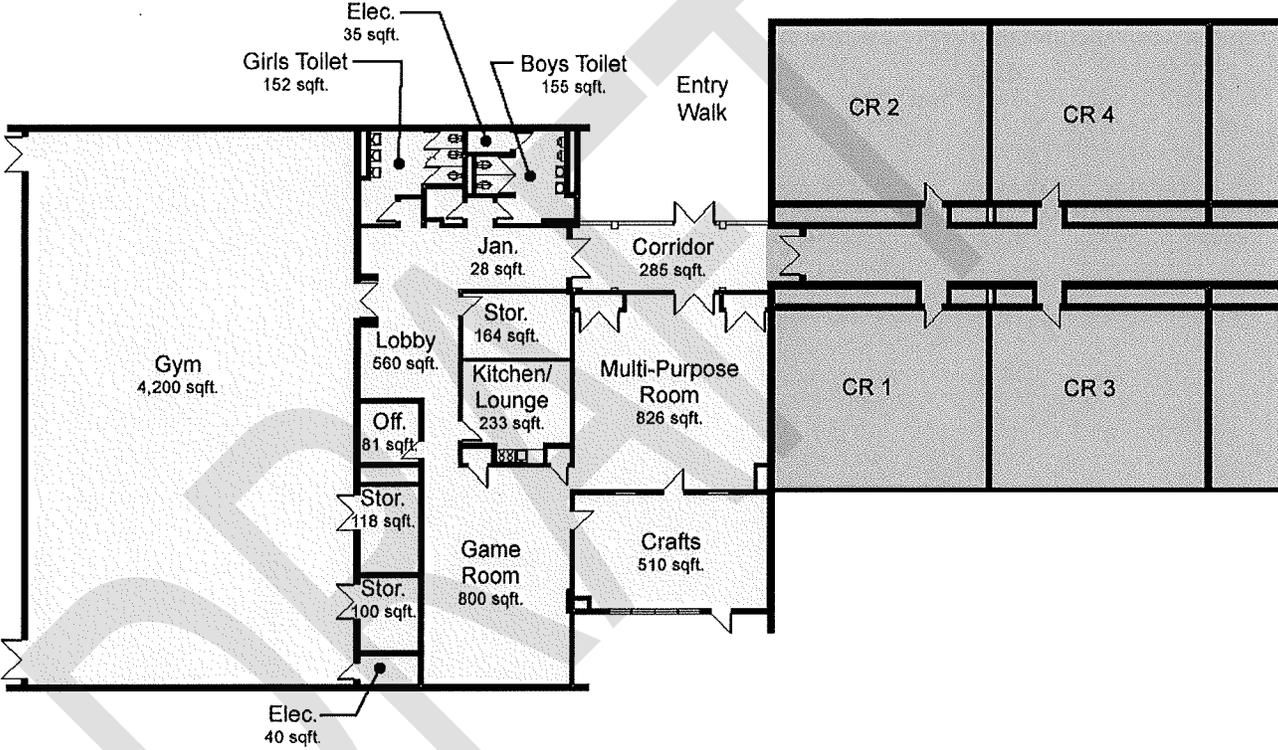
II. EXISTING CONDITIONS

C. ARCHITECTURAL



II. EXISTING CONDITIONS

C. ARCHITECTURAL



Existing Patrick Henry Recreation Center

II. EXISTING CONDITIONS

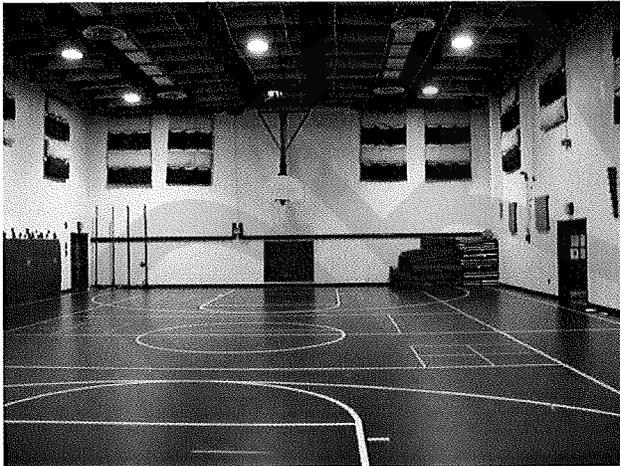
PATRICK HENRY RECREATION CENTER - INTERIOR



Lobby looking toward gym



Lobby looking toward school



Gym



Multi-purpose room

II. EXISTING CONDITIONS

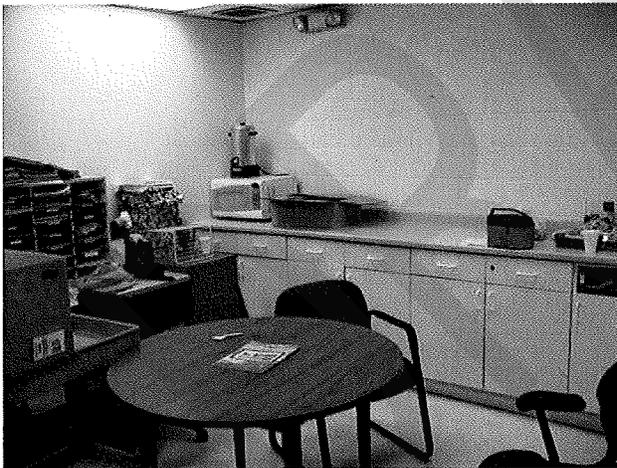
PATRICK HENRY RECREATION CENTER - INTERIOR



Arts and Crafts Room



Game Room



Kitchen/Conference Room



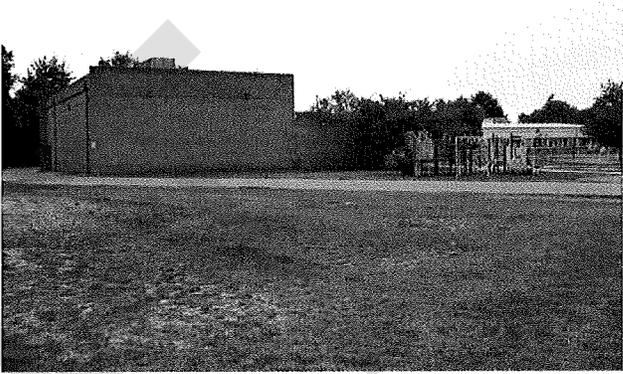
Storage Room

II. EXISTING CONDITIONS

PATRICK HENRY RECREATION CENTER - EXTERIOR



Entry



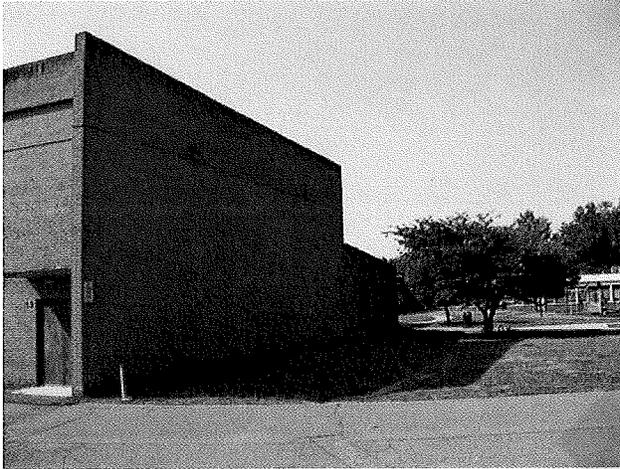
Gym from play fields



Gym & Playground

II. EXISTING CONDITIONS

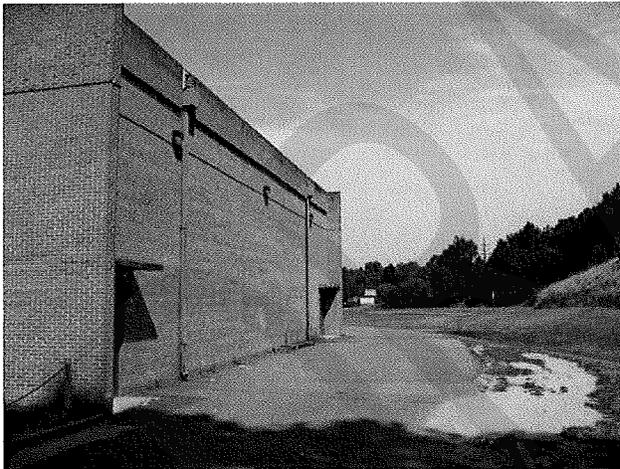
PATRICK HENRY RECREATION CENTER - EXTERIOR



Gym – North side



School looking toward crafts room



Gym – East side



Exterior of crafts & game room

Exterior of crafts & game room

II. EXISTING CONDITIONS

C. STRUCTURAL

The existing building is a single story structure of CMU bearing walls and structural steel framing with areas of flat and sloping roofs. The building plan shape is a series of rectangles formed by the bearing walls. Roof heights vary with the highest over the gymnasium. A portion of the building appears to be an infill between the gymnasium structure and school building. Drawings for the infill were available for review. These drawings are dated May 1990.

The overall roof framing consists of standard steel bar joists spanning between the exterior and interior CMU bearing walls. The infill area also includes steel wide flange girders along the two exterior walls to support the roof joists. The majority of the exterior walls is solid CMU and includes brick façades. The roof joists are placed at some 5'-0" o.c. and covered by a painted metal roof deck. Joist sizes vary depending upon the spans. The bearing walls are 8" or 12" thick CMU depending upon location and height. Foundations are spread footings with safe soil bearing pressures of 2000 psf noted on the infill documents. The lack of any significant cracking noted in the masonry suggests a stable foundation. The slab on grade is noted as 4" thick in the infill documents. The existing structure **appears sound** but the bearing wall system limits flexibility as walls are not easily relocated without significant structural impact and cost.

II. EXISTING CONDITIONS

D. MECHANICAL

The HVAC system for the recreation center consists of three gas fired rooftop units with DX cooling. The unit that serves the gymnasium is original to the recreation center construction and the other two units were installed when the 1990 modernization was done. One of these units serves the Craft and Multipurpose rooms and the other serves the Game Room. All of the units appeared to be functional however they are at the end of their useful life for this type of equipment. In addition to the rooftop units there is electric baseboard radiation in toilet rooms and a central exhaust fan that serves these rooms which is located on the roof. The HVAC controls consist of time clocks and package controllers by the equipment manufacturers.

E. PLUMBING

The domestic water service for the building appears to be coming from the school. A 2-inch service enters the recreation center under the floor and rises in a closet that is adjacent to the Game Room. The domestic water service supplies the toilet fixtures and a limited area sprinkler system that serves the storage rooms. The plumbing fixtures are original to the building but appear to be in fair condition. There is an electric water heater located in the janitor's closet adjacent to the toilet rooms that supplies the lavatories in the toilet rooms. There is a dedicated gas service and utility company meter for the recreation center that enters the front of the building.

F. ELECTRICAL

The power company serving this facility is Dominion VA Power. The facility is served with two electrical services which is a code violation per the National Electric Code (NEC), Article (Art.) 230. The first service is fed through a pad mounted utility transformer. The transformer is located in the front of the building. The enclosure is rusted and the cooling fins on the enclosure are damaged. This service is 350 amps, 480/277 volts, 3 phase, and 4 wire. The power company CT cabinet and meter are in an electrical room/closet accessed from within the men's toilet room. This room also includes Federal Pacific power and lighting electrical panels. Federal Pacific has been out of business for over 30 years and replacement parts and breakers will be difficult to obtain. This electrical room is approximately 4 feet wide with live electrical components on both opposing walls. This is a code violation per NEC, Art. 110, for minimum working clearance requirements. This electrical room also has a step down transformer that feeds a 208/120 volt panel. This electrical room serves interior lighting, receptacles, water heater and one of the roof top HVAC units.

II. EXISTING CONDITIONS

F. ELECTRICAL - CONTINUED

The second service is located on the opposite side of the facility in a closet off of the gymnasium. This service is fed through pole mounted utility transformers at the property line and continues underground to the building. This service is 200 amps, 208/120 volts, 3 phase, and 4 wire. This electrical room serves exterior lighting at basketball and tennis courts, the two of the three roof top HVAC units, fire alarm system and a heat pump. This electrical room houses the fire alarm control. This electrical room also houses time clocks and lighting contactors for exterior lighting control.

FIRE ALARM SYSTEM

The fire alarm system is located in the electric room adjacent to the Gymnasium. The current system is Simplex Model 4002. The current fire alarm control panel would have to be upgraded to handle additional devices that are required by current code requirements. There are single action pull stations at the exits. In this type of facility we recommend that these devices be double action, requiring person to stop and perform two distinct actions, to help deter pranksters from initiating costly alarms. Notification appliances are through horns and flashing lights. The existing flashing lights do not meet current code or ADA requirements. In addition, the locations of the notification appliances do not meet current code location and spacing requirements.

LIGHTING

The interior lighting is a combination of recessed and surface mount fluorescent fixtures. These fixtures utilize T12 technology which is outdated and are not energy efficient. Some of the fixture lenses are missing. The exterior lighting is old. All lenses are discolored and some are broken. These fixtures are fed from surface mounted conduit. Emergency lighting is through battery packs. The gymnasium has HID lighting.

RECEPTACLES

The facility has original receptacles that are recessed in the block walls. Surface mounted receptacles with surface mounted conduit have been added since the original construction of the building.

AUXILIARY SOUND SYSTEM

The gymnasium has an auxiliary sound system. There are six speakers spaced throughout the gymnasium.

III. SPACE NEEDS

Patrick Henry Recreation Center - Space Needs Program			
Space	Exist SF	Program	Notes
Lobby	560	500	
Administration	477	848	Offices, Conf., Break Room, Copy, Files, Stor - 6 people
Gymnasium & Storage	4,418	4,418	
Shared Program Space			
Multi-Purpose Room & Stor	854	1,280	Comm. Meetings, Edu. Programs, Workshops, Music, and Preschool
Kitchen	0	250	Cooking classes, Preparation/warming area for Receptions
Adult Program Room	0	600	Educational Programs, Classes, Drama, Cooking Instructions
Crafts Room	510	850	Arts and Crafts, Painting - 20 people
Weight / Fitness Room	0	1,200	Nautilus Equip., Free Weights - 10-15 people
Game Area	800	1,200	Adjacent to Lobby & Reception Desk, Pool, Ping Pong, etc - 20 people
Dance Room	0	700	Dance classes, Exercise Classes, Yoga - 15-20 people
Library / Computer Room	0	300	5-7 PCs, Tutoring, Homework, Reading, Laptop Use
Mens Toilet	155	350	Toilet to include changing area, 1 shower and 10 lockers
Womens Toilet	155	350	Toilet to include changing area, 1 shower and 10 locker
Total Net SF	7,929	12,846	
Walls, Circulation, M & E	833	3,854	30% of subtotal
Total Gross SF	8,762	16,700	

IV. RECOMMENDATIONS

A. DESIGN APPROACH

The City has made the decision to enlarge the existing recreation center rather than construct a new facility. There are several reasons for this:

1. The existing school gym can serve both the school and recreation center.
2. The play fields are left intact and serve as a buffer between the neighborhood and school/recreation center.
3. Parking can be shared between the school and recreation center.
4. There is the potential for shared facilities between the school and recreation center.
5. The existing structure is sound and can be renovated to become part of a new enlarged facility.
6. The HVAC and electrical systems are adaptable and can be used in an enlarged facility.

Two options have been illustrated. They share several attributes:

1. Access from the school to the gym is possible without going through the recreation center.
2. The recreation center can operate independently of the school enabling it to offer day time activities.
3. The existing gym has an independent access from the recreation center.
4. The plan provides for a future, larger gym to be added.
5. The playing fields are either not or minimally affected.
6. The existing parking lot can be shared.
7. The playground is easily accessible from both uses.

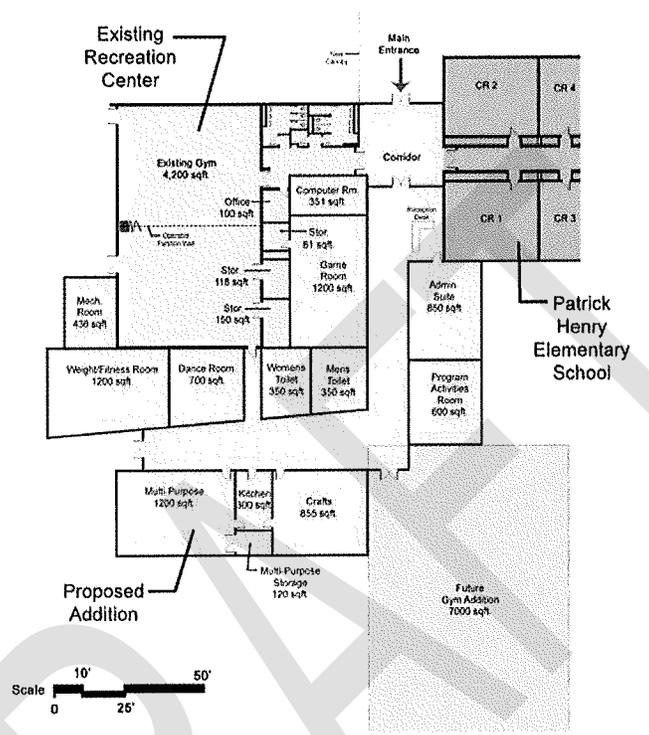
The key design issues that are important to the development of the design are:

- The ability to expand the existing facility while allowing the ring road to continue around the building and minimally affecting the school and its playground. Since there is a hillside close to the gym, the majority of the expansion must be accomplished in the outdoor play area and available open space.
- Finding a location for a future larger gym without overwhelming the scale of the school.

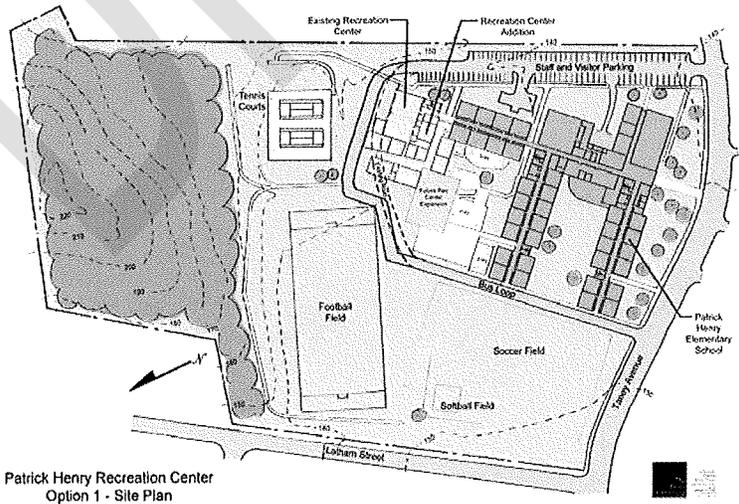
Both options illustrated address these items, but result in different architectural massing and different aesthetic solutions.

IV. RECOMMENDATIONS

A. DESIGN APPROACH



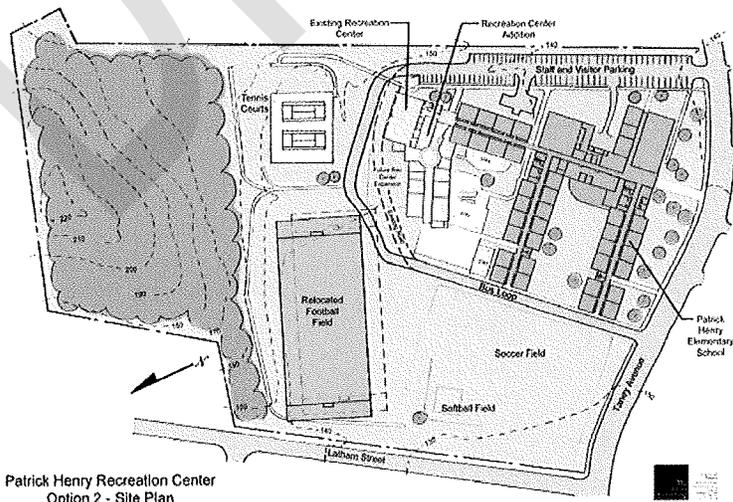
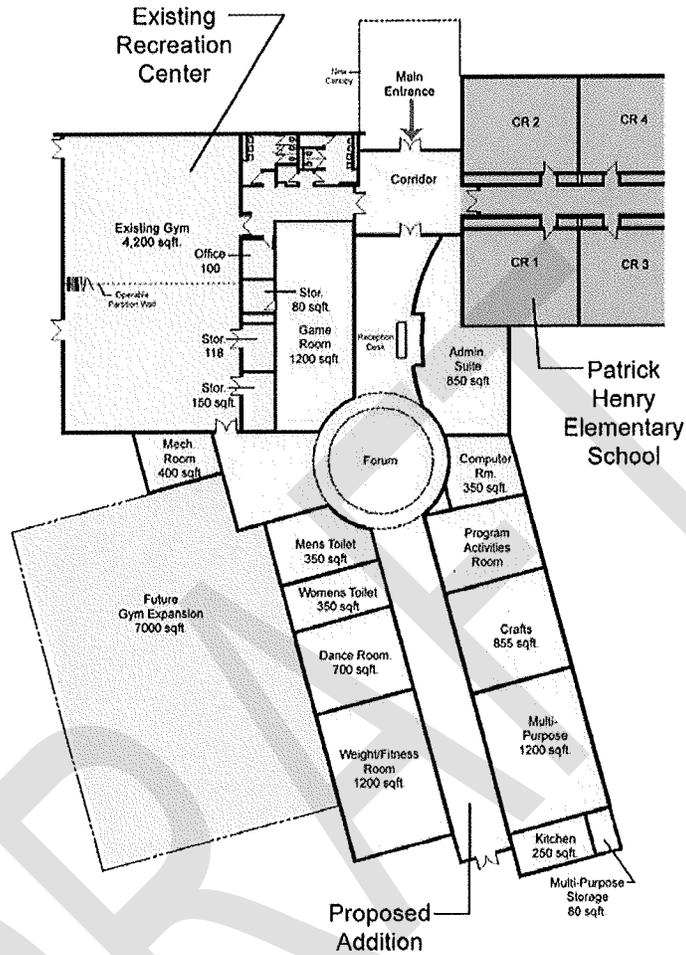
**PATRICK HENRY REC. CENTER
OPTION 1**



**Patrick Henry Recreation Center
Option 1 - Site Plan**

IV. RECOMMENDATIONS

A. DESIGN APPROACH



Patrick Henry Recreation Center
Option 2 - Site Plan

V. PROPOSED SOLUTIONS

A. CIVIL

B. ARCHITECTURE

C. STRUCTURAL

D. MECHANICAL

E. PLUMBING

F. ELECTRICAL

DRAFT

V. PROPOSED SOLUTIONS

G. ISSUES TO BE RESOLVED BY FINAL DESIGNER

Before any decision about the final design, there remain a number of issues that must be resolved by the final designer. Many of them are outlined in the memo from the IDR meeting (See Appendix A)

Issues include:

- Preparation of a parking study and application for a parking reduction
- Preparation of a topographic and utility survey
- Resolution of storm water management design approach
- Resolution of route and surface material of the “ring road”

DRAFT

City of Alexandria

Patrick Henry Facility Survey **Executive Summary Report**

Overview of the Methodology

ETC Institute conducted a City of Alexandria Patrick Henry Facility Survey to help determine potential outdoor and indoor facilities to improve or develop at the Patrick Henry Facility. The survey was designed to obtain statistically valid results from households. The survey was administered by mail, web and phone.

ETC Institute worked extensively with the City of Alexandria officials in the development of the survey questionnaire. This work allowed the survey to be tailored to issues of strategic importance to help plan the future indoor and outdoor facility improvements and developments.

A five-page survey was mailed to a random sample of 3,000 households. Approximately three days after the surveys were mailed each household that received a survey also received an automated voice message encouraging them to complete the survey. In addition, about two weeks after the surveys were mailed ETC Institute began contacting households by phone. Those who had indicated they had not returned the survey were given the option of completing it by phone.

The goal was to obtain a total of at least 400 completed surveys. ETC/Leisure Vision met that goal with a total of 438 surveys completed. The results of the random sample of 438 households have a 95% level of confidence with a precision rate of at least +/-4.6%.

The following pages summarize major survey findings.

Major Findings:

- ❖ **Facilities Currently Available at the Patrick Henry Facility that Respondent Households Have Used Over the Past 2 Years:** Based on the percentage of overall respondents over the past two years, the following facilities have been available and have been used: Patrick Henry playground (15%), Patrick Henry sports fields (14%), Patrick Henry Recreation Center (12%), Patrick Henry tennis court (9%), and Patrick Henry basketball court (9%).
- ❖ **Respondent Household Participation in Programs or Activities Conducted by the City of Alexandria Department of Recreation, Parks and Cultural Activities at the Patrick Henry Facility during the Past 2 Years:** Based on the percentage of respondents, 14% indicated that household respondents *have participated* in a program or activity conducted by the City of Alexandria at the Patrick Henry Facility over the past two years, while 86% of respondents indicated that they have not participated in a program or activity.
- ❖ **Programs or Activities Respondent Households Have Participated in at the Patrick Henry Facility During the past 2 years:** Based on the percentage of respondents who indicated that they had participated in a program or activity over the past two years, 46% participated in *recreation center registered programs* followed by sports fields registered programs (28%) and Patrick Henry playground (28%). The least participation was for tennis court registered programs (7%).
- ❖ **Ways Respondent Households Have Traveled to the Patrick Henry Facility Over the Past 2 Years:** Based on percentage of respondents who used Patrick Henry facilities over the past two years, the modes of transportation used to get to the facilities were: Walking (18%), bike (9%) and public transportation (3%).
- ❖ **OUTDOOR Parks, Trails and Facilities Respondent Households Would Use if Developed at the Patrick Henry Facility:** Sixty-one percent (61%) of respondents indicated that they would most use walking/biking trails if it was developed at the Patrick Henry Facility. It was followed at a somewhat sizeable margin by outdoor running/walking track (42%) and a close third of natural areas and wildlife habitats (36%). The facility respondents would use the least include: Skateboard Park (6%) and outdoor volleyball court (5%).

- ❖ **OUTDOOR Parks, Trails and Facilities Respondent Households Would Use MOST OFTEN if Developed at the Patrick Henry Facility:** Based on the sum of respondents who selected the item as one of their top four choices, 50% indicated that they would most often use walking/biking trails if developed. Thirty-two percent (32%) indicated that they would use outdoor running/walking track followed by natural areas and wildlife habitats (24%).
- ❖ **OUTDOOR Parks, Trails and Facilities Respondent Households Would be Most Willing to Support to Construct with Their Tax Dollars:** Based on percentage of respondents who selected the item as one of their top four choices, 40% of respondents would be most willing to support the construction of walking/biking trails with tax dollars. Twenty-seven percent (27%) would support the construction of outdoor running/walking track with tax dollars, and 21% would support natural areas and wildlife habitats construction. Respondents would least support outdoor rope courts (2%) and outdoor volleyball courts (1%).
- ❖ **How Respondent Household Feel the Cost for Operating the Types of OUTDOOR Parks, Trails and Facilities Most Important to their Household Should be Paid:** Based on the overall percentage of respondents, 44% indicated that property taxes should pay the majority of costs for operating the outdoor facilities and fees from users the remaining costs. Thirty-five percent (35%) indicated fees from users should pay the majority of costs and property taxes the remaining costs, while 21% indicated the entire cost should come through property taxes.
- ❖ **INDOOR Recreation and Cultural Arts Facilities Respondent Households Would Use if Developed at the Patrick Henry Facility:** Forty-two (42%) of respondents indicated that they would most use indoor swimming pools if developed at the Patrick Henry Facility. It was followed by indoor exercise and fitness space (40%), and indoor running/walking track (32%). The respondents would least support indoor futsal space (5%) and bocce courts (3%).
- ❖ **INDOOR Parks, Trails and Facilities Respondent Households Would Use MOST OFTEN if Developed at the Patrick Henry Facility:** Based on percentage of respondents who selected the item as one of their top four choices, 34% indicated that they would use indoor swimming pools most often if it was developed. Twenty-eight percent (28%) indicated that they would use indoor exercise and fitness space second most often, followed by indoor running/walking track (22%).

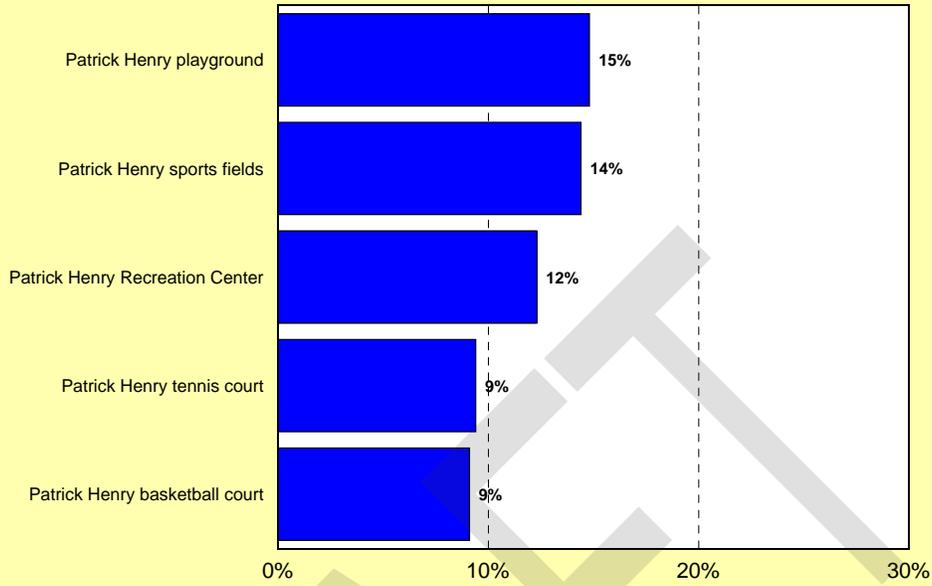
- ❖ **INDOOR Parks, Trails and Facilities Respondent Households Would be Most Willing to Support to Construct with Their Tax Dollars:** Based on percentage of respondents who selected the item as one of their top four choices, 29% of respondents would be most willing to support the construction of indoor swimming pools with their tax dollars. Twenty-seven percent (27%) would support the construction of indoor exercise and fitness space with tax dollars, and 21% would support indoor running/walking track construction. Respondents would least support indoor rope courts (2%) and bocce courts (1%) for construction with their tax dollars.

- ❖ **How Respondent Households Feel the Cost for Operating the Types of INDOOR Parks, Trails and Facilities Most Important to their Household Should be Paid:** Based on percentage of respondents for the construction of *indoor facilities*, 40% indicated that property taxes should pay the majority of costs for operating the indoor facilities and fees from users the remaining costs. Forty-three percent (43%) indicated fees from users should pay the majority of costs and property taxes the remaining costs, while 17% indicated the entire cost should come through property taxes.

- ❖ **How Respondent Households Feel Renovated or New Facilities Should be Developed at the Patrick Henry Facility:** Based on the percentage of overall respondents, 50% of respondents indicated that both outdoor and indoor recreation & cultural arts facilities should be developed. Twenty percent (20%) indicated that only outdoor parks and recreation facilities should be developed, while 17% indicated that only indoor recreation and cultural arts facilities should be developed. Thirteen percent (13%) indicated that no renovated or new facilities should be developed.

Q1. Facilities Currently Available at the Patrick Henry Facility that Respondent Households Have Used Over the Past 2 Years

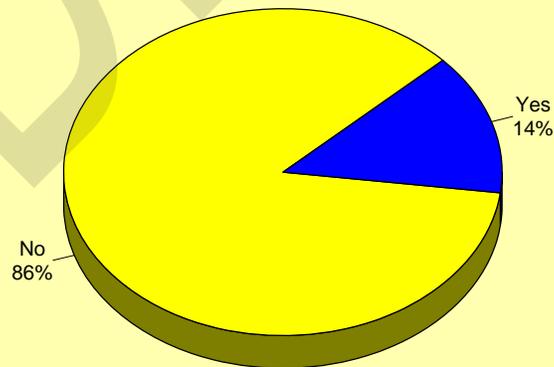
by percentage of respondents (multiple selections possible)



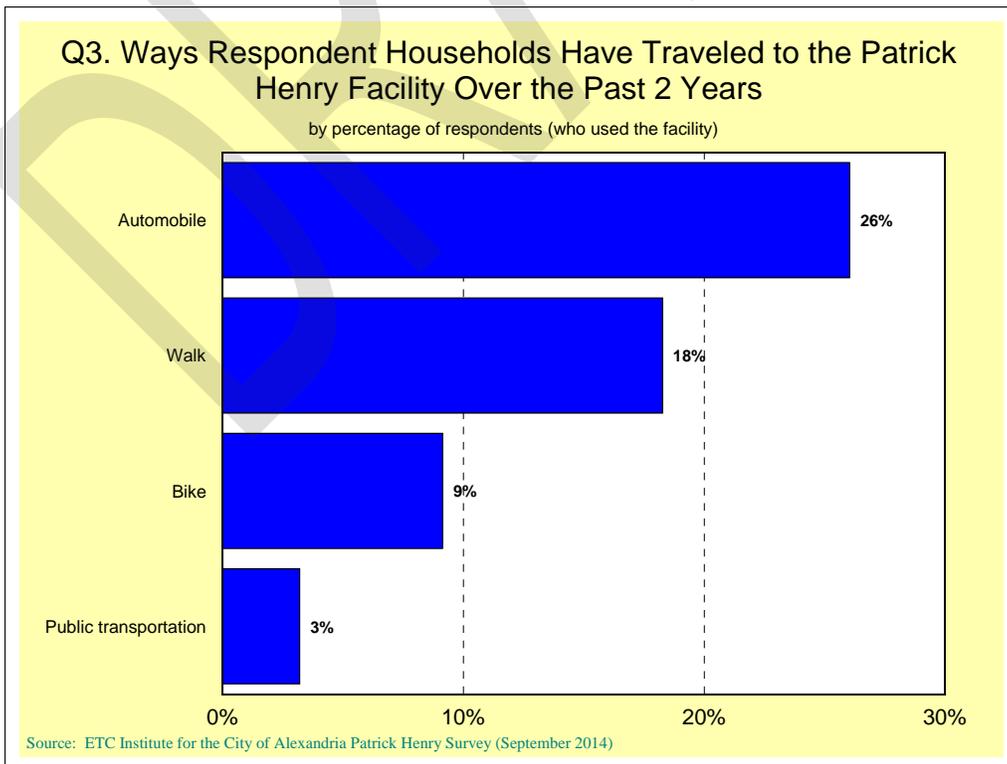
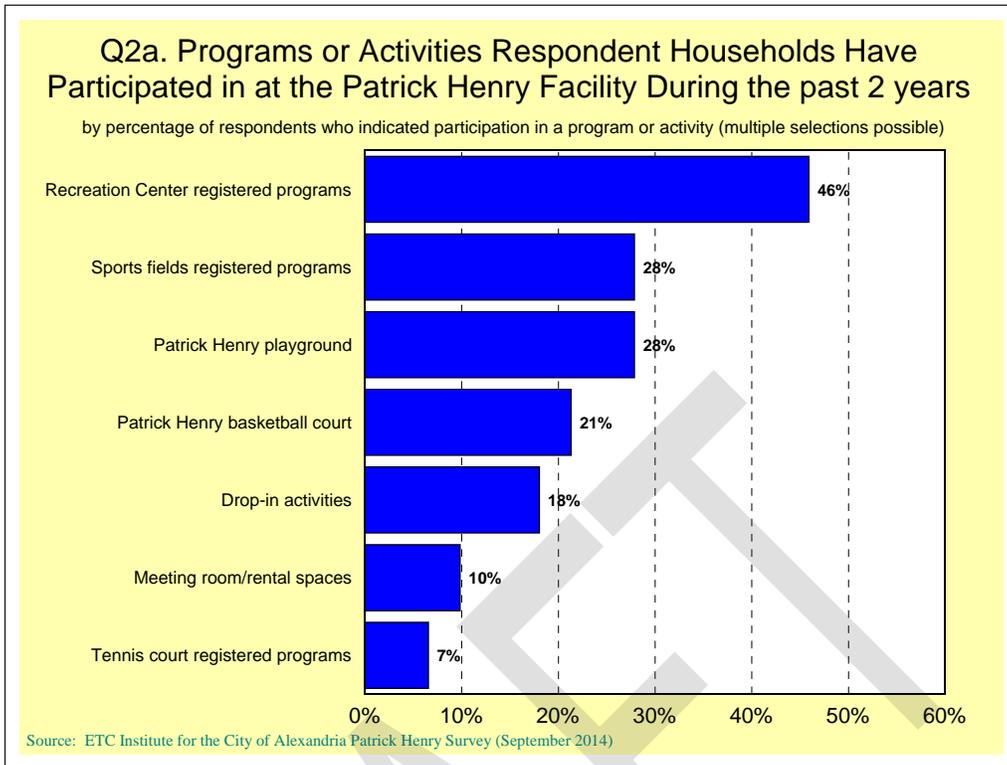
Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q2. Respondent Household Participation in Programs or Activities Conducted by the City of Alexandria Department of Recreation, Parks and Cultural Activities at the Patrick Henry Facility during the Past 2 Years

by percentage of respondents (who indicated they had participated in programs or activities)

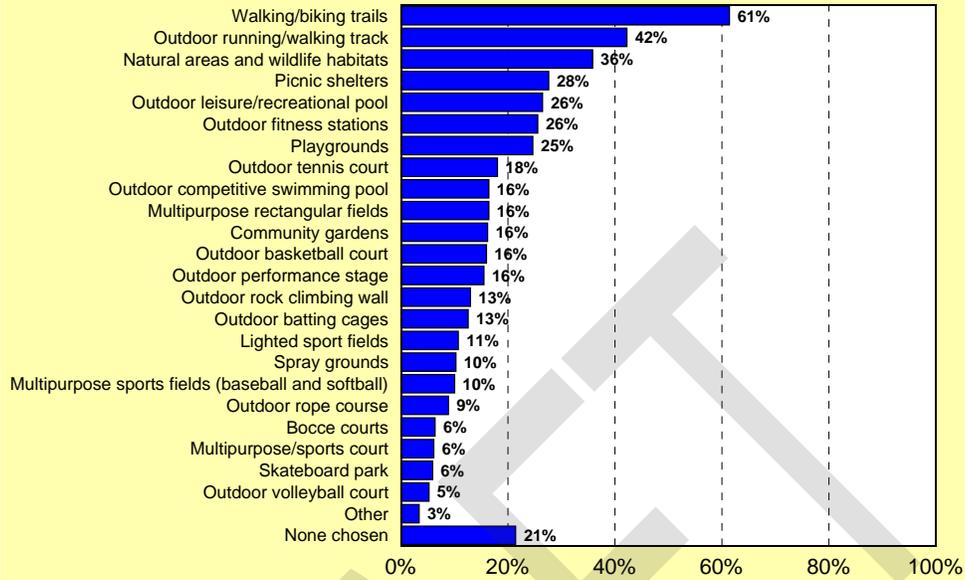


Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)



Q4. OUTDOOR Parks, Trails and Facilities Respondent Households Would Use if it was Developed at the Patrick Henry Facility

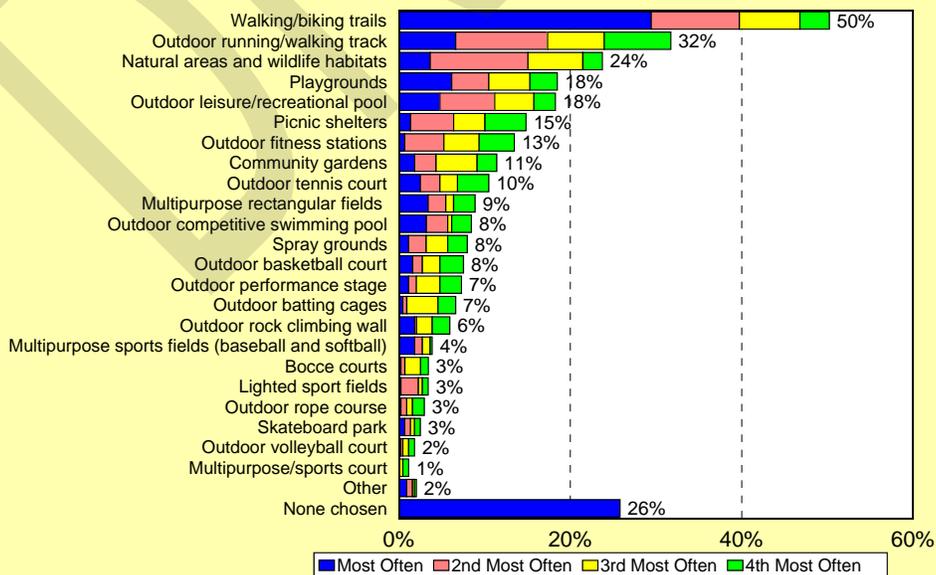
by percentage of respondents



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q5. OUTDOOR Parks, Trails and Facilities Respondent Households Would Use MOST OFTEN if it was Developed at the Patrick Henry Facility

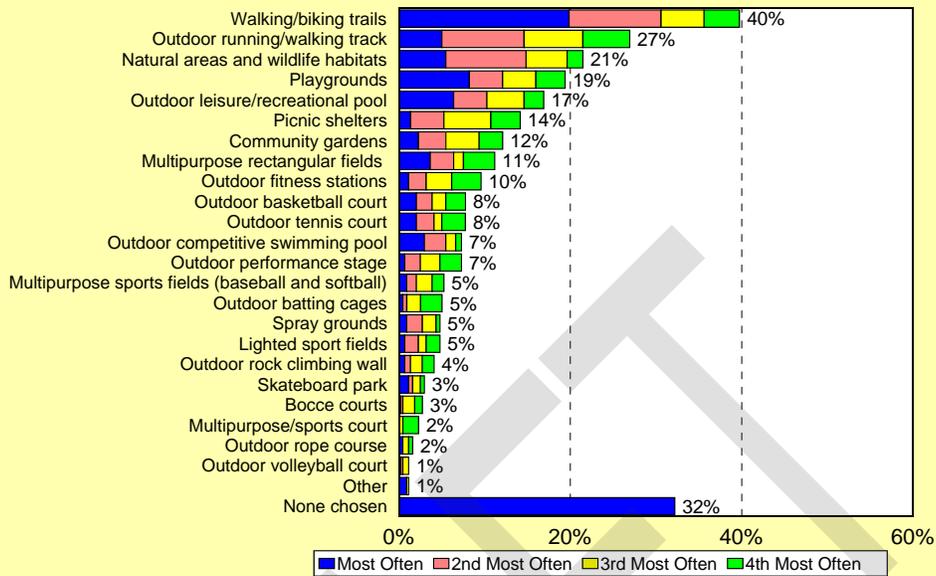
by percentage of respondents who selected the item as one of their top four choices



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q6. OUTDOOR Parks, Trails and Facilities Respondent Households Would be Most Willing to Support to Construct with Their Tax Dollars

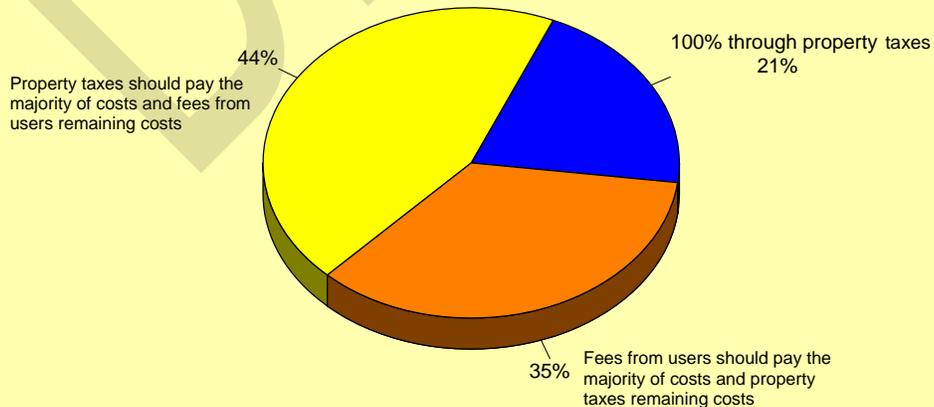
by percentage of respondents who selected the item as one of their top four choices



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q7. How Respondent Household Feel the Cost for Operating the Types of OUTDOOR Parks, Trails and Facilities Most Important to their Household Should be Paid

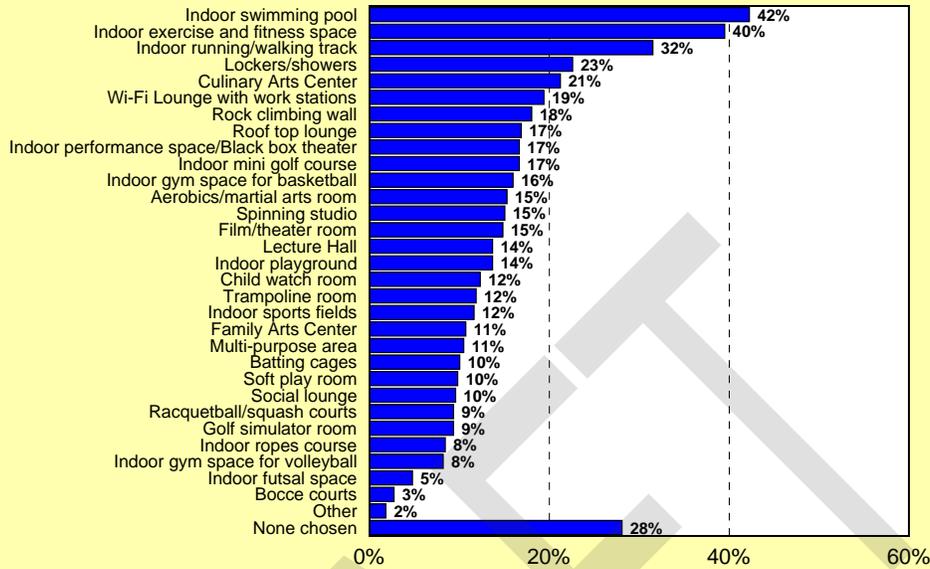
by percentage of respondents



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q8. INDOOR Recreation and Cultural Arts Facilities Respondent Households Would Use if Developed at the Patrick Henry Facility

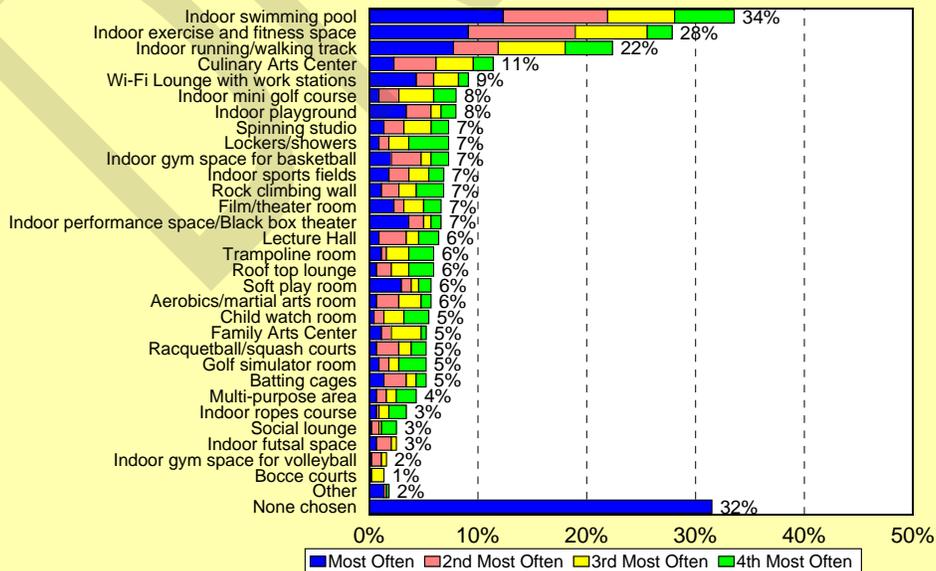
by percentage of respondents (multiple selections possible)



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q9. INDOOR Parks, Trails and Facilities Respondent Households Would Use MOST OFTEN if it Was Developed at the Patrick Henry Facility

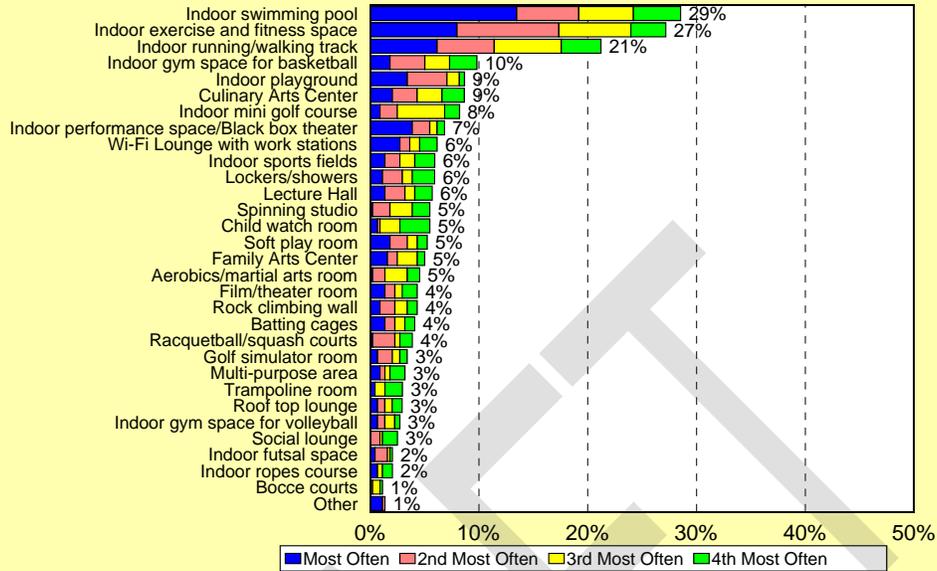
by percentage of respondents who selected the item as one of their top four choices



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q10. INDOOR Parks, Trails and Facilities Respondent Households Would be Most Willing to Support to Construct with Their Tax Dollars

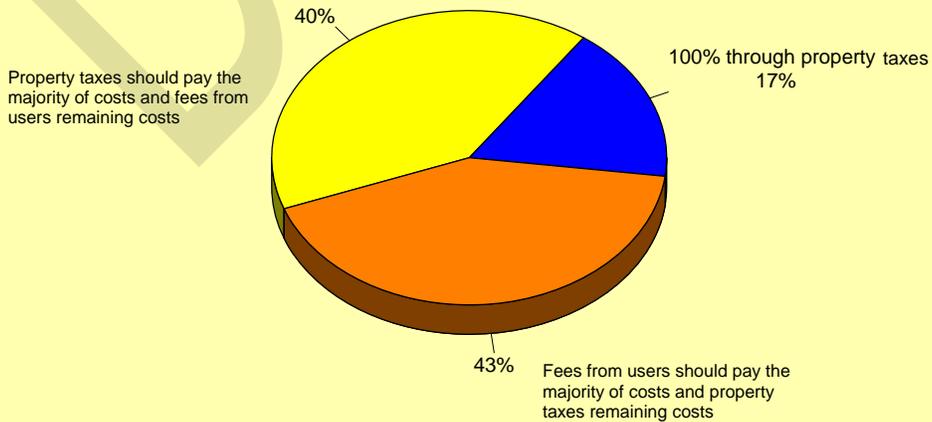
by percentage of respondents who selected the item as one of their top four choices



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q11. How Respondent Households Feel the Cost for Operating the Types of INDOOR Parks, Trails and Facilities Most Important to their Household Should be Paid

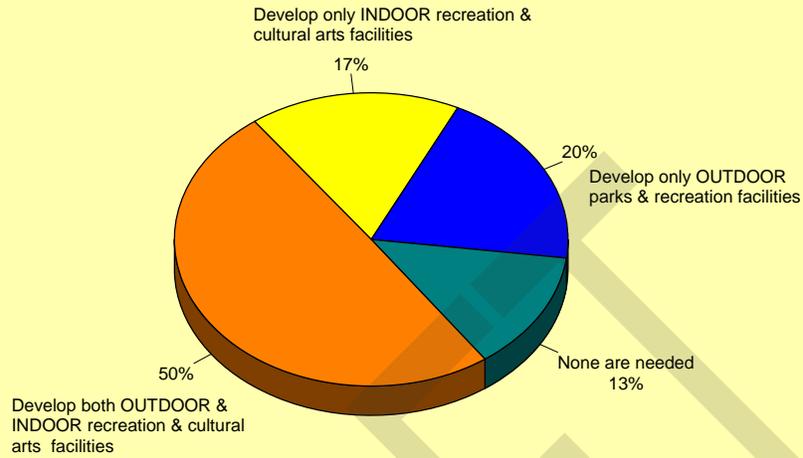
by percentage of respondents



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q12. How Respondent Households Feel Renovated or New Facilities Should be Developed at the Patrick Henry Facility

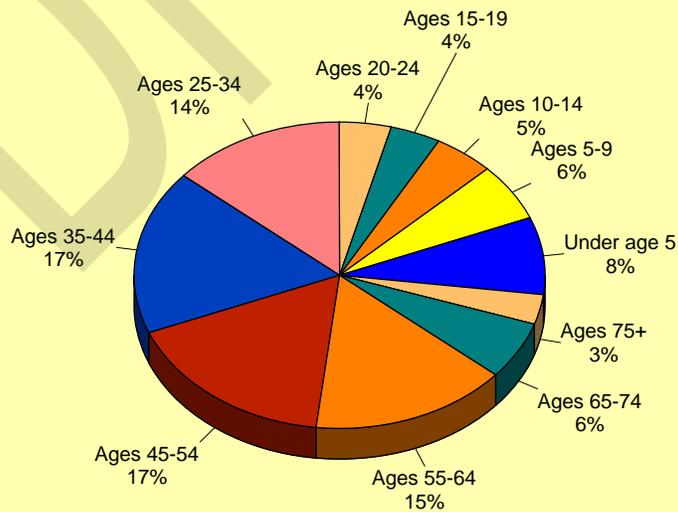
by percentage of respondents



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q13. Demographics: Ages of People in Respondent Households

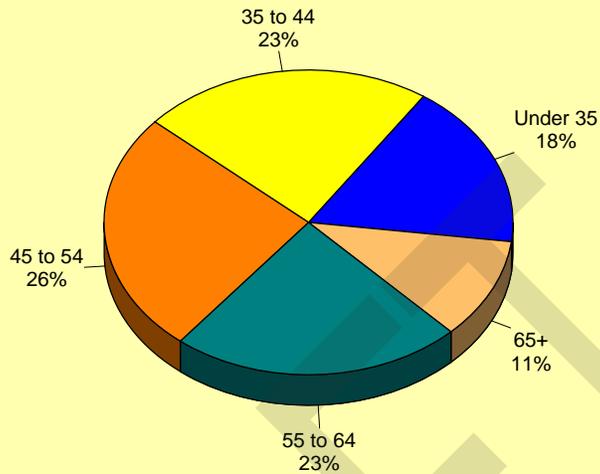
by percentage of respondents



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q14. Demographics: Age of Respondent

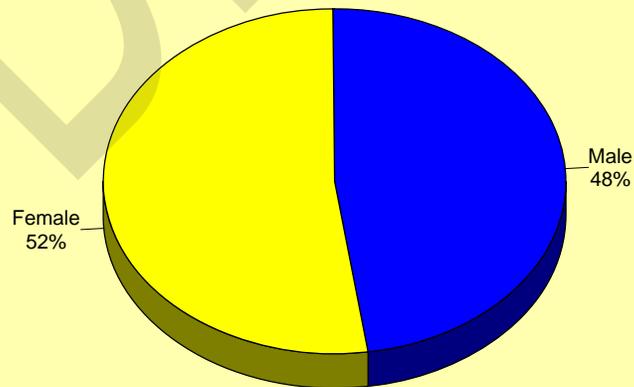
by percentage of respondents



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q15. Demographics: Gender

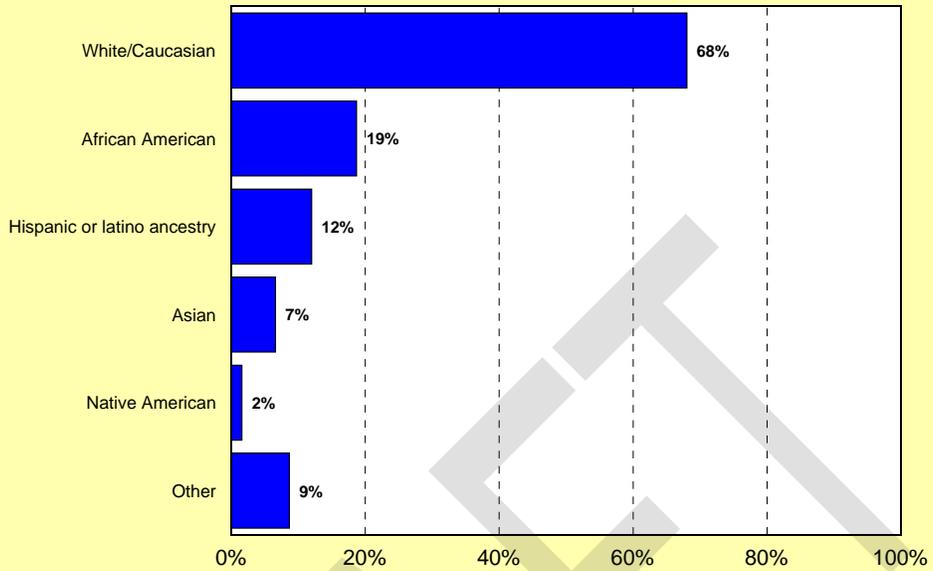
by percentage of respondents



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q16. Demographics: Race and Ethnicity

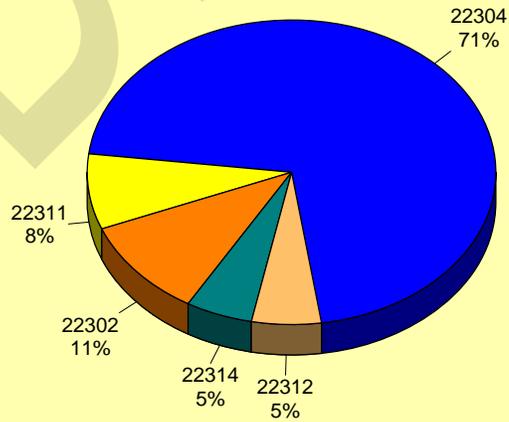
by percentage of respondents (multiple selections possible; without not provided)



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q18. Demographics: Zip Code

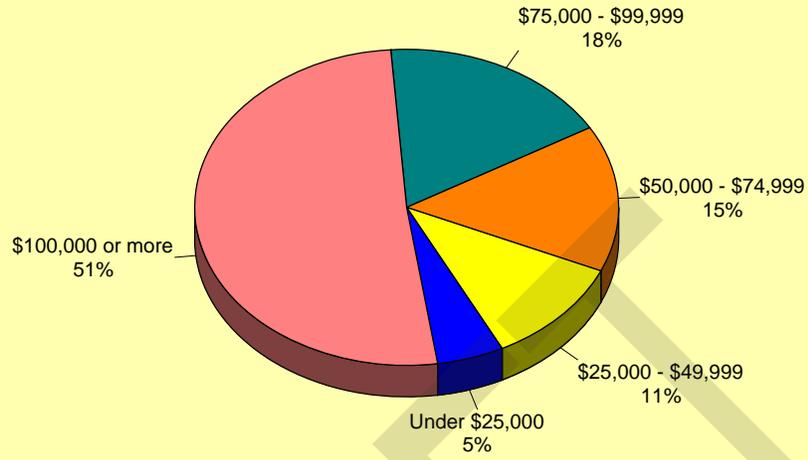
by percentage of respondents



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

Q19. Demographics: Household Income

by percentage of respondents (excluding not provided)



Source: ETC Institute for the City of Alexandria Patrick Henry Survey (September 2014)

DRAFT

PREFACE

In November 2014, Brailsford & Dunlavey, Inc. ("B&D") was retained by the City of Alexandria Recreation Parks & Cultural Activities ("RPCA") to conduct a study evaluating the viability of a field house facility at the location of the current Patrick Henry Recreation Center (the "Study"). B&D's engagement on this assignment was limited to the provision of a preliminary market analysis, an evaluation of comparable facilities located near the proposed site, and the provision of recommendations for additional analysis.

DRAFT

INTRODUCTION

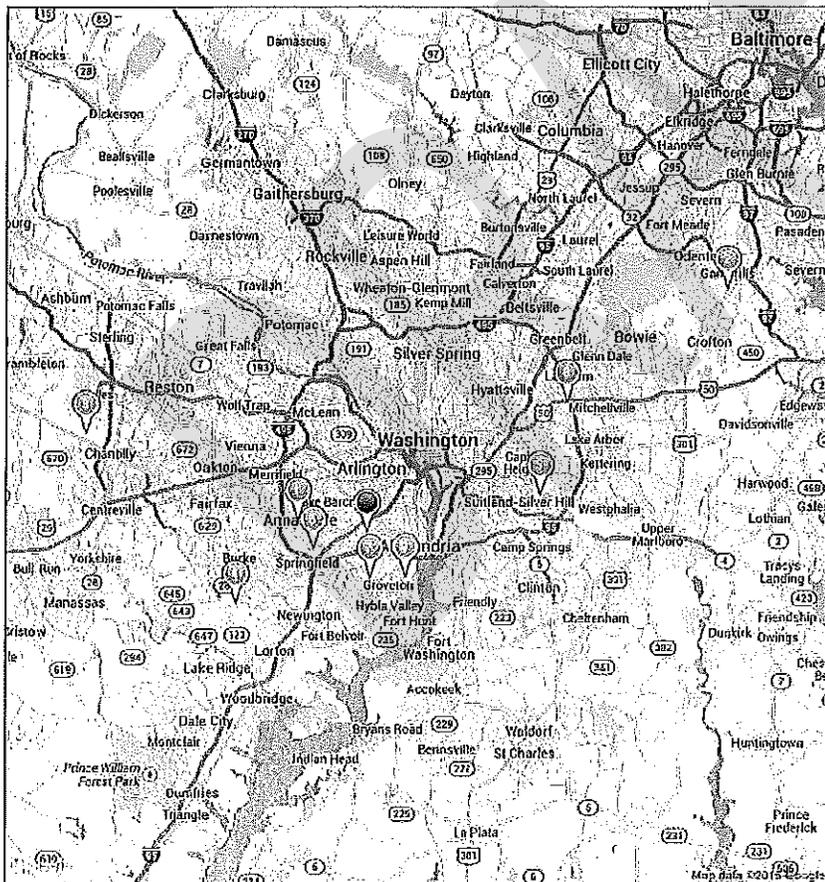
In November 2014, Brailsford & Dunlavy, Inc. ("B&D") was retained by the City of Alexandria Recreation Parks & Cultural Activities ("RPCA") to conduct a study evaluating the viability of a field house facility at the location of the current Patrick Henry Recreation Center (the "Study"). The field house will become part of the overall development of the new Patrick Henry PK-8 School and RPCA was interested in evaluating alternatives to their traditional recreation center concepts to accommodate demand in the city for evening field rental without having to light existing fields, which is often controversial.

BACKGROUND

This Study analyzed unmet market demand, identified programmatic options, and evaluated operating costs and revenues for a proposed field house to be located at the Patrick Henry Elementary School site in Alexandria, Virginia. Detailed market analyses and case studies were conducted to establish preliminary program needs, which were then translated into a recommended facility size.

PRELIMINARY MARKET ANALYSIS

B&D evaluated the supply of similar field-house facilities located near the proposed project site and identified eight facilities which are shown below in Map 1.



1. Fairfax Athletic Club
2. A.P.I
3. NOVA Field House
4. South Run Rec Center
5. Capital SportsPlex
6. Fairfax Sports Complex
7. Lee Center
8. Soccer Dome III
9. Mount Vernon Rec Center

Map 1: Competitive Facilities

B&D reviewed each facility to develop a side-by-side comparison of amenities and programs. Table 1.0 below provides a summary.

On average, the competitive facilities offer a range of mid-size turf fields, flex courts, a meeting/party rental room, and (in two of the facilities), an indoor track. The sports typically offered include soccer, lacrosse, football, volleyball, basketball, and futsal. Most of the facilities host both adult and youth leagues, as well as tournaments and summer camps.

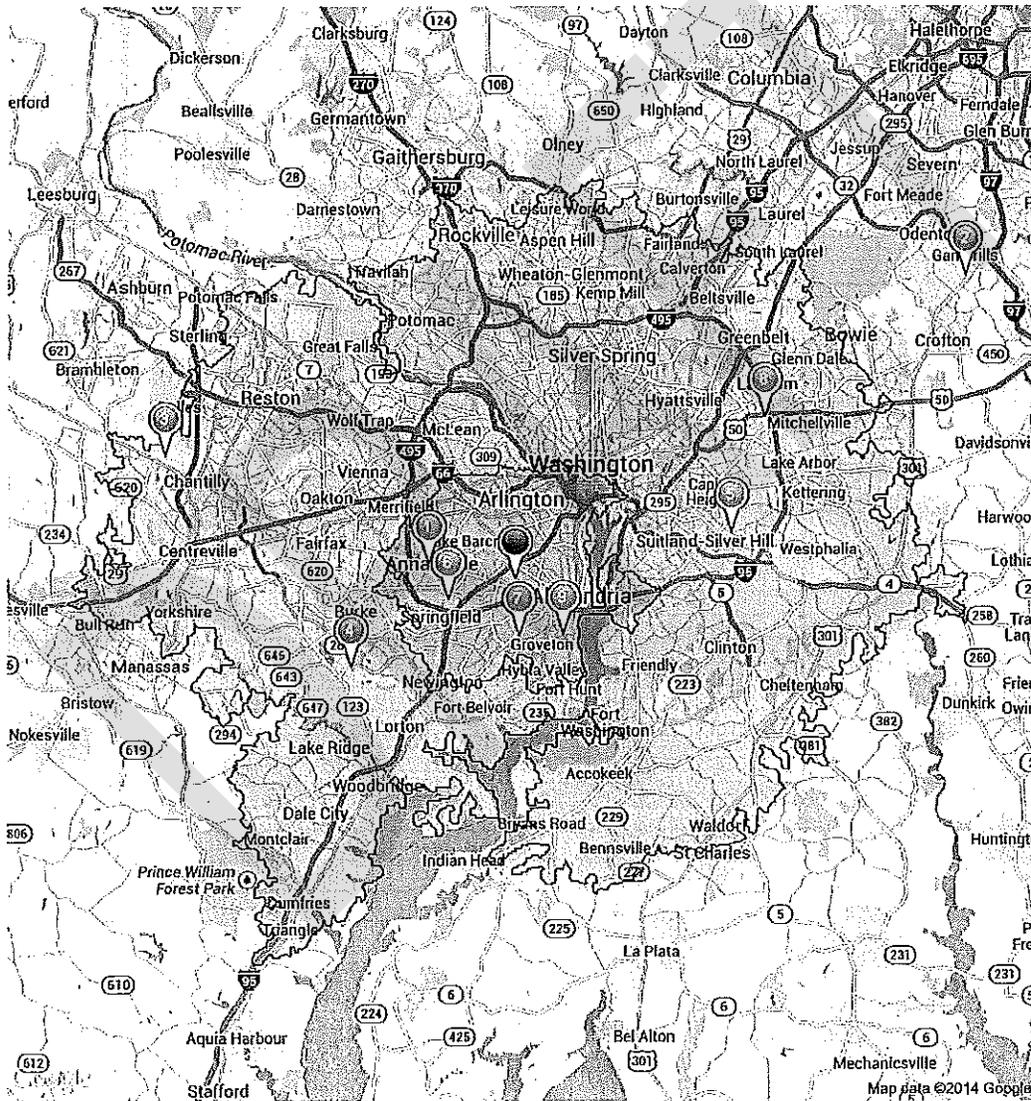
Facility Number	1	2	3	4	5
Facility Name	Fairfax Athletic Center	A.P.I	NOVA Field House	South Run Rec	Capital SportsPlex
Facility Address	4317 Ravensworth Rd Annandale	740 MD Route 3 South, Gambrills, MD 21054	14810 Murdock Street, Chantilly, Virginia	7550 Reservation Drive, Springfield, VA 22153	10011 Good luck Rd, Glenn Dale Maryland 20769
Area	Not Available	55,000	35,530	24,000	45,000
Turf Fields	2	2 (200x100, 160x120)	2 (170x97)	1 (160x100)	2
Flex Courts	2	2 (7,500)	1 (94x50)	2 Outdoor	No Court
Track	No Track	4 lane/50 meter	No Track	No Track	No Track
Meeting/Party Room	Not Available	1000 S.F	1,134 S.F	NO Meeting/Party Room	2 x 1000 S.F
Distance From Site (Miles)	5.4	38.8	27	14	26
Drive Time (Min)	14	50	40	22	30
	Private	Private	Private	Public	Private
Uses					
Soccer	X	X	X	X	X
Futsal	X	NO	X	NO	X
Lacrosse	X	X	X	X	
Football	X	X	X	X	X
Field Hockey	X	X	X	X	X
Volleyball	X	X	X	NO	X
Basketball	X	X	X	NO	X
Ice Hockey	NO	NO	NO	NO	NO
Skating	NO	NO	NO	NO	NO
Gymnastics	NO	NO	X	NO	NO

Facility Number	6	7	8	9
Facility Name	Fairfax Sportsplex	Leo Center	Soccer Domell	Mount Vernon Rec
Facility Address	6800 Commercial Dr, Springfield, VA 22151	6601 Telegraph Road Franconia, Virginia	8400 ARDWICK ARDMORE RD LANDOVER, MD 20785	2017 Belle View Blvd, Alexandria, VA 22307
Area	Not Available	20,400	20,000 +	Not Available
Turf Fields	5 (115x70)	No Turf Fields	3 (60x30) 1 (30x11)	No Turf Fields
Flex Courts	2	1 big divided 2 small	1 (60x27)	No Flex Courts
Track	No Track	3 lane 1/10th mile	No Track	No Track
Meeting/Party Room	No Meeting/Party Room	Not Available	Table Space	Not Available
Distance From Site (Miles)	5.3	5.8	24.5	6
Drive Time (Min)	15	13	32	15
	Public	Public	Private	Public
Uses				
Soccer	X	NO	X	NO
Futsal	NO	NO	NO	NO
Lacrosse	NO	NO	NO	NO
Football	NO	NO	NO	NO
Field Hockey	NO	NO	NO	NO
Volleyball	X	X	NO	NO
Basketball	NO	X	NO	NO
Ice Hockey	NO	NO	NO	X
Skating	NO	NO	NO	X
Gymnastics	NO	X	NO	NO

Table 1.0: Competitive Facilities

MARKET AREA DEMOGRAPHICS

B&D evaluated market area demographic characteristics in order to compare the area surrounding the proposed Patrick Henry site with the locations of the case study facilities discussed below. These demographic characteristics also allow B&D to preliminarily evaluate market demand based upon a comparison with national sports participation data. In order to perform this analysis, B&D used mapping software to identify 20 and 40 minute drive time area surrounding the proposed site. Map 2.0 below provides an overlay of these two drive time areas along with the eight competing facilities identified in Map 1.0. Based upon the locations of competing facilities in the 20-minute drive area, the proposed facility is more likely to draw patrons from within the City of Alexandria or areas directly to the North than it is from areas South or West.



Map 2.0: 20- and 40-minute drive times

B&D compared population, age breakdown, and household income distribution in the 20 and 40 minute drive areas surrounding the proposed facility. As shown in Table 2.0, below, in 2014 the 20-minute drive area had a population of 749,133 residents and thirty-five percent (35%) of the residents in the 20 and 40-minute drive areas are between the ages of 20 and 39, which is well above the DC, regional, and national percentages. These individuals could provide a critical patron base for the proposed facility since they participate regularly in recreational activities.

	10-20 Minute Drive Time	20-40 Minute Drive Time	Washington, DC MSA	United States
Total Population				
2010 Census	697,273	5,303,723	5,636,232	308,745,538
2104 Estimate	749,133	5,602,114	5,952,645	314,419,291
2019 Projection	816,037	6,968,946	6,372,350	323,000,000
Age Distribution				
Age 19 Years or Less	22%	28%	26%	27%
Age 20 to 39 Years	35%	35%	29%	26%
Age 40 to 64 Years	32%	37%	34%	33%
Age 65 Years or Over	11%	13%	12%	13%
Household Income Distribution				
HH Income \$150,000 or More	11%	10%	11%	3%
HH Income \$100,000 to \$149,999	8%	7%	7%	1%
HH Income \$75,000 to \$99,999	6%	5%	5%	4%
HH Income \$50,000 to \$74,999	6%	6%	6%	7%
HH Income \$35,000 to \$49,999	3%	3%	3%	4%
HH Income \$25,000 to \$34,999	2%	2%	2%	4%
HH Income \$15,000 to \$24,999	2%	2%	2%	4%
HH Income \$10,000 to \$14,999	3%	3%	2%	5%

Table 2.0: Market Demographics

Household income within a market area is a key determiner of a recreational facility's success. At \$118,543 and \$121,179 respectively, both the 20 and 40-minute drive areas have high average household incomes, which presents a very favorable indicator of high participation.

The National Sporting Goods Manufacturers Association (NSGA) in its *2013 Sports Participation in the USA* 2013 edition states that the vast majority of recreational users will be between 20 and 39 years of age and earning household incomes of \$100,000 or more annually.

Utilizing data from the NSGA Sports Participation in the USA 2013 edition, Table 3.0, below, indicates the national participation rates for frequent participants (50+ times/year) between the ages of 25-39, then applies those rates against the age distribution for the 20 minute drive area to predict the population of core participants residing within that geography.

Sport	National Participation	10-20 min Participation
Volleyball	2%	11964
Soccer	3%	18867
Football	1%	7921
Lacrosse	1%	3979
Running	6%	40373
Basketball	5%	18867

Table 3.0: Participation Data

The strong market area demographics and the large quantity of core participants predicted is strong indicator of demand for the types of programs found within the typical field house facility. The presence of four competing facilities within the 20 minute drive area is also an indicator of strong market support for the facility type under consideration.

CASE STUDIES

B&D conducted case study analyses of several regional field houses in order to gain an understanding of their programmatic offerings, featured amenities, and major revenue opportunities. The indoor field houses included in this analysis are multi-use facilities that offer athletic and recreational opportunities to members of the community through youth camps, clinics, and youth and adult league play. Flag football, indoor soccer, basketball, and indoor lacrosse were sports commonly available for league play or other activities among the facilities.

B&D targeted the following five facilities to conduct case studies based on the breadth of programs and activities offered, the size and quality of the facility, in addition to geographic and demographic similarities to the Patrick Henry site.

- Fairfax Athletic Center
- A.P.I
- NOVA Field House
- South Run
- Capital SportsPlex

Table 4.0 provides an overview of the programs and facility amenities included within each of the case study sites.

Facility Number	1	2	3	4	5
Facility Name	Fairfax Athletic Center	A.P.I.	NOVA Field House	South Run Rec	Capital SportsPlex
Facility Address	4317 Ravensworth Rd Annandale	740 MD Route 3 South, Gambrills, MD 21054	14810 Murdock Street, Chantilly, Virginia	7550 Reservation Drive, Springfield, VA 22153	10011 Good Luck Rd, Glenn Dale Maryland 20769
Area	Not Available	55,000	35,530	24,000	45,000
Turf Fields	2	2 (200x100, 160x120)	2 (170x97)	1 (160x100)	2
Flex Courts	2	2 (7,500)	1 (94x50)	2 Outdoor	No Court
Track	No Track	4 lane/60 meter	No Track	No Track	No Track
Meeting/Party Room	Not Available	1000 S.F	1,134 S.F	NO Meeting/Party Room	2 x 1000 S.F
Distance From Site (Miles)	5.4	38.8	27	14	26
Drive Time (Min)	14	50	40	22	30
	Private	Private	Private	Public	Private
Uses					
Soccer	X	X	X	X	X
Futsal	X	NO	X	NO	X
Lacrosse	X	X	X	X	
Football	X	X	X	X	X
Field Hockey	X	X	X	X	X
Volleyball	X	X	X	NO	X
Basketball	X	X	X	NO	X
Ice Hockey	NO	NO	NO	NO	NO
Skating	NO	NO	NO	NO	NO
Gymnastics	NO	NO	X	NO	NO
Programs					
Home School	NO	X	NO	NO	NO
Tournaments	X	X	X	X	X
Adult Leagues	X	X	X	X	X
Youth Leagues	X	X	X	X	X
Family Gathering	X	X	X	X	X
Camps	X	X	X	X	X
Concerts	NO	NO	NO	NO	X
Rentals	X	X	X	X	X

Table 4.0: Case Study Facilities

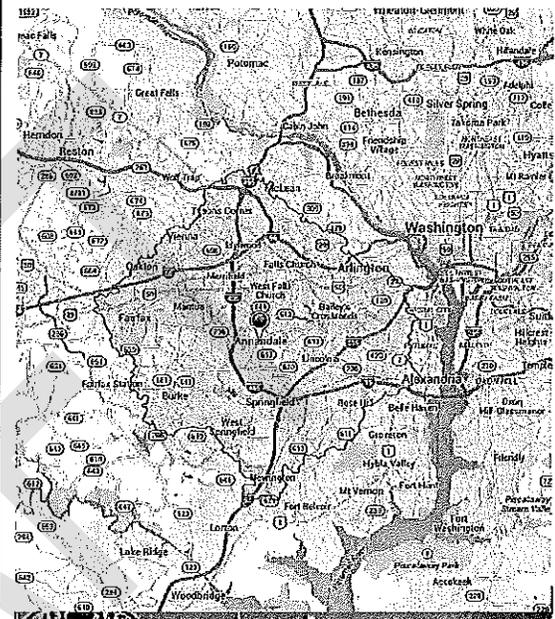
The indoor field houses included in this analysis are multi-use facilities that offer athletic and recreational opportunities to members of the community through youth camps, clinics, and youth and adult league play. Flag football, indoor soccer, basketball, and indoor lacrosse were sports commonly available for league play or other activities among the facilities.

FAIRFAX ATHLETIC CENTER (KIDS CHOICE SPORTS CENTER)

Fairfax Athletic Center is a privately owned and operated sports complex located in Annandale, Virginia roughly 5.5 miles from the proposed Patrick Henry site. The facility includes two turf fields, two flex courts and offers both court and field rentals, and offers both youth and adult soccer leagues.

	Patrick Henry	Fairfax Athletic Club	
Total Population			
2010 Census	697,273	780,193	
2014 Estimate	749,133	840,962	
2019 Projection	816,037	731,847	
Age Distribution			
Age 19 Years or Less	22%	183504	22%
Age 20 to 39 Years	35%	241894	29%
Age 40 to 64 Years	32%	261133	31%
Age 65 Years or Over	11%	93663	11%
Household Income Distribution			
HH Income \$150,000 or More	27%	91639	31%
HH Income \$100,000 to \$149,999	20%	60308	20%
HH Income \$75,000 to \$99,999	14%	38,925	13%
HH Income \$50,000 to \$74,999	16%	43,979	15%
HH Income \$35,000 to \$49,999	8%	23,092	8%
HH Income \$25,000 to \$34,999	5%	13,768	5%
HH Income \$15,000 to \$24,999	5%	12,596	4%
HH Income \$10,000 to \$14,999	6%	12183	4%

Table 5: Fairfax Athletic Club Demographics



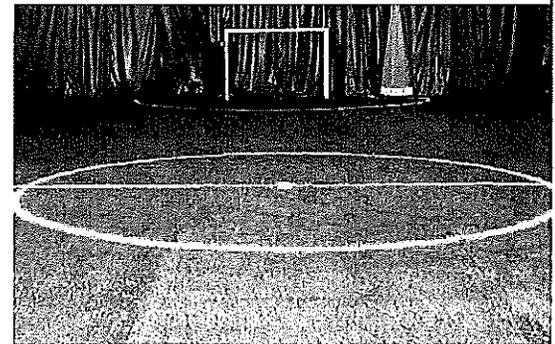
Map 3: 20 minute drive time

Table 5, above shows the demographic characteristics of the 20 minute drive-time radius surrounding Fairfax Athletic Club as shown in the map below.

As expected, the market characteristics are very similar to that of the proposed Patrick Henry site.

Fairfax Athletic Center charges \$100/hour for field rentals on weekdays, \$135 on weekends and \$80/hour for the flex courts.

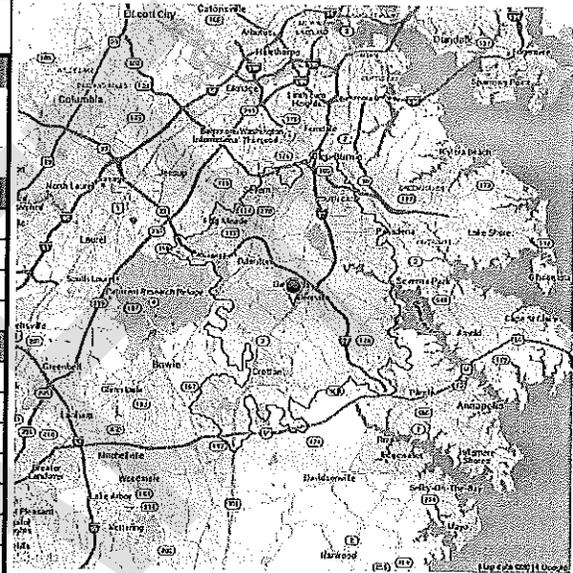
Ownership declined to disclose operating or revenue data for this facility.



A.P.I

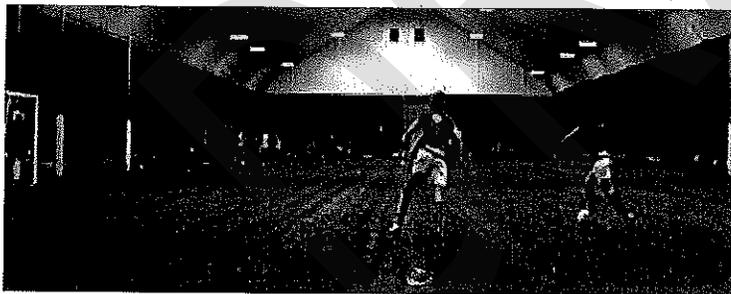
A.P.I is a privately owned and operated sports complex located in Gambrills, Maryland roughly 40 miles from the site. The facility opened in 2011, and is home to over 10,000 athletes and families with over 250,000 visitors a year. The facility has both indoor (55,000 S.F) and outdoor (40,000 S.F) space 40,000 S.F of which are turf fields, and plays host to camps, clinics and lessons, leagues and tournaments, rentals, fields trips, and birthday parties. Table 6 shows the demographic characteristics of the 20 minute drive-time area surrounding A.P.I, as shown in the map below.

	Patrick Henry	API	
Total Population			
2010 Census	697,273	176,708	
2014 Estimate	749,133	190,143	
2019 Projection	816,037	168,774	
Age Distribution			
Age 19 Years or Less	22%	47218	25%
Age 20 to 39 Years	35%	47851	25%
Age 40 to 64 Years	32%	60411	32%
Age 65 Years or Over	11%	21230	11%
Household Income Distribution			
HH Income \$150,000 or More	27%	16860	26%
HH Income \$100,000 to \$149,999	20%	14196	22%
HH Income \$75,000 to \$99,999	14%	9,552	15%
HH Income \$50,000 to \$74,999	16%	10,414	16%
HH Income \$35,000 to \$49,999	8%	5,814	9%
HH Income \$25,000 to \$34,999	5%	3,570	5%
HH Income \$15,000 to \$24,999	5%	2,919	4%
HH Income \$10,000 to \$14,999	6%	2381	4%



Map 4: 20 minute drive time

Table 6: API Demographics

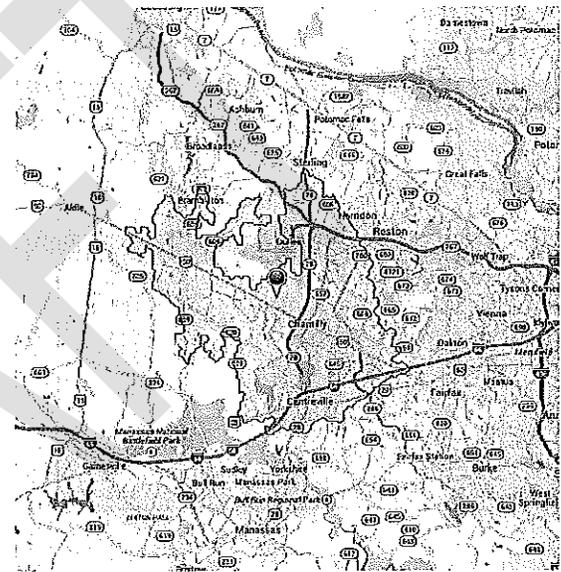


NOVA FIELD HOUSE

The NOVA Field House is a privately owned and operated sports complex located in Chantilly, Virginia roughly 27 miles and about 40 minutes from the site. The facility is privately run and utilizes advertising and marketing partnerships as one of its revenue generators. The facility features two of Northern Virginia’s largest indoor turf fields, as well as an NBA regulation sized basketball court

Table 7 below shows the demographic break down of the 20 minute drive time area surrounding NOVA, as shown in the map below. The market demographics surrounding this facility are similar to those surrounding the Patrick Henry site, with the exception of a notably higher percentage of persons aged 19 or younger.

	Patrick Henry	NOVA	
Total Population			
2010 Census	697,273	247,572	
2014 Estimate	749,133	267,307	
2019 Projection	816,037	232,521	
Age Distribution			
Age 19 Years or Less	22%	71,785	27%
Age 20 to 39 Years	35%	76,368	29%
Age 40 to 64 Years	32%	82,898	31%
Age 65 Years or Over	11%	16,520	6%
Household Income Distribution			
HH Income \$150,000 or More	27%	32,183	39%
HH Income \$100,000 to \$149,999	20%	18,895	23%
HH Income \$75,000 to \$99,999	14%	10,168	12%
HH Income \$50,000 to \$74,999	16%	11,011	13%
HH Income \$35,000 to \$49,999	8%	4,982	6%
HH Income \$25,000 to \$34,999	5%	2,349	3%
HH Income \$15,000 to \$24,999	5%	1,995	2%
HH Income \$10,000 to \$14,999	6%	1,176	1%



Map 5: 20 minute drive time

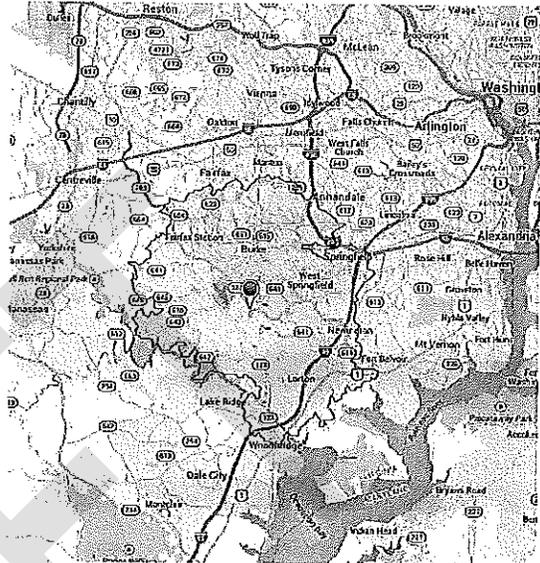
Table 7: API Demographics

NOVA field house charges \$140/hour for the field rental in the summer, \$195/hour during the winter and \$90/hour for the flex courts. B&D was not able to obtain operating or revenue data for this facility.



SOUTH RUN REC CENTER

The South Run Rec Center is owned and operated by Fairfax County and is located roughly 20 minutes southeast of the proposed site. The building was initially constructed as an equestrian center in 1988, but was repurposed as an indoor field house facility in 2005. Table 8 below shows the demographic break down of the 20 mile drive time area surrounding South Run RecCenter as shown in the map below. The distribution of incomes and ages is very similar to that of the area surrounding the proposed Patrick Henry site except for the notably higher percentage of persons aged 19 or younger.



Map 5: 20 minute drive time

	Patrick Henry	South Run	
Total Population			
2010 Census	697,273	265,263	
2014 Estimate	749,133	280,307	
2019 Projection	816,037	252,215	
Age Distribution			
Age 19 Years or Less	22%	72,045	26%
Age 20 to 39 Years	35%	61,260	22%
Age 40 to 64 Years	32%	98,065	35%
Age 65 Years or Over	11%	33,892	12%
Household Income Distribution			
HH Income \$150,000 or More	27%	33,402	38%
HH Income \$100,000 to \$149,999	20%	18,894	22%
HH Income \$75,000 to \$99,999	14%	10,720	12%
HH Income \$50,000 to \$74,999	16%	10,677	12%
HH Income \$35,000 to \$49,999	8%	5,668	7%
HH Income \$25,000 to \$34,999	5%	2,775	3%
HH Income \$15,000 to \$24,999	5%	2,498	3%
HH Income \$10,000 to \$14,999	6%	2,213	3%

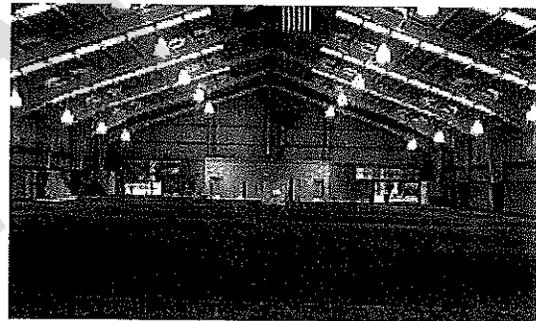


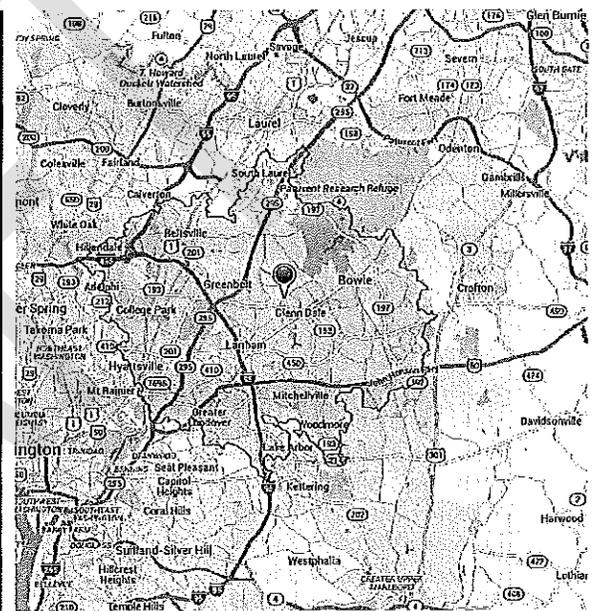
Table 8: South Run Demographics

B&D was informed that Fairfax park service charges rental rates of \$130/hour during the prime season and \$100/hour during the rest of the year (Memorial Day-Labor Day). Rental revenue has remained consistent the past few years at about \$160,000/year, and is usually rented by soccer and lacrosse leagues. Fairfax County Park Authority operates the field house as part of the larger facility, so although staffing costs were not available, average yearly maintenance costs were reported to be approximately \$40-\$60K annually.

CAPITAL SPORTSPLEX

Capital Sportsplex is a privately owned and operated indoor facility located in Glen Dale, Maryland approximately 45 minutes to the north east of the proposed Patrick Henry site. The demographic characteristics indicated for the market area in the table below are significantly different from that of the Patrick Henry site. There is a significantly higher percentage of the population under 20 years of age and the income is more evenly distributed with a much larger percentage of the population earning less than \$75,000 annually. This likely means that this facility is more reliant on youth sports and its convenient access to major transportation routes to draw more regional participation. The facility offers a variety of youth and adult soccer and futsal leagues in addition to tournaments, classes, personal training, and facility rentals.

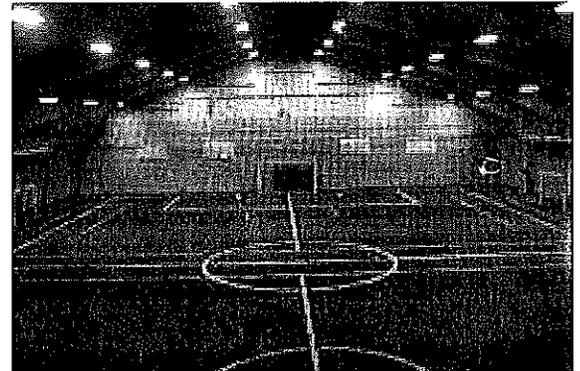
	Patrick Henry	Capital SportsPlex	
Total Population			
2010 Census	697,273	314,591	
2014 Estimate	749,133	322,707	
2019 Projection	816,037	329,533	
Age Distribution			
Age 19 Years or Less	22%	88976	28%
Age 20 to 39 Years	35%	99727	31%
Age 40 to 64 Years	32%	100604	31%
Age 65 Years or Over	11%	33398	10%
Household Income Distribution			
HH Income \$150,000 or More	27%	19293	18%
HH Income \$100,000 to \$149,999	20%	20056	19%
HH Income \$75,000 to \$99,999	14%	15810	15%
HH Income \$50,000 to \$74,999	16%	20652	19%
HH Income \$35,000 to \$49,999	8%	12361	12%
HH Income \$25,000 to \$34,999	5%	6437	6%
HH Income \$15,000 to \$24,999	5%	5372	5%
HH Income \$10,000 to \$14,999	6%	6747	6%



Map 6: 20 minute drive time

Table 9: South Run Demographics

Capital Sportsplex charges \$90/hour for full field rentals on weekdays before 4pm, \$115/hour on weekdays after 4pm and \$115/hour on weekends. Half field rates are \$65/hour when available. B&D was not able to obtain operating or revenue data for this facility.



PRELIMINARY MARKET ANALYSIS FINDINGS

- The 20-minute drive area surrounding the proposed Patrick Henry site has the appropriate characteristics to support the proposed facility type based upon national participation data, as well as the market characteristics of similar facilities evaluated in this study.
- The existence of two competitive facilities within the 20 minute drive area (the Fairfax Athletic Center and the Fairfax Sports Complex) will have an impact on the ability of the proposed site to draw participation from a significant portion of this geography.
- The average size of the case study sites was 46,000 GSF and all sites featured turf fields and basketball courts.
- Field houses are generally managed and/or owned by private organizations which could be indicative of market strength and/or the complexity of operating and programming this type of facility

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PRELIMINARY PROGRAM RECOMMENDATIONS

B&D developed an outline program that reflects RPCA's vision for the facility along with the competitive context and market analysis findings. It is important to note that this preliminary recommendation is only based upon offerings of comparable facilities and has not been adjusted to reflect a detailed analysis of demand for programs in the market area. Table 10 below shows the list of all revenue-generating elements that are recommended for the field house, including recreation spaces, specialized activity spaces, and community spaces.

Program/Area (Sq.Ft.)	
<u>Turf Fields</u>	
Number of fields	2
Field Area (100 x 50)	5000
Total Field Area	10000
Stands	1000
Total	11000
<u>Flex Courts</u>	
Number Of Courts	1
Field Area (84 x 50)	4200
Total Field Area	4200
Stands	1000
Total	5200
<u>Running Track</u>	
Number of Lanes	4
Length of Lanes	400
Width of Lane	3.6
Total	5760
<u>Other</u>	
Office	500
Fitness Room	1000
Meeting/Party Room	500
Locker Rooms	2000
Total	4000
Total Program Area	25960
Circulation Etc.	20%
	5192
Total Field House Area	31,152

PARKING

RPCA requested that B&D evaluate parking requirements for facilities of this type. Based upon a review of zoning requirements in Alexandria City, the closest use type is either the Commercial Amusement Enterprise for indoor spaces or Institutional Community Buildings. Both of these use types have a requirement of one space per 200 SF. Based upon the initial program recommendation of 31,000 GSF, application of this parking standard will result in the need for approximately 155 spaces. Standard parking site analysis usually dictates allocation of approximately 150 square feet per parking space to account for drive aisles, lighting, and ingress/egress pathways. This preliminary analysis results in a parking requirement of approximately 23,000 square feet of space on site.

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PROJECT ECONOMICS

Table 11 below indicates the key revenue drivers by sport and further more by playing surface.

Sports/Revenue Source	
<u>Turf</u>	<u>Court</u>
Soccer Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events	Basketball Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events
Lacrosse Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events	Volleyball Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events
Football Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events	Futsal/Indoor Soccer Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events
Other Sports and recreational Activities Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events	Other Sports and recreational Activities Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events
Running Track Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events	

Table 11: Revenue Sources

Sports/Revenue Source	
<u>Turf</u>	<u>Court</u>
Soccer Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events	Basketball Rentals/Leagues Instructional Clinics & Camps Drop-in Play Events

Lacrosse

Rentals/Leagues
 Instructional Clinics & Camps
 Drop-in Play
 Events

Football

Rentals/Leagues
 Instructional Clinics & Camps
 Drop-in Play
 Events

Other Sports and recreational Activities

Rentals/Leagues
 Instructional Clinics & Camps
 Drop-in Play
 Events

Running Track

Rentals/Leagues
 Instructional Clinics & Camps
 Drop-in Play
 Events

Volleyball

Rentals/Leagues
 Instructional Clinics & Camps
 Drop-in Play
 Events

Futsal/Indoor Soccer

Rentals/Leagues
 Instructional Clinics & Camps
 Drop-in Play
 Events

Other Sports and recreational Activities

Rentals/Leagues
 Instructional Clinics & Camps
 Drop-in Play
 Events

Because the majority of case study facilities evaluated are privately operated, operating revenue was not made available to our team. We suggest as a next step expanding our search regionally in order to identify a similar, publicly-operated facility from another market in order to find this information and to build a detailed revenue model and operating proforma to evaluate cost recovery characteristics for this type of facility.

Next Steps:

- It is recommended that additional conversations with planning and zoning officials be undertaken to account for the fact that this building will be co-located with school and that shared parking will likely result in a reduced need for dedicated parking associated with this facility.
- The market analysis findings reveal strong local demographic characteristics that are similar to those of other comparable facilities that are currently operating successfully. It is unknown at this time the extent of the impact on local demand that will result from the existence of two competing facilities in the 20-minute drive area. B&D recommends that interviews and focus group interviews be conducted with organizations and individuals who are most likely to be users of the facility in order to gain a better understanding of local market dynamics and demand for the preliminary program offerings listed herein.
- The distribution of programs and allocation of spaces should be driven by detailed market demand results and an evaluation of operating revenue potential. As mentioned above, we suggest as a next step expanding our search regionally in order to identify a similar, publicly operated facility from another market in order to find this information and to build a detailed revenue model and operating proforma to evaluate cost recovery characteristics for this type of facility. If we are unable to identify an acceptable facility, B&D recommends developing a revenue model and operating proforma based upon historical data our firm has compiled and in conjunction with B&D's Centers team (<http://www.centersusa.com/>) which operates recreation facilities nationwide.

APPENDIX

Total	Age:									
	7-11	12-17	18-24	25-34	35-44	45-54	55-64	65-74	75+	
Base: Total U.S. (Age 7+)	285533	20426	25045	31318	42249	40483	44239	38569	23981	19221
	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0
	100.0	7.2	8.8	11.0	14.8	14.2	15.5	13.5	8.4	6.7
Basketball	25579	4037	5436	4635	4656	3615	2212	748	229	10
	9.0	19.8	21.7	14.8	11.0	8.9	5.0	1.9	1.0	0.1
	100.0	15.8	21.3	18.1	18.2	14.1	8.6	2.9	0.9	0.0
Football (Tackle)	7889	1500	3139	1428	879	539	195	208	--	--
	2.8	7.3	12.5	4.6	2.1	1.3	0.4	0.5	--	--
	100.0	19.0	39.6	18.1	11.1	6.8	2.5	2.6	--	--
Hockey (Ice)	2900	372	397	418	738	503	408	53	13	--
	1.0	1.8	1.6	1.3	1.7	1.2	0.9	0.1	0.1	--
	100.0	12.8	13.7	14.4	25.4	17.3	14.1	1.8	0.4	--
Lacrosse	2749	324	1179	409	528	235	42	21	10	--
	1.0	1.6	4.7	1.3	1.2	0.6	0.1	0.1	0.0	--
	100.0	11.8	42.9	14.9	19.2	8.6	1.5	0.8	0.4	--
Running/Jogging	40031	2364	5613	7923	8907	7318	4593	2006	788	418
	14.0	11.6	22.4	25.3	21.1	18.1	10.6	5.2	3.3	2.2
	100.0	5.9	14.0	19.8	22.3	18.3	11.7	5.0	2.0	1.0
Soccer	13690	4041	2078	1972	2210	1703	613	205	50	19
	4.8	19.8	11.5	6.3	5.2	4.2	1.4	0.5	0.2	0.1
	100.0	28.5	18.0	14.4	18.1	12.4	4.5	1.5	0.4	0.1
Volleyball	10250	1180	2584	1857	1958	1004	1044	521	81	50
	3.6	5.8	10.2	5.9	4.6	2.5	2.4	1.4	0.3	0.3
	100.0	11.5	24.9	18.1	19.1	9.8	10.2	5.1	0.8	0.5
Weight Lifting	31083	158	2580	5648	6030	5725	4809	3413	1743	878
	10.9	0.8	10.3	18.0	14.3	14.1	11.1	8.8	7.3	4.6
	100.0	0.5	8.3	18.2	19.4	18.4	15.8	11.0	5.6	2.8
Work Out At Club	35241	196	1628	5258	5802	5855	6125	4255	3750	2371
	12.3	1.0	6.5	16.8	13.7	14.5	13.8	11.0	15.6	12.3
	100.0	0.6	4.6	14.9	16.5	16.6	17.4	12.1	10.6	6.7

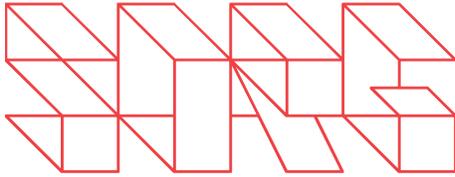
Table 2: NSGA national sports participation by age group

Total	Household Income								
	Under \$15,000	\$15,000-\$24,999	\$25,000-\$34,999	\$35,000-\$49,999	\$50,000-\$74,999	\$75,000-\$99,999	\$100,000-\$149,999	\$150,000+	
Base: Total U.S. (Age 7+)	285533	38098	32173	32165	40026	49902	32754	33831	26584
	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0
	100.0	13.3	11.3	11.3	14.0	17.5	11.5	11.8	9.3
Basketball	25579	3952	2069	2218	3265	4316	3305	3564	3191
	9.0	9.6	6.4	6.9	8.2	8.6	10.1	10.5	12.0
	100.0	14.3	8.1	8.7	12.8	16.9	12.9	13.9	12.5
Football (Tackle)	7889	1019	706	1111	1545	1078	1131	887	411
	2.8	2.7	2.2	3.6	3.9	2.2	3.5	2.6	1.5
	100.0	12.9	8.9	14.1	19.6	13.7	14.3	11.2	5.2
Hockey (Ice)	2900	419	124	129	302	398	339	673	517
	1.0	1.1	0.4	0.4	0.8	0.8	1.0	2.0	1.9
	100.0	14.5	4.3	4.4	10.4	13.7	11.7	23.2	17.8
Lacrosse	2749	276	149	120	73	234	277	520	1101
	1.0	0.7	0.5	0.4	0.2	0.5	0.8	1.5	4.1
	100.0	10.1	5.4	4.4	2.6	8.5	10.1	18.9	40.0
Running/Jogging	40031	4136	2695	2824	4925	6604	6079	6665	7101
	14.0	10.9	8.4	8.8	12.3	13.2	18.6	19.7	26.7
	100.0	10.3	6.7	7.1	12.3	16.5	12.7	16.7	17.7
Soccer	13690	1791	647	1029	1669	1994	1441	2463	2655
	4.8	4.7	2.0	3.2	4.2	4.0	4.4	7.3	10.0
	100.0	13.1	4.7	7.5	12.2	14.8	10.5	18.0	19.4
Volleyball	10250	1515	974	896	1195	1696	1018	1624	1534
	3.6	4.0	3.0	2.8	3.0	3.2	3.1	4.5	5.8
	100.0	14.8	9.5	8.7	11.7	15.8	9.9	14.9	15.0
Weight Lifting	31093	2789	2283	2163	3748	5607	4165	5347	4982
	10.9	7.3	7.1	8.7	9.4	11.2	12.7	15.8	18.7
	100.0	9.0	7.3	7.0	12.1	18.0	13.4	17.2	18.0
Work Out At Club	35241	2419	2387	2528	4089	6408	5257	6340	5805
	12.3	8.3	7.4	7.9	10.2	12.9	16.0	19.7	21.8
	100.0	6.9	6.8	7.2	11.6	18.2	14.9	18.0	18.5

Table 3: NSGA national sports participation by income

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8.4 Meeting Minutes



Location: Cafeteria
Meeting Date: 17 December 2014
Meeting Time: 6:00 pm
Subject: Community Meeting
Project: Patrick Henry Elementary School & Recreation Center
Project #: 1417

Attending:

Mark Eisenhour (ME)	ACPS	meisenho@acps.k12.va.us	703 772 1072
Bill Conkley (BC)	Sorg	Billc@sorgarchitects.com	202 393 6445
Clair Wholean (CW)	Sorg	Clairw@sorgarchitects.com	202 393 6445

Meeting Minutes

1. Introduction

- 1.1. ME introduced the project and Sorg Architects to the group. The project is in the early stages of a feasibility study to determine the needs of the school and recreation center. An outreach process is ongoing with the school staff, administration and the community to obtain feedback. Two meetings with teachers have been held and this is the first community meeting.

2. Project Goal

- 2.1. ME explained the project goal is to house a K-8 school and a recreation center in either a renovation of the existing building, demolition of existing building and construction of a new building, or some combination of renovation and addition. The future school is estimated to house 800 students and the recreation center is for use by the broader community. The feasibility study will evaluate the options along with associated costs and impact to the operation of the school. The study will be completed by Sorg architects in May 2015. ACPS will then review the study and make a decision on which option to move forward with. Design will take about a year, followed by construction anticipated to be a year and a half.
- 2.2. An attendee inquired if students would stay in the existing school or be relocated during construction. ME explained that this would depend on which option is pursued, and impact to the students will be carefully considered. A phasing strategy will be developed as part of the study.

- 2.3. An attendee inquired if the school and recreation center would be one building or two, and if they would be built simultaneously. ME stated that the goal is for them to be one project constructed at the same time.
- 2.4. ME explained that the Recreation Center will be funded separately, through RPCA. ACPS and RPCA are coordinating to determine the budget.

3. Design Team Update

- 3.1. BC gave an overview of the observations of the existing building architecture, structure, and MEP systems.
- 3.2. BC described two teacher meetings held recently and feedback generated from those meetings about the program for the school. An architectural program describes what functional spaces are included in a building, and things like which rooms are near each other, and how many people use each room. Teacher feedback as well as community feedback will be incorporated into the program.

4. Community Visioning Activity: Meaning and Friendship

- 4.1. CW introduced an activity where participants fill out a handout with questions about what is meaningful in their community and where they meet friends. Participants then discussed the topic and shared their ideas with the large group.
- 4.2. There was a great variety of responses, but some comments were unanimous. The following ideas were largely agreed on by the group.
 - Identify 3 things about your community that are meaningful to you.
 - Diversity
 - Neighbors/family to family relationships/tight-knit community
 - Library
 - Parks
 - Identify 3 things about your community that you'd like to change.
 - Traffic
 - Safe Playground for all ages
 - Improve Rec Facilities/Add locations/extend hours
 - Walkability/pedestrian friendly/ Sidewalks/trails
 - Community Meeting Place
 - Name a place that you met a friend, or your child met a friend, in your community.
 - Library
 - School
 - Playground
 - Park

5. Community Visioning Activity: Local Inspiration

- 5.1. Following the same process, participants filled out a handout with questions about what is inspirational in their community and where they meet friends. Participants then discussed the topic and shared their ideas with the large group.

5.2. Attendees were inspired by a wide variety of places and events in their community. Here are a few answers that capture the essence of the conversation:

- Name a place in your community that inspires you.
 - Families playing together
 - When the children are happy
 - Being Healthy
 - Preschool
 - Harbor/Old Town
 - Bike Trails/Nature/Trees
 - Brennan Pond Turtle Park/Cameron Park
 - Cultural: Masonic Temple/Alexandria Birthday/Lyceum/Torpedo Factory

- What quality or qualities makes Alexandria special?
 - Diversity
 - Location
 - Own identity
 - History

- Name a quality of another place you have lived or visited that you would enjoy seeing in Alexandria.
 - Traffic calming
 - Walkability
 - Community Building Activities

- What qualities would an ideal school have?
 - Modern Buildings with technology
 - Community Activities/Involvement

6. Community Visioning Activity: Public Places in and Around the School

6.1. CW and BC asked participants to share what they envision for public spaces within and around the school, such as the Lobby, Main Office, Playground, Outdoor Spaces, and Parent Pick-Up area. It was recorded on the white board and discussed. A list of the ideas generated follows below. Sorg architects will consider this information to inform the design.

7. Wrap-Up

7.1. ME invited participants to continue in this process. There will be about one meeting per month for the duration of the study. ME also invited the community to attend a tour of the Southern Region Technology and Recreation Complex, a project recently completed by Sorg Architects, on January 14th at 6pm. A group will meet at 5pm at the Patrick Henry Cafeteria and travel together to the building.

Qualities of Public Spaces in and Around the School

Lobby/ Main Office

- Ample size
- Welcoming
- Inviting
- Open
- Bright
- Security
- Good PA system
- Colorful
- Kiosk/information desk
- Displays
- Efficient
- Visibility/transparency

Playground

- All-purpose playing fields
- Picnic area
- Shared amenities
- Separate play areas by age groups
- Age-appropriate play areas
- Secure
- Outdoor restroom
- Lights
- Trees/green space
- Eco-friendly materials
- Trails

Outdoor Spaces

- Increase bikability
- Increase walkability
- Parking area
- Separate entrance for Pre-K
- Turnarounds
- Clear, organized traffic patterns
- Separate Bus & Car areas

Parent Pick-Up

- Safe
- Lighting
- Logical traffic flow
- Turnaround

Recreation Areas

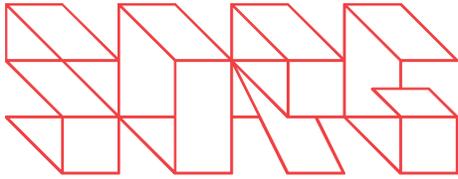
- Adequate size
- Age-appropriate
- Pool
- Gathering space/amphitheater
- Turf

- END OF MEETING -

Errors and omissions should be brought to our attention within ten business days so as to be made a part of this record.

Recorded by: Clair Wholean

Copies to: Attendees



Location: RPCA Lee Center - White Board Room
Meeting Date: 03 February 2015
Meeting Time: 4:00 pm
Subject: Recreation Program Meeting
Project: Patrick Henry Elementary School & Recreation Center
Project #: 1417

Attending:

Mark Eisenhour (ME)	ACPS	meisenho@acps.k12.va.us	703 772 1072
Beth Znidersic (BE)	RPCA	Bethany.Znidersic@alexandriava.gov	703 746 5492
Ron Kagawa (RK)	RPCA	ron.kagawa@alexandriava.gov	703 746 5489
Adrian King (AK)	Alexandria DGS	Adrian.King@alexandriava.gov	703 746 4770
Dana Wedeles (DW)	RPCA	dana.wedeles@alexandriava.gov	703 746 5491
William Chesley (WC)	RPCA	william.chesley@alexandriava.gov	703 746 4343
Bill Conkley (BC)	Sorg	Billc@sorgarchitects.com	202 393 6445
Clair Wholean (CW)	Sorg	Clairw@sorgarchitects.com	202 393 6445

Meeting Minutes

1. Needs Assessment

- 1.1. DW introduced the findings from a needs assessment survey done for residents immediately surrounding Patrick Henry. 86% of respondents do not use the current facility, so there is clearly need for something new. There is stronger need for individual uses rather than group uses, especially space for running, walking, and passive outdoor recreation. In order of preference, the top desired outdoor amenities are trails, a track, and natural open space. The most wanted indoor facilities are a pool, exercise space, and a track. A pool is not in the budget for this project, but the other amenities can definitely be included in the project.
- 1.2. Operationally, the goal is for Patrick Henry to recover 80% of its cost. A blend of amenities that are revenue generating and non-revenue generating will accomplish this. RPCA is in the process of doing a study with Brailsford & Dunlavey to determine what this could be. RK stated that a copy of the study, which includes a list of programmatic functions, will be shared with Sorg for their use in developing an architectural program.
- 1.3. Beyond the top 3 amenities, the needs assessment survey identified many other possible activities, such as a culinary arts center, indoor performance space, spinning studio, martial arts, etc. An indoor multipurpose space that could accommodate many of these possibilities would be ideal.

- 1.4. The needs assessment survey identified a need for indoor and outdoor tracks or trails. The outdoor track need not be a traditional 1/4 mile running track, it could be any type of looped path that has a measurement and signs posted for the distance, i.e. 10 loops = 1 mile, similar to the track in Sorg's Tech Rec project around the gym. Dana noted that there is a naturalist that works with DGS that Sorg can speak to about the outdoor trails.

2. Recreation Program

- 2.1. Fields were discussed. Currently there are not enough fields, but by 2020 the city will meet its needs for turf fields. For this project, RPCA would like an indoor field with a track around it, as well as an outdoor field. Both fields would be 100' x 120', and would be split in half so that so two games could be played simultaneously. The indoor field could have a lightweight structure such as steel joists. A rigid structure is preferred to a bubble structure.
- 2.2. The indoor track could be elevated or on ground level, depending if an elevator is feasible.
- 2.3. In summary, a list of indoor spaces is as follows:
- Indoor soccer field 100' x 120'
 - Indoor track around soccer field with measured distance
 - fitness area
 - lockers (possibly shared with school)
 - gymnasium (shared with school)
 - office
 - toilets
 - storage
 - multipurpose indoor hard-surface court 1/4 size of soccer field
 - indoor rental space for parties/events
 - indoor multipurpose activity space
- Outdoor spaces include:
- turf court 1/4 size of soccer field
 - paved court 1/4 size of soccer field
 - soccer field, grass, 210'x 360'
 - looped track or trail with measured distance
- 2.4. BC inquired about construction sequencing. RPCA would like the existing Rec center to stay operational during construction. Ideally, the new Rec center would be built at the same time as the school and open at the same time as the school.
- 2.5. RPCA would like the existing tennis courts to be demolished.
- 2.6. RK explained that ideally, the Rec center and the school would be separate buildings. Sorg described the cost and space savings that would result from shared space between the two facilities. A common atrium or connecting space can be provided, so that there is access between the school and Rec center, but also a separate, secure entrance for each. RK noted

that operation and maintenance for the Rec center needs to be independent from the school, so building systems should be separate.

- 2.7. Visual surveillance of the Rec center is a priority - the building should be arranged so that there is a single point of entrance that has visual connection to all other spaces.
- 2.8. RK inquired about the comments to the architectural program for the school that were given to Sorg. BC is coordinating with ME and will respond by 2/11.

3. Next Steps

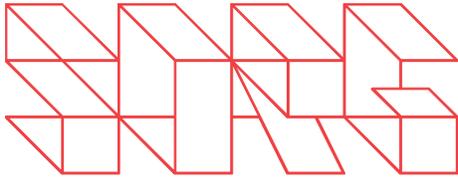
- 3.1. At the next core group meeting, there will be a schedule and program update, and the preliminary Rec center program will be shared with the core group.
- 3.2. The group may go on some field trips of indoor fields to look at precedent projects.
- 3.3. Sorg will provide an updated schedule for the feasibility study.
- 3.4. Sorg will respond to comments to the school program.
- 3.5. The recreation center program is in progress by Sorg, pending further information- RPCA to provide a copy of the study by Brailsford & Dunlavey.

- END OF MEETING -

Errors and omissions should be brought to our attention within ten business days so as to be made a part of this record.

Recorded by: Clair Wholean

Copies to: Mark Eisenhour, Ron Kagawa, Bill Conkey



Location: RPCA Lee Center - White Board Room
Meeting Date: 03 February 2015
Meeting Time: 4:00 pm
Subject: Recreation Program Meeting
Project: Patrick Henry Elementary School & Recreation Center
Project #: 1417

Attending:

Mark Eisenhour (ME)	ACPS	meisenho@acps.k12.va.us	703 772 1072
Beth Znidersic (BE)	RPCA	Bethany.Znidersic@alexandriava.gov	703 746 5492
Ron Kagawa (RK)	RPCA	ron.kagawa@alexandriava.gov	703 746 5489
Adrian King (AK)	Alexandria DGS	Adrian.King@alexandriava.gov	703 746 4770
Dana Wedeles (DW)	RPCA	dana.wedeles@alexandriava.gov	703 746 5491
William Chesley (WC)	RPCA	william.chesley@alexandriava.gov	703 746 4343
Bill Conkley (BC)	Sorg	Billc@sorgarchitects.com	202 393 6445
Clair Wholean (CW)	Sorg	Clairw@sorgarchitects.com	202 393 6445

Meeting Minutes

1. Needs Assessment

- 1.1. DW introduced the findings from a needs assessment survey done for residents immediately surrounding Patrick Henry. 86% of respondents do not use the current facility, so there is clearly need for something new. There is stronger need for individual uses rather than group uses, especially space for running, walking, and passive outdoor recreation. In order of preference, the top desired outdoor amenities are trails, a track, and natural open space. The most wanted indoor facilities are a pool, exercise space, and a track. A pool is not in the budget for this project, but the other amenities can definitely be included in the project.
- 1.2. Operationally, the goal is for Patrick Henry to recover 80% of its cost. A blend of amenities that are revenue generating and non-revenue generating will accomplish this. RPCA is in the process of doing a study with Brailsford & Dunlavey to determine what this could be. RK stated that a copy of the study, which includes a list of programmatic functions, will be shared with Sorg for their use in developing an architectural program.
- 1.3. Beyond the top 3 amenities, the needs assessment survey identified many other possible activities, such as a culinary arts center, indoor performance space, spinning studio, martial arts, etc. An indoor multipurpose space that could accommodate many of these possibilities would be ideal.

- 1.4. The needs assessment survey identified a need for indoor and outdoor tracks or trails. The outdoor track need not be a traditional 1/4 mile running track, it could be any type of looped path that has a measurement and signs posted for the distance, i.e. 10 loops = 1 mile, similar to the track in Sorg's Tech Rec project around the gym. Dana noted that there is a naturalist that works with DGS that Sorg can speak to about the outdoor trails.

2. Recreation Program

- 2.1. Fields were discussed. Currently there are not enough fields, but by 2020 the city will meet its needs for turf fields. For this project, RPCA would like an indoor field with a track around it, as well as an outdoor field. Both fields would be 100' x 120', and would be split in half so that so two games could be played simultaneously. The indoor field could have a lightweight structure such as steel joists. A rigid structure is preferred to a bubble structure.
- 2.2. The indoor track could be elevated or on ground level, depending if an elevator is feasible.
- 2.3. In summary, a list of indoor spaces is as follows:
- Indoor soccer field 100' x 120'
 - Indoor track around soccer field with measured distance
 - fitness area
 - lockers (possibly shared with school)
 - gymnasium (shared with school)
 - office
 - toilets
 - storage
 - multipurpose indoor hard-surface court 1/4 size of soccer field
 - indoor rental space for parties/events
 - indoor multipurpose activity space
- Outdoor spaces include:
- turf court 1/4 size of soccer field
 - paved court 1/4 size of soccer field
 - soccer field, grass, 210'x 360'
 - looped track or trail with measured distance
- 2.4. BC inquired about construction sequencing. RPCA would like the existing Rec center to stay operational during construction. Ideally, the new Rec center would be built at the same time as the school and open at the same time as the school.
- 2.5. RPCA would like the existing tennis courts to be demolished.
- 2.6. RK explained that ideally, the Rec center and the school would be separate buildings. Sorg described the cost and space savings that would result from shared space between the two facilities. A common atrium or connecting space can be provided, so that there is access between the school and Rec center, but also a separate, secure entrance for each. RK noted

that operation and maintenance for the Rec center needs to be independent from the school, so building systems should be separate.

- 2.7. Visual surveillance of the Rec center is a priority - the building should be arranged so that there is a single point of entrance that has visual connection to all other spaces.
- 2.8. RK inquired about the comments to the architectural program for the school that were given to Sorg. BC is coordinating with ME and will respond by 2/11.

3. Next Steps

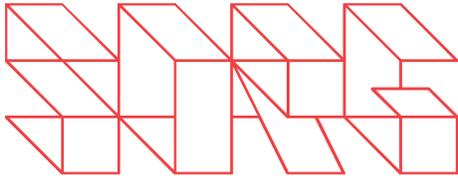
- 3.1. At the next core group meeting, there will be a schedule and program update, and the preliminary Rec center program will be shared with the core group.
- 3.2. The group may go on some field trips of indoor fields to look at precedent projects.
- 3.3. Sorg will provide an updated schedule for the feasibility study.
- 3.4. Sorg will respond to comments to the school program.
- 3.5. The recreation center program is in progress by Sorg, pending further information- RPCA to provide a copy of the study by Brailsford & Dunlavey.

- END OF MEETING -

Errors and omissions should be brought to our attention within ten business days so as to be made a part of this record.

Recorded by: Clair Wholean

Copies to: Mark Eisenhour, Ron Kagawa, Bill Conkey



Location: Sorg Architects
Meeting Date: February 18, 2015
Meeting Time: 9:00 am
Subject: Patrick Henry Design Charrette
Project: Patrick Henry School and Recreation Center
Project #: 1417

Attending:

Mark Eisenhour (ME)	ACPS	Meisenho@acps.k12.va.us	703 965 3418
Laurel Hamming (LH)	ACPS	Laurel.hammig@acps.k12.va.us	703 619 8298
Ingrid Bynum (IB)	Alexandria PHES	Ingrid.bynum@acps.k12.va.us	240 354 4859
Ron Kagawa (RK)	Alexandria RPCA	Ron.kagawa@alexandriava.gov	703 746 5489
Beth Znidersic (BZ)	Alexandria RPCA	Bethany.znidersic@alexandriava.gov	703 746 5492
Tan Cross (TC)	Alexandria GIS	Titania.cross@alexandriava.gov	703 746 3203
Adrian King (AK)	Alexandria GSA	Adrian.king@alexandriava.gov	571 215 5345
Suman Sorg (SS)	Sorg	SumanS@sorgarchitects.com	202 393 6445
Bob Widger (BW)	Sorg	BobW@sorgarchitects.com	202 393 6445
Bill Conkey (BC)	Sorg	BillC@sorgarchitects.com	202 393 6445
Rachel Chung (RC)	Sorg	RachelC@sorgarchitects.com	202 393 6445
Sehee Kim (SK)	Sorg	SeheeK@sorgarchitects.com	202 393 6445
Clair Wholean (CW)	Sorg	ClairW@sorgarchitects.com	202 393 6445

Meeting held to review the preliminary design options for the Patrick Henry School and Recreation Center.

Meeting Minutes

1. Introductions

- 1.1. BC introduced all attendees.
- 1.2. The project is currently in feasibility stage. Sorg has evaluated existing conditions and developed the program for the school and recreation center with input from the client. The programming process is nearly complete, and master planning is in progress. Sorg has developed four concepts, two renovations and two new buildings. The purpose of the charrette is to review these design options and select 3 to develop further, consistent with the scope of work for the feasibility study.

2. Existing Conditions

2.1. CW presented diagrams of existing conditions on the site, microclimate, vegetation, topography, site access, adjacent land use and zoning. The following is a summary of site opportunities and constraints:

Site Opportunities

- Wooded open space to north of site provides views, natural recreation
- South facing warm slope on north end of site
- Street frontage along Taney
- Connection to greenspace through adjacent bike paths
- Local enrollment encourages walking to school
- Shared program with Rec makes site a focal point in community

Site Constraints

- Traffic flow
- Access to rear of site
- Zoning of POS
- Location and size of existing building
- Small scale of adjacent single family homes

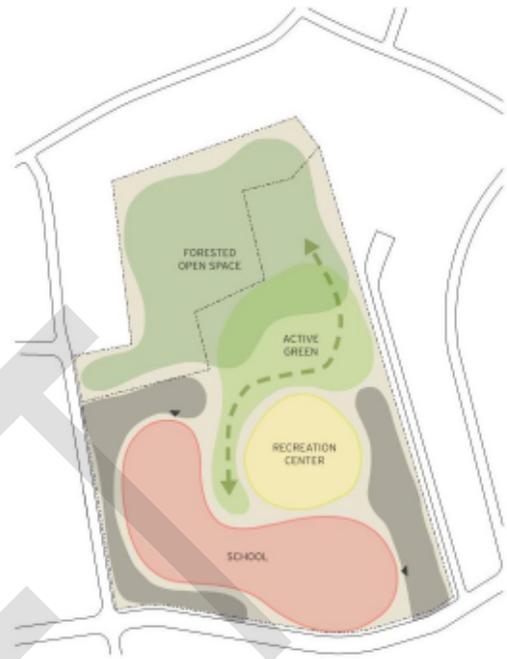
2.2. The program was described along with a graphic comparison of relative sizes of spaces.

3. Design Charette

3.1. SS presented an overview of each design, shown by a site plan, bubble diagram, and axonometric view. One of the key design decisions for the renovation schemes is which part of the existing building to demolish. There are only a few “hinges” in the existing building where a wing can be cleanly demolished. In both renovation concepts, the existing boiler room remains. The following items describe the discussion of pros and cons of each concept.

3.2. Renovation Option 1

New 3 story, compact addition wing connects to an atrium which fills the area formerly occupied by modular classrooms. An addition is also constructed along Taney Avenue. This concept keeps most of the existing school. Modular classrooms are relocated off-site. The gymnasium wing and Pre-K classroom wing are demolished. This scheme keeps at least 75% of the existing school.



PROS

- New face to school along Taney Ave
- Close relationship between Rec & School
- Rec Center is close to field
- Building massing most prominent along Taney Ave - gives presence to community
- Soft edges - there is less building massing along the perimeter of the site - sensitive to neighbors

CONS

- Requires swinging kids off-site to renovate existing building
- Earthwork needed to flatten tennis courts for soccer field
- Geotechnical analysis of tennis courts needed - possibly filled with debris
- Requires phasing by contractor - longer construction duration
- Large building footprint - less open space on site
- Power lines may need to be relocated for soccer field
- Some mature trees on Taney avenue in front of the school would be lost
- No clear identification of Recreation Center from street

Renovation Option 2

The classroom wing nearest Taney Ave is demolished to allow for a new 3-story addition. Modular classrooms are relocated off-site and the area is infilled with an atrium. The recreation center is positioned at the corner of N. Latham Street and Taney Ave. This scheme keeps about 75% of the existing school. Based on comments at the meeting, this scheme will be modified to switch the Recreation center and the parking. This option is the preferred renovation concept.



CONS

- Requires off-site swinging space during renovation of existing building
- Earthwork needed to flatten tennis courts for soccer field
- Large shared parking could create traffic congestion on Latham, takes up a lot of space, and unfriendly to neighbors
- Parking located along street frontage on N. Latham
- Large building footprint - less open space on site
- Bus drop off alongside Taney Ave - ACPS prefers off of road

PROS

- New face to school along Taney Ave
- Rec & school each have own presence to community
- Rec has street frontage, own identity
- Bus parking
- Most building volume positioned along Taney Ave

3.3. New Building Option 1

The new building is located to allow the existing building to remain in operation during construction. The building volumes are arranged around an active green forecourt, along Taney Ave, to be built upon demolition of the existing school building.



PROS

- Compact building massing frees up green space on site
- No swing space required
- Perspective view of building set back from street
- Massing with outdoor terraced classroom space
- Minimal earthwork required
- Activate street with green space

CONS

- 3-story building massing adjacent to single family homes on N. Latham
- Building volume disconnects program spaces into two blocks

3.4. New Building Option 2

The new building is arranged in an L-shape to anchor the corner of Taney Ave and N. Latham. The building is split into a 2-story volume along N. Latham and a 4-story volume along Taney Ave, opening its arms to the play fields in the center of the site. This concept requires removal of 1/3 of the existing building before construction. This option is the design team's preferred new building concept.



PROS

- School massing activates corner while still set back from street
- Plentiful open space on site - soccer forecourt and central play area balance building volumes, greenspace flows to wooded hill
- 2-story volume along N. Latham sensitive to neighbors
- 3-story along Taney activates street frontage
- Bus loop along N. Latham provides a buffer between building and street
- Large volume of Rec in back of site
- Sheltered play area large enough for multiple age groups

CONS

- Swing necessary for part of existing building
- Recreation parking adjacent to field
- Moving Rec building to back of site cuts up contiguous green space

4. Open Discussion

SITE CONSIDERATIONS

- 4.1. The question of recreation and school as separate buildings or connected was discussed. The city prefers separate buildings for simpler operations, but there is a functional advantage to creating a connection between the two for shared access. SS noted that all of the schemes could potentially be connected or separate. If connected, the connection should not be a conditioned space.
- 4.2. The volume of the recreation center will be large, since the indoor field needs a 40'-0" ceiling height. This is in the range of a 3-story volume, with a blocky square massing. Appropriately positioning this volume on the site is key to the design.
- 4.3. Soccer field also used for lacrosse, field hockey.
- 4.4. Parking landscape guidelines require 1 tree per 10 parking spaces. This will be considered.
- 4.5. Dispersed parking preferred.
- 4.6. Bus parking takes up a lot of space. It was agreed that the number of buses will be reduced to 13-15 instead of 20, and combining the bus loop with recreation center parking or with Pre-K drop off will be explored. ACPS confirmed that during dismissal, all buses are on the site at the same time.
- 4.7. Curb cuts were discussed as they relate to adjacent streets and pedestrian access. Distances between curb cuts will be considered for each concept.
- 4.8. ME noted that the school currently has two existing fields, and the concepts all have one field. This is in response to the site program. The recreation center will provide a second field, indoors.
- 4.9. The service entrance location was discussed. This will be considered for each concept.
- 4.10. The location of the school entrance depends on where the bus drop off is located.
- 4.11. Avoid bus drop off on street, especially on Taney.
- 4.12. All schemes allow for simultaneous construction of School and Rec center.
- 4.13. Maximize placement of the school for urban design response to neighborhood.
- 4.14. Needs of different age groups for playgrounds and site access will be considered.

BUILDING DESIGN

- 4.15. All schemes are using a double-loaded corridor for maximum efficiency.
- 4.16. With a multi-floor school, there is increased need for outdoor space. Sorg is exploring design possibilities of outdoor classroom areas integral to the building.
- 4.17. There was concern about the use of atrium space. SS explained that the area would contain program spaces that are appropriate in a large volume - it would not be wasted space.

OTHER CONSIDERATIONS

- 4.18. None of the options are just a renovation. The program is approximately 160,000 SF which doesn't fit into the 86,000 SF existing building.

- 4.19. School and Recreation have different occupant populations, this will be taken into consideration.
- 4.20. IB commented that new building options are strongly preferred to renovations.

QUESTIONS

- 4.21. Question of soccer field being turf - how does this relate to Alexandria RPCA's long term plans?
- 4.22. Question of relationship between school and recreation center, school and outdoor soccer. Should the school be a quiet zone away from these playful, active areas? Or should the school embrace them?
- 4.23. For renovation options, how much capacity could the existing building support while the addition is under construction? Sorg will identify this and include it in the study.

BUDGET

- 4.24. TC inquired about how the concepts work with the budget. SS responded that budget is a major consideration for all options, and that both new building options could cost the same. If at any point the budget is unrealistic Sorg will bring this up for discussion. SS remarked that the budget must be spent wisely - use money where it has the most impact on the kids, not on moving earth.
- 4.25. Federal funding could be potentially used for this project, such as Department of Energy, DOE grants, Gates Program, or HUD Resilient Design.

SUMMARY - MAJOR FACTORS DRIVING THE DESIGN FOR ALL OPTIONS

- Emphasize corner of N. Latham and Taney
- Activate Taney Ave through greenspace or building frontage
- Massing set back from N. Latham, be sensitive to neighbors
- Appropriate location of Rec Center massing
- Minimize hardscape, maximize open space
- Budget

NEXT STEPS

- 4.26. There is a programming meeting for the recreation center scheduled for Monday Feb 23 at 4pm.
- 4.27. A design meeting for the interior of the school layout will be held Thursday Feb 26 11am-3pm.
- 4.28. Sorg will follow up by sending the presentation electronically, with changes incorporated.
- 4.29. It was agreed that Sorg will develop 1 renovation scheme (Renovation Option 2 modified with parking away from the corner) and 2 new building schemes.

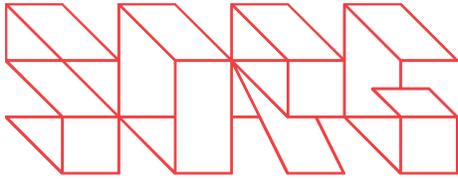
- END OF MEETING -

Errors and omissions should be brought to our attention within ten business days so as to be made a part of this record.

Recorded by: Clair Wholean

Copies to: Attendees

DRAFT



Location: Alexandria Department of General Services Conference Room
Meeting Date: 23 February 2015
Meeting Time: 4:00 pm
Subject: Recreation Program Meeting
Project: Patrick Henry Elementary School & Recreation Center
Project #: 1417

Attending:

Mark Eisenhour (ME)	ACPS	meisenho@acps.k12.va.us	703 772 1072
Beth Znidersic (BE)	RPCA	Bethany.Znidersic@alexandriava.gov	703 746 5492
Ron Kagawa (RK)	RPCA	ron.kagawa@alexandriava.gov	703 746 5489
Adrian King (AK)	Alexandria DGS	Adrian.King@alexandriava.gov	703 746 4770
Bill Conkey (BC)	Sorg	Billc@sorgarchitects.com	202 393 6445

Meeting Minutes

1. Recreation Center Program Elements

- 1.1. The size of the indoor soccer field has been revised from previous discussion; the field is now 180' x 120' and is to include a 10' wide runoff area on all sides.
- 1.2. In addition to the field and runoff area, on one of the short sides of the field there is to be a 20' x 140' hardscape area that is to be used for team gathering, recreational storage, lockable storage cubbies, and observation of games. A portion of this area is to become a "Flex court".
- 1.3. Clear height inside the soccer field area of the building is to be a minimum of 40'.
- 1.4. The required indoor track is to be elevated along the outside of the soccer field portion of the building and is to be 10' - 12' wide.
- 1.5. Other program elements noted in the Recreation Program are to be located in a two story portion of the building providing views overlooking the indoor soccer field.
- 1.6. The lower level of this volume could contain the Multipurpose/Rental space noted in the program with roll-up doors opening into the soccer field area.
- 1.7. There should be some type of Lobby/Atrium type of element that indicates where the main entrance to the building is located and serves as a design focal point for the exterior of the building.

- 1.8. The 20' x 30' exterior Hard-surface court indicated on the program may become an exterior element that is located adjacent to the building.
- 1.9. The Gymnasium Storage and Multi-purpose Activity spaces indicated on the program have been eliminated.
- 1.10. Bathroom/locker rooms with 1 male and 1 female shower are to be located on the lower level of the building adjacent to the field area.
- 1.11. Additional program elements as shown in the attached revised building program have been added.
- 1.12. The target for the overall recreation building size is to be between 33,000 and 35,000 square feet.

2. Recreation Building Components

- 2.1. Exercise rooms are to include cardio machines only, no weightlifting machines.
- 2.2. Roof of field area of building is to allow for areas of daylight.
- 2.3. Large garage doors could be included at the building exterior to allow for service access and to open the field area to the exterior when weather permits.
- 2.4. Scoreboards are to be provided in the field area.
- 2.5. Interested in soft flexible ductwork in the field area in place of hard ductwork to reduce potential damage by impact with play equipment.

- END OF MEETING -

Errors and omissions should be brought to our attention within ten business days so as to be made a part of this record.

Recorded by: Bill Conkey

Copies to: Mark Eisenhour, Ron Kagawa, Bill Conkey

02.23.2015

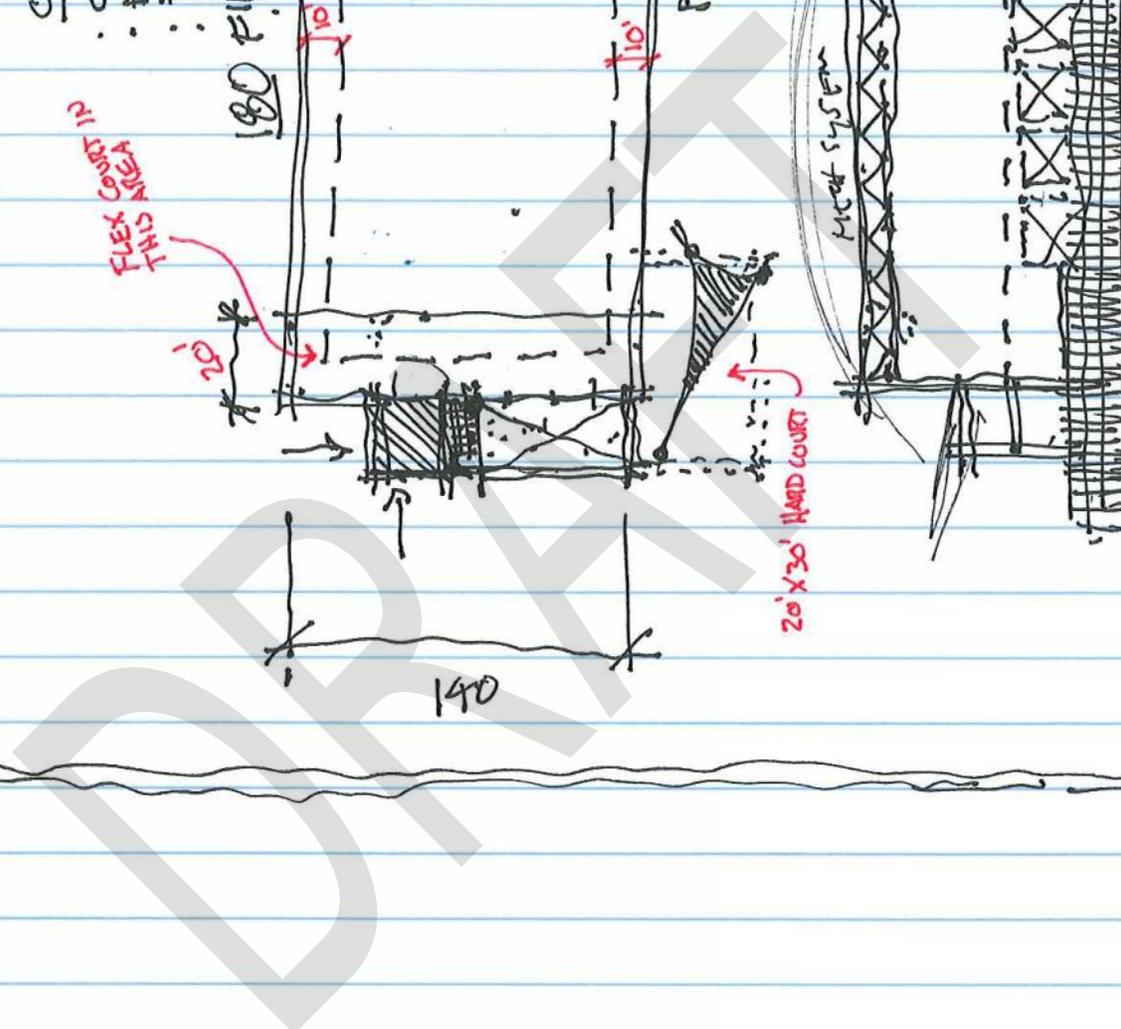
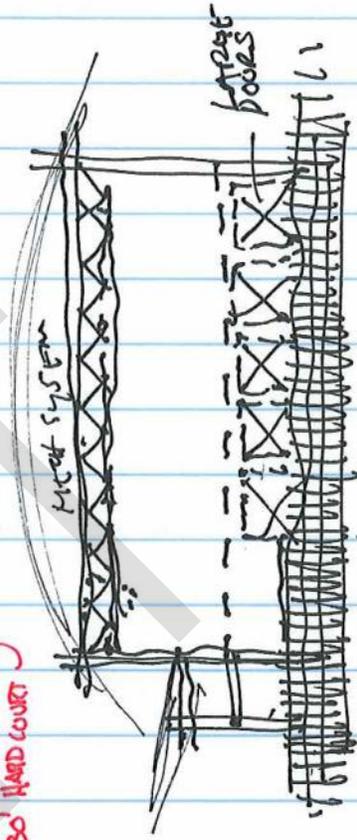
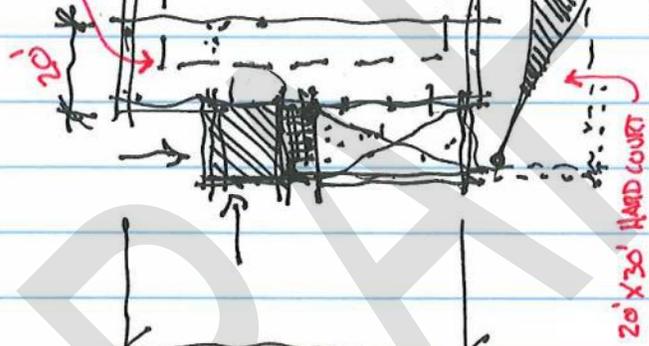
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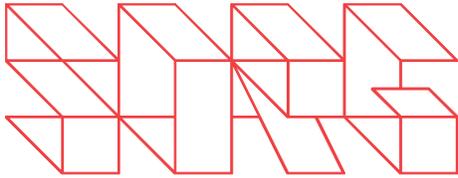
180 FIELD KING

120 FIELD

PLANS

20' X 30' HARD COVER
FLEX COVER THIS AREA





Location: ACPS Conference Room
Meeting Date: February 26, 2015
Meeting Time: 11:00 am
Subject: Patrick Henry Design Charrette
Project: Patrick Henry School and Recreation Center
Project #: 1417

Attending:

Mark Eisenhour (ME)	ACPS	Meisenho@acps.k12.va.us	703 965 3418
Laurel Hamming (LH)	ACPS	Laurel.hammig@acps.k12.va.us	703 619 8298
Ron Kagawa (RK)	Alexandria RPCA	Ron.kagawa@alexandriava.gov	703 746 5489
Tan Cross (TC)	Alexandria GIS	Titania.cross@alexandriava.gov	703 746 3203
Adrian King (AK)	Alexandria GSA	Adrian.king@alexandriava.gov	571 215 5345
Suman Sorg	Sorg	SumanS@sorgarchitects.com	202 393 6445
Bill Conkey (BC)	Sorg	BillC@sorgarchitects.com	202 393 6445
Sehee Kim (SK)	Sorg	SeheeK@sorgarchitects.com	202 393 6445
Clair Wholean (CW)	Sorg	Clairw@sorgarchitects.com	202 393 6445

Meeting held to review the preliminary interior layouts for the design options for the Patrick Henry School, and the updated program for the Recreation Center.

Meeting Minutes

1. Introductions

- 1.1. BC introduced all attendees and described the status of the project. Sorg is developing interior layouts for the design options, and finalizing the Recreation Center program.

2. Design

- 2.1. SS explained that the goal for these layouts is to identify the ideal adjacencies and stacking of the program. For now, the two new building options have been developed with an interior layout. Once adjacencies and stacking are determined, a layout for the renovation/addition scheme will be developed.

3. Discussion of School Layout

- 3.1. ME commented that special education should be integrated into the academic areas of the school, including the autism classrooms. The design will be revised to reflect this. OT & PT should be centrally located.

- 3.2. Shared spaces should have close proximity to the Recreation Center. This includes the Gym and Black Box or Auditorium, which would be used for cultural activities. These public spaces need to be able to be isolated from the rest of the school during the evening. At 4pm the Gym is used by the Rec center. People check in at the Rec center and then go to the gym.
- 3.3. In new building option 1, shared spaces will be moved closer to the Rec Center.
- 3.4. ME brought up the location of seating in the gym, especially when a screen is dropped. For assemblies, a U-shape bleacher arrangement may work well, so the gym could be used as an auditorium.
- 3.5. For a K-8 school with 10 grades, ideally there are 3 spaces for PE - a gym which can be subdivided and a multipurpose room. Currently the program does not include a multipurpose room.
- 3.6. Multipurpose Space can be used for dance, recess, and floor hockey, among other things. When accessible to the community, this would be accessed through the Rec center.
- 3.7. Media center would ideally go on the second floor.
- 3.8. Location of the art rooms should be distributed since kids of most grades travel to it. Since there are two rooms, they should be split up on different floors, such as on 2nd and 3rd floors.
- 3.9. Music room is ideally on a higher floor since it is easier to isolate sound.
- 3.10. ELA's should have proximity to the kitchen both vertically and horizontally.
- 3.11. Elevator must be next to kitchen.
- 3.12. Black Box and Gym go on ground floor for connection to Rec Center and community.
- 3.13. Service to buildings should be from an interior parking lot, not from the street, and be adjacent to the kitchen. The site should have a service lane which provides access to the Rec building and school.
- 3.14. The number of entrances was discussed. LH noted that ACPS would prefer to have all visitors pass through a single main entrance with security. This includes staff, parents, and kids. However, multiple entrances could work for arrival, since so many people are arriving at once. Sorg will look at how the student entrance works with security.
- 3.15. Pre-K Head Start starts 30 minutes after school, this helps with congestion of the entrance in the morning.
- 3.16. For new building option 2, the wing containing public spaces could be shifted closer to the Rec center, while still having the building anchor the corner.
- 3.17. The outdoor terrace on new building option 2 provides plentiful space for outdoor learning. Service access to this space will be considered.
- 3.18. Art & CTE do not need to be next to each other - ideally distributed throughout the academic areas.
- 3.19. Current designs have 14 buses on the site - this could be reduced to 10 or 12 if necessary.

4. Discussion of Recreation Program

- 4.1. BC presented the current Recreation program and explained changes in response to RPCA input.
- 4.2. The indoor soccer field would be subdivided into 3 fields for regular use. The size of the field is in question, RK will talk to Brailsford & Dunlavey to confirm the field size.
- 4.3. The field extension area could be smaller than 2,800 SF, with a portion of it used for storage space.
- 4.4. The multipurpose rental space would ideally be glass, with a partition to subdivide the space.
- 4.5. No bleachers are needed.
- 4.6. The target cost is \$175/SF, for 30,000-35,000 SF.
- 4.7. There may be possible environmental mitigation necessary for the ground below the tennis courts.
- 4.8. RK commented that everything in the Rec center should be multipurpose - no single-use spaces.
- 4.9. The architecture of the entrance should create a visual definition, with a 2-story space around the elevator. RK referenced the Greater Richmond Aquatics Center as an example of this.
- 4.10. Sorg will prepare a layout for the current program.

5. Next Steps

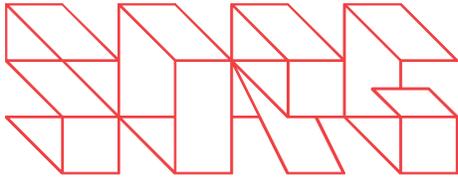
- 5.1. One more meeting will be held to discuss the layouts for the school design as well as the Recreation center layout. This will be Friday March 6 at 9am in the ACPS Conference Room.
- 5.2. The next community meeting is scheduled for March 18th.

- END OF MEETING -

Errors and omissions should be brought to our attention within ten business days so as to be made a part of this record.

Recorded by: Clair Wholean

Copies to: Attendees



Location: ACPS Conference Room
Meeting Date: March 9, 2015
Meeting Time: 11:30 am
Subject: Patrick Henry Design Charrette 3
Project: Patrick Henry School and Recreation Center
Project #: 1417

Attending:

Mark Eisenhour (ME)	ACPS	Meisenho@acps.k12.va.us	703 965 3418
Laurel Hamming (LH)	ACPS	Laurel.hamming@acps.k12.va.us	703 619 8298
Ron Kagawa (RK)	Alexandria RPCA	Ron.kagawa@alexandriava.gov	703 746 5489
Elijah Cross	ACPS		
Beth Znidersic (BZ)	Alexandria RPCA	Bethany.znidersic@alexandriava.gov	703 746 5492
Tan Cross (TC)	Alexandria GIS	Titania.cross@alexandriava.gov	703 746 3203
Adrian King (AK)	Alexandria GSA	Adrian.king@alexandriava.gov	571 215 5345
Ingrid Bynum (IB)	Alexandria PHES	Ingrid.bynum@acps.k12.va.us	240 354 4859
Suman Sorg	Sorg	SumanS@sorgarchitects.com	202 393 6445
Bill Conkey (BC)	Sorg	BillC@sorgarchitects.com	202 393 6445
Sehee Kim (SK)	Sorg	SeheeK@sorgarchitects.com	202 393 6445
Clair Wholean (CW)	Sorg	Clairw@sorgarchitects.com	202 393 6445

Meeting held to review the preliminary interior layouts for the design options for the Patrick Henry School and the Recreation Center.

Meeting Minutes

1. Introductions

- 1.1. BC introduced all attendees and described the status of the project. This meeting is the third design charrette. In the first charrette, Sorg presented four design options for the site as a whole, and three options were selected to be included in the feasibility study, two new building options and one renovation option. In the second charrette, Sorg presented interior layouts for the school and a program for the Recreation Center. Feedback from the discussion at the second charrette was incorporated into the design options, for presentation today at the third charrette.

2. Design

- 2.1. SS presented a revised design for each option, along with it a layout for the Recreation Center is included. The following describes updates to each option:

NEW BUILDING OPTION 1

- Revised to show multipurpose spaces shared with Recreation near Rec Center on first floor
- Administration is visible from both entrances
- Dining service entrance is included, from the rear
- Atrium with outdoor classrooms
- Building is set back from North Latham St.
- Bridge and covered walkway between second floor of school and Rec Center
- This option is focused around greenspace, buildings surround forecourt

NEW BUILDING OPTION 2

- Revised to show multipurpose spaces shared with Recreation near Rec Center
- Huge opportunity for outdoor learning to drive the design
- This option is focused on anchoring the corner of Taney & North Latham
- Efficiency, security, operations slightly enhanced over Option 1

RENOVATION OPTION

- Multipurpose spaces placed towards the front along Taney Ave
- Placement of large spaces is in addition because existing building doesn't have high enough ceilings. These spaces need height, ground floor, and adjacency to Rec Center.
- Outdoor classrooms off of 3rd floor
- Less efficient because keeps existing building
- Requires swinging kids during renovation of existing building
- Some trees are sacrificed
- Rec Center is placed along N. Latham St.

3. Discussion - All Options

- 3.1. The program will be revised to reflect the discussion on Special Education and Administration.
- 3.2. Parking was discussed. Currently the options show 190 parking spaces; this is based on the estimated number of staff at the school and gross square foot area of recreation. The parking layout shown incorporates compact and non-compact parking spaces per zoning requirements. Jefferson Houston has 126 parking spaces. It may be possible to reduce the number of parking spaces for Patrick Henry. Currently, many teachers park on surrounding streets. As parking is taking up a large footprint on the site, the quantity of parking needs to be feasible to fit on the site.
- 3.3. Sorg will prepare a phasing snapshot for each option, to be included in the feasibility study, with a visual comparison of each option to the original building.

- 3.4. A comment was made to indicate multiple age playgrounds on the drawings.
- 3.5. A comment was made to show the raised tennis court area on the drawings.
- 3.6. The school playground is open to the community during off hours.
- 3.7. Modular classrooms will be used elsewhere. It was noted that they have screw piles rather than footings.
- 3.8. The use of the main entrance in the morning and afternoon will be staggered base on arrival and dismissal timing. IB noted that the school would like to keep the same sequence, or similar, in the new building. The current arrival times are:
- 7:30 am Bus riders K-8 (held in auditorium for 10 minutes)
 - 7:40 am teachers
 - 7:45 am walkers and kiss & ride kids
 - Grades 2-5 have breakfast in the classroom
 - Kindergarten & 1st grade have breakfast in the cafeteria
- 3.9. For dismissal, Patrick Henry currently follows this sequence:
- Kindergarten walkers/kindergarten pickups
 - Bus Stage 1 - 4 buses
 - Bus Stage 2 - 4 buses
 - Walkers - parents pick up
 - Campanya & Rec after school
 - Pre-K - parents pick up at 2 doors
- 3.10. The most recent budget includes \$38M for the school and \$5M for the Rec Center. Jefferson Houston was \$45M.
- 3.11. TC commented that it is important to identify which schemes are realistic for the proposed budget. Sorg explained that each option should be as efficient and budget conscious as possible.
- 3.12. Kiss and Ride is best located on East side of site, to avoid traffic on N. Latham, and should be as close as possible to the school.
- 3.13. Include footprint of existing building on site diagrams for reference.
- 3.14. The Rec center will have a lower cost per square foot, compared to the school, because it will be a pre-engineered structure.
- 3.15. The entrance to the school should be pronounced and highly visible.
- 3.16. Areas of outdoor open space and outdoor usable recreation space should be included for each option.
- 3.17. The Rec center does not need a separate playground.

4. Discussion - New Building Option 1

- 4.1. RK commented that the circulation in Option 1 is more efficient than Option 2.

4.2. Placing the large volumes further back on the site (New Building Option 1) is friendlier to neighbors.

5. Discussion - New Building Option 2

- 5.1. Placing the kiss and ride near the soccer field was suggested.
- 5.2. Intersection improvements will likely be required because of the bus loop.
- 5.3. Flipping the school east-west could be considered.
- 5.4. ME commented that the main entrance is less clear in this option.
- 5.5. RK commented that a retaining wall would not be desired behind the recreation center. SS noted that the area of the former tennis courts will be terraced down to the building.
- 5.6. RK noted that rotating the field in this option pushes the Rec center and turnaround further into the site. The field creates a large open space in front of the building.

6. Discussion - Renovation Option

- 6.1. ACPS noted that financial analysis may be needed for the renovation option, it may be more expensive to retrofit. Sorg will evaluate this with their cost estimator.
- 6.2. In the renovation option, the field is positioned in the area most difficult to build in, topographically. Because the existing building takes up a large footprint on the site, placement of the field is constrained. TC suggested rotating the field in the renovation option.
- 6.3. The kitchen will be larger in the renovation option compared to the new building options.
- 6.4. The atrium could be a teaching garden.

7. Discussion - Renovation

- 7.1. The outdoor multipurpose field will be referred to as a multipurpose field, not a soccer field, and it should be drawn without lines shown.
- 7.2. The Rec center will likely require a traffic management plan through Planning & Zoning, this will drive the hours of operation. It could open at 6am if traffic is managed effectively.
- 7.3. Recreation layouts could be swapped between the new building options.
- 7.4. The Recreation Center will have its own address for deliveries.
- 7.5. Classroom proportions are more variable in this scheme. SS noted that this can be adjusted.

8. Next Steps

- 8.1. March 12th is the draft submission of the report. Sorg will share the draft Table of Contents with ACPS for review. This will be discussed at the next core group meeting.
- 8.2. Now that the designs are developed, Sorg will be working on the cost estimate for each option with the cost estimator.
- 8.3. In the study, ACPS would like to see a comparison of the quantity of outdoor open space and built footprint of the open area on site.

8.4. It was noted that all meeting minutes are subject to change with public input.

8.5. The next community meeting, scheduled for March 18th, will include the following:

- Open house Q & A discussion
- Review of cost recovery, school program, existing site, existing conditions diagrams
- Sorg will describe feasibility study process and schedule
- Introduction of Field House concept for Recreation Program
- Brief Presentation of Design Options

Sorg will prepare the presentation and send to ACPS for review.

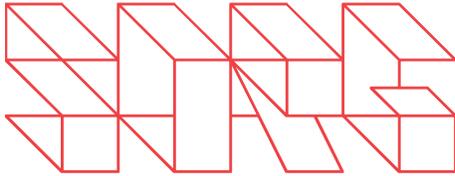
- END OF MEETING -

Errors and omissions should be brought to our attention within ten business days so as to be made a part of this record.

Recorded by: Clair Wholean

Copies to: Attendees

DRAFT



Location: Lee Center (RPCA)
Meeting Date: March 23, 2015
Meeting Time: 2:00 pm
Subject: Patrick Henry Design Meeting
Project: Patrick Henry School and Recreation Center
Project #: 1417

Attending:

Mark Eisenhour (ME)	ACPS	Meisenho@acps.k12.va.us	703 965 3418
Laurel Hamming (LH)	ACPS	Laurel.hammig@acps.k12.va.us	703 619 8298
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Elijah Cross (EC)	ACPS	Elijah.gross@acps.k12.va.us	
Beth Znidersic (BZ)	Alexandria RPCA	Bethany.znidersic@alexandriava.gov	703 746 5492
Adrian King (AK)	Alexandria GSA	Adrian.king@alexandriava.gov	571 215 5345
Suman Sorg (SS)	Sorg	SumanS@sorgarchitects.com	202 393 6445
Bill Conkey (BC)	Sorg	BillC@sorgarchitects.com	202 393 6445
Sehee Kim (SK)	Sorg	SeheeK@sorgarchitects.com	202 393 6445

Meeting held to review the preliminary interior layouts for the design options for the Patrick Henry School and the Recreation Center.

Meeting Minutes

1. Introductions

- 1.1. EC introduced the purpose of this meeting. The purpose of this meeting is to review the site plans presented at the community meeting on March 18, 2015 and discuss how these plans can be modified to incorporate the feedback received from the community.
- 1.2. ME indicated that the main concern of the community is the proximity of the new buildings to N. Latham St and the impact it might have on the smaller-scale houses across the street. The community is also concerned about the increase in traffic the new construction is expected to create.

2. Site Plan Discussion

- 2.1. SS presented five site plans for discussion including three site plans that were presented at the community meeting and two modified site plans based on the comments received during the community meeting.

- New Building Option 1A site plan is the New Building Option 1 site plan presented at the community meeting.
 - New Building Option 1B site plan is a modified version of New Building Option 1A site plan, where the new school construction will occur on the site of the existing school. This is to incorporate the feedback from the community by placing all the buildings as far from N. Latham St. as possible. It is noted here that this option would require all of the students to be placed into swing spaces as the existing school will have to be completely demolished in advance of the new building construction. Recreation Center retains its location but is rotated to face Latham St instead of Taney Ave to maintain a presence on a street front.
 - New Building Option 2A site plan is the New Building Option 2 site plan presented at the community meeting.
 - New Building Option 2B site plan is the modified version of New Building Option 2A site plan where the school building is moved away from N. Latham St as much as possible without infringing on the existing school site.
 - Renovation + Addition Option site plan is the Renovation + Addition option presented at the community meeting.
- 2.2. RK outlined the main items that the revised site plans should achieve: strengthen the street front on Taney Ave, move away from N. Latham St as much as possible and more interior parking and traffic circulation to limit traffic congestion on N Latham St.
- 2.3. New Building Option 1A and Option 1B site plans
- New Building Option 1A can be modified to limit the amount of swing space.
 - EC stated the need for New Building Option 1B to show that the design team had considered this type of strategy (building on the existing school site) in the comparative analysis as it directly incorporates the feedback from the community. It will be helpful to study the impact of using this strategy compared Option 1A.
 - It is noted that New Building Option 1B will require about 40 classroom trailers, administration space trailer and Dining space trailer, a significantly larger number compared to other options.
 - It is noted that these trailers can possibly be an investment made by the city for future projects.
 - New Building Option 1B is to be developed as a variation of Option 1A. It does not need to be a full-blown scheme and only the site plan and cost are required for comparison.
- 2.4. New Building Option 2A and Option 2B site plans
- It was agreed that the design team will move forward with New Building Option 2B site plan instead of New Building Option 2A site plan as it better incorporates the feedback from the community. New Building Option 2A can be eliminated.
 - It was noted that this option would not require any swing space for the school.
- 2.5. Renovation + Addition Option site plan

- It was suggested that the Recreation Center be relocated away from N. Latham St and more towards the central area of the site. There was a concern that this would result in the Recreation Center losing its presence on a street front. SS suggested rotating the Recreation Center to face N. Latham St. instead of Taney Ave to maintain a presence on a street front.
 - Phasing will be further studied to limit the amount of swing space required for the School.
- 2.6. In all options, there is a possibility of using a lay-by lane for bus pick-up and drop-off in lieu of an interior bus loop to conserve some site area.
- 2.7. In all options, perimeter roads should be incorporated to provide more interior traffic circulation to alleviate traffic congestion on the adjacent roads.

3. Feasibility Report 1st Draft Comments

- 3.1. RK emphasized that the main purpose of this feasibility study is to analyze whether the proposed program fits within the given site and the cost. The report can be more condensed and geared toward these goals.
- 3.2. RK stated that the separate landscape plan for each option is not necessary and the rendered site plans are sufficient to show the concept. It was suggested that the landscape section in each of the options is eliminated and condensed into a single landscape concept narrative.
- 3.3. School and Recreation Center should be discussed separately.
- 3.4. RK is concerned that the feasibility study contains too much information for the school board to review. The options and their respective pros & cons should be highlighted and formatted in a way that can be easily understood by the school board.
- 3.5. BC suggested having a separate executive summary document, significantly condensed version of the feasibility study, for submission to the school board.

4. Next Steps

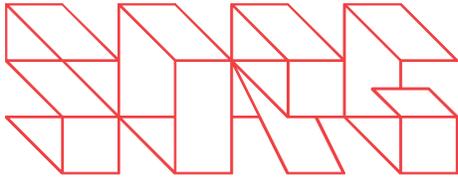
- 4.1. Revised site plans to be sent via email by Sorg for review before the submission of the Feasibility Study - 2nd draft.
- 4.2. The format of the executive summary document will be included in the 2nd draft submission of the Feasibility Study for review and comments before the final submission.

- END OF MEETING -

Errors and omissions should be brought to our attention within ten business days so as to be made a part of this record.

Recorded by: Sehee Kim

Copies to: Attendees



Location: Lee Center (RPCA)
Meeting Date: May 28, 2015
Meeting Time: 1:30 pm
Subject: Patrick Henry Recreation Center Design Meeting
Project: Patrick Henry School and Recreation Center
Project #: 1417

Attending:

Mark Eisenhour (ME)	ACPS	Meisenho@acps.k12.va.us	703 965 3418
Titania Cross (TC)	Alexandria DGS	Titania.cross@alexandriava.gov	703 746 3203
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Adrian King (AK)	Alexandria DGS	Adrian.king@alexandriava.gov	571 215 5345
Suman Sorg (SS)	Sorg	SumanS@sorgarchitects.com	202 393 6445
Rachel Chung (RC)	Sorg	RachelC@sorgarchitects.com	202 393 6445
Bill Conkey (BC)	Sorg	BillC@sorgarchitects.com	202 393 6445
Sehee Kim (SK)	Sorg	SeheeK@sorgarchitects.com	202 393 6445
Dennis Quinn (DQ)	CCL	Dennisquinn@ccl-eng.com	703 273 6820

Meeting held to discuss the updated building design options for the Patrick Henry Recreation Center and their impact on the current site plans and School building plans.

Meeting Minutes

1. Introductions

- 1.1. RC introduced the purpose of the meeting: to present the latest design options for the Patrick Henry Recreation Center and to discuss their impact on the current site and building plans for the School.
- 1.2. RK questioned whether the updated site plans were designed based on the Parks and Recreation Needs Assessment for design principles. Basis of site design principles should be produced to guide the site design process.
- 1.3. BC responded that site design principles and how the site program responds to the Needs Assessment will be incorporated in the feasibility study.

2. Site and Building Design Options Discussion

- 2.1. BC presented the updated Recreation Center building plans.
 - Recreation Center design option A is a one-story building option that can be either free-standing or attached to the new School. In the case this building is attached to the new

School, slight modifications to the east wing of the School are required. This option has a 100'x120' indoor multi-use recreations space with a 12' wide track around it.

- Recreation Center design option B is a two-story version of Option A that can also be either free-standing or attached to the new School. In the case the building is attached, the east wing of the School requires slight plan modifications. This option has a 100'x120' indoor multi-use recreation space with a 12' wide runoff around it and an elevated track above.
- Recreation Center design option C is a one-story building scheme, attached to the new school, with no indoor multi-use recreation space. Instead, there is a 50' x 84' Flex Court with a 12' track around it. In order to accommodate the Recreation Center plan, attached to the new School, slight modifications to the School east wing plan are required.

2.2. BC presented the updated site plans. All Recreation Center site plan options utilize Option 2 Site Plan, the option preferred by the School.

- Site plan option 2.1 maintains most of the original site plan option 2 with slight modifications only occurring around the Recreation Center building. This site option illustrates Recreation Center building option A and B free-standing on the northeast corner of the site.
- Site plan option 2.2 maintains most of the original site plan option 2 with slight modifications only occurring around the Recreation Center building. This site option illustrates Recreation Center building option A and B attached to the east wing of the School.
- Site plan option 2.3 maintains most of the original site plan option 2 with slight modifications only occurring around the Recreation Center building. This site option illustrates Recreation Center building option C attached to the east wing of the School.

2.3. SS outlined some important features of the updated site plans.

- In all of the site options, Recreation Center has a clear, separate entrance from the School even in cases where the Recreation Center is attached to the School.
- Recreation Center is clearly visible from Taney Ave.
- There's a generous amount of open space directly in front of the Recreation Center
- Convenient site access to the Recreation Center is maintained.
- School building plans are impacted minimally.

3. Site Discussion

3.1. RK voiced budget concerns related to cutting into the hillside and having retaining walls on the north end of the site. Questioned whether there are alternatives to avoid such measures.

- DQ stated that the important factor in sitework is balancing cut and fill. Because the site is large, cut will be required perform sitework with little import. This explanation will be elaborated in the civil narrative and incorporated into the feasibility study.

- SS added that sitework to cut down the hill adds valuable land area to the project. Moreover, the site cost estimate includes excavation of the hill.
- 3.2. RK voiced concerns about stormwater management and whether stormwater strategies are incorporated into the site plan options.
- DQ listed several possible stormwater strategies such as green roof, bio-retention ponds, cisterns and grass swales. At this early stage, detailed programming and drawings are not provided. Details will be developed once an option is selected.
 - RC added that stormwater strategies are incorporated into the site cost estimate. There are many suitable areas in the site plans for the listed stormwater strategies. More details will be incorporated into the feasibility study.
 - AK stated that the site plans will be shared with stakeholders and it is important to show exactly how the budget is being spent on what areas of the site. Therefore, more information on the specifics of the site is required, such as the number of parking for the School versus the number of parking for the Recreation Center and the specific stormwater strategies.
 - BC explained that the number of parking spaces needs to be verified with Alexandria Planning & Zoning. The details will be provided in the feasibility study.
- 3.3. ME highlighted some areas on the current site plans that are not working such as the Kiss & Ride and access from Latham. But stated that these issues should be dealt with in the next phase and confirmed that the School is not expecting any new drawings for the current phase.
- TC also confirmed the direction given to Sorg, to keep the appearance of the site plans provided to the School Board, with slight modifications directly surrounding the Recreation Center. Additional site details can also be worked out in the next phase.

4. Feasibility Study

- 4.1. RC confirmed that more details on stormwater management, number of parking required, grading and outdoor programs will be incorporated into the feasibility study.
- 4.2. TC requested that Sorg incorporate discussion on tree preservation along Taney Ave. in the feasibility study.
- 4.3. ME questioned the process moving forward with the updated Recreation Center building designs.
- TC responded that the team will make a recommendation to the Council and the Council will make a decision on the program options.
- 4.4. TC stated that the Recreation department is also looking at an option of not having a Recreation Center building and just providing after-school care that gets absorbed into the School building.
- 4.5. TC confirmed that rendered images of the Recreation Center buildings are not required at this stage.

4.6. TC questioned whether site costs will be different from the ones previously provided. RC confirmed that at this stage, site cost is the same for different site plan options and won't change from the previous cost estimate.

5. Next Steps

5.1. Feasibility Study to be sent via email by Sorg for review by June 5, 2015.

- END OF MEETING -

Errors and omissions should be brought to our attention within ten business days so as to be made a part of this record.

Recorded by: Sehee Kim

Copies to: Attendees

DRAFT